



FUNCTIONAL SERVICING REPORT

Water, Sanitary, and Stormwater Management

CONDOMINIUM TOWNHOUSE AND COMMERCIAL DEVELOPMENT

16114 AIRPORT ROAD
TOWN OF CALEDON

OUR FILE: 1599

PREPARED FOR SHACCA CALEDON HOLDINGS INC.

OCTOBER 29, 2019

REVISION HISTORY

DATE	REVISION	SUBMISSION
March 15, 2017		Issued for Rezoning Application
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1.0 INTRODUCTION

1.1 Scope of Functional Servicing Report

This report had been prepared in support of the Rezoning Application made by Shacca Caledon Holdings Inc. (the client) for the site located at 16114 Airport Road (the Subject Lands) in the Town of Caledon. The report examines the availability of municipal services (storm, sanitary and water) to support the proposed development, and provides preliminary servicing details and outlines storm water management requirements. The information provided in this report is preliminary, and detailed engineering designs and drawings will need to be undertaken as part of the site plan design and prior to submission for building permits.

This report has been prepared for the exclusive use of Shacca Caledon Holdings Inc. and cannot be used by other parties without the prior written approval of Trafalgar Engineering Ltd. *Revisions to the body of this report are indicated in italics.*

1.2 Site Location and Description

The site is located on the north-west corner of the intersection of Airport Road and Walker Road in the Town of Caledon. The site is approximately 4.08 ha and currently occupied by a two storey designated heritage home known as Allison's Grove. The site is bounded by Airport Road to the east, a condominium townhouse development to the north, wetland to the west, and Walker Road West to the south. Approximately one third of the southern frontage is interrupted by two residential lots: numbers 5 and 7 Walker Road West.

A copy of the topographic survey has been included in Appendix 'A' for reference.

1.3 Proposed Development

The client proposes to divide the subject lands into two development parcels; a breakdown of the proposal is as follows: a residential condominium of approximately 0.944 ha, a commercial lot of approximately 0.557 ha, an Airport Road widening of approximately 0.067 ha, and the remaining 2.514 ha of wetland, *compensation area, and associated buffer* to remain in its natural condition beyond the limits of the development. The heritage home sits along Airport Road approximately half way along the site frontage and is to remain.

The townhouse condominium consists of *five* blocks of between *six* and *eight* units each for a total of 32 units. Access to the residential condominium is proposed via condominium road from Airport Road, approximately 170 m north of Walker Road. The T-shaped condominium road runs west from Airport Road with a north-south stem dividing the residential and commercial components; on-street parking is proposed along the east side of the stem.

The commercial component of the development consists of the heritage home converted to commercial space, as well as a large commercial building to its south. Two entrances are proposed to the commercial re-development: one from Airport Road approximately 95 m north of Walker Road, and one from Walker Road approximately 55 m west of Airport Road. A copy of the site plan is included in Appendix 'A'.

1.4 Grading Requirements and Restrictions

The grading of the site is widely constricted due to the bounding conditions around the site and the large elevation difference from north to south. Grades along the northerly property line are approximately five metres higher than those along the southerly limit, giving a general down gradient of 2.6% in a north-south direction. The site falls off generally at a gradient of 10% east to west from a central mound at the north end of the site; the grade difference is proposed to be made up by *daylighting through the mount at a mild slope to match the existing grade along the west limit*. TRCA has reviewed the cross-sections proposed (shown on the Grading Plan, G1) and finds the approach acceptable. A retaining wall is proposed along the northern limit to allow the townhouse units to sit at a reasonable height relative to the fronting condominium road, while preserving the existing elevations at the rear lot line; the wall along the north limit ranges from approximately 1.1 m to 2.4 m high.

The commercial development is graded to be generally compatible with the condominium road grading to the west, the existing heritage house to the north, and with consideration to a 2.0% boulevard along both Walker and Airport Roads; a lane widening and urbanization of Airport Road has been anticipated in the preliminary grading.

The preliminary grading design has attempted to contain drainage from the development, however local portions of the existing heritage frontage will continue to sheet flow to Airport Road. A copy of the preliminary grading plan is included in Appendix 'B' for reference.

2.0 MUNICIPAL WATER AND WASTEWATER

Development of the subject lands will require that municipal services are designed in conformance with the Ontario Building Code, current Region of Peel standards, and current Town of Caledon standards for condominium developments. A servicing plan showing the proposed water, sanitary, and storm servicing is included in Appendix 'B'.

2.1 Water

There are two existing watermains along Airport Road adjacent to the development: an existing 400 mm PVC watermain running along the west side of Airport Road (constructed in 2014), and a

300 mm PVC watermain running along the east side of Airport Road (constructed in 1984). There is a 300 mm watermain located on the north side of Walker Road West.

The proposal includes a 150 mm watermain connected at one end to the 300 mm watermain on Airport Road and the 150 mm watermain on Walker Road West at the other to service the condominium development. A 150 mm fire and 50 mm domestic connection is proposed to service the commercial development. Fire and domestic sizing for the commercial development should be confirmed by a mechanical consultant prior to building permit approval. The heritage house is currently connected to the 300 mm watermain, and that connection will remain in service.

Water usage for each service has been estimated using the Region's design criteria and is provided below. Refer to supporting calculations provided in Appendix 'C' for further detail.

	Commercial Service	Residential Service
Average Daily Demand	5	20 (L/min)
Maximum Hourly Demand	16	60 (L/min)
Maximum Daily Demand	8	40 (L/min)
Estimated Fire Demand (FUS 1999)	8000	9000 (L/min)
Maximum Daily Plus Fire Demand	8008	9040 (L/min)

2.2 Wastewater

There are existing 250 mm PVC sanitary sewers on both Airport Road and Walker Road West. The Airport Road sewer will serve as the connection point for the commercial development, while the condominium townhouse development connects to the Walker Road West sewer.

The 250 mm PVC sanitary on Airport Road drains southerly beginning just north of the subject lands. The sewer was constructed to collect drainage from the adjacent townhouse development to the north; the sewer runs along the east side of Airport Road alongside the 300 mm watermain.

It does not appear as though the existing heritage house has a connection to the sanitary sewer; we therefore recommend that any existing sewage system be located and removed subject to approval, and replaced with a 150 mm sanitary connection to Airport Road complete with property line inspection maintenance hole. It is proposed that the commercial development be provided with a separate 150 mm sanitary connection and inspection maintenance hole south of the new connection to the heritage house.

Due to the site grading constraints, the proposed townhouse development will need to be serviced by a 200 mm connection to Walker Road West via an easement over the proposed commercial development. It is not practical to work against the gradient of the site to connect at a higher point along Airport Road. The existing sanitary sewer on Walker Road West drains in an easterly

direction just south of the centreline of the roadway. The sanitary flows have been estimated using the Region's criteria and are summarized below. Refer to the calculations provided in Appendix 'C' for further detail.

	Commercial Service	Residential Service
Average Daily Dry Weather Flow	13.0	13.0 (L/s)
Peak Daily Flow (Incl. Infiltration)	13.7	15.3 (L/s)

3.0 STORM DRAINAGE AND STORMWATER MANAGEMENT

3.1 Existing Condition

The pre-development condition of the site is mostly pervious; the impervious areas of the site consist of the existing heritage house and gravel driveway. The stormwater management quantity control requirements for the site are to use unit flow rates for the Humber Watershed (Sub-basin #6, Equation B), refer to the attached email correspondence from TRCA in Appendix 'D'. Because the unit flow rates determine the allowable discharge for the development, pre-development conditions (including pre-development flow) are not examined in-depth.

3.2 Proposed Condition

Due to grading constraints around the existing heritage building, a small portion of the commercial area drains back into the residential area; the drainage areas are adjusted accordingly and illustrated on Figure 1, Drainage Area Plan, in Appendix 'D'. The drainage areas are used to determine the allowable discharge for each component of the development. TRCA requires site discharge for each component (Residential and Commercial) to be limited to a rate determined using Equation B for Humber Watershed sub-basin #6. A table summarizing the allowable discharge rates for the following equation is provided below:

TABLE 1 – ALLOWABLE RELEASE RATES

Return Period	Equation B (Sub-basin 6)	Residential Allowable (Drainage Area=0.977)	Commercial Allowable (Drainage Area=0.523)
100-year	$Q = 15.199 - 0.751 \times \ln(A)$	$Q = 15 \text{ L/s}$	$Q = 8 \text{ L/s}$
50-year	$Q = 12.692 - 0.623 \times \ln(A)$	$Q = 12 \text{ L/s}$	$Q = 7 \text{ L/s}$
25-year	$Q = 10.488 - 0.522 \times \ln(A)$	$Q = 10 \text{ L/s}$	$Q = 6 \text{ L/s}$
10-year	$Q = 7.707 - 0.382 \times \ln(A)$	$Q = 8 \text{ L/s}$	$Q = 4 \text{ L/s}$
5-year	$Q = 5.755 - 0.283 \times \ln(A)$	$Q = 6 \text{ L/s}$	$Q = 3 \text{ L/s}$
2-year	$Q = 3.288 - 0.159 \times \ln(A)$	$Q = 3 \text{ L/s}$	$Q = 2 \text{ L/s}$

The storage required to attenuate the flow to each allowable discharge rate is determined using HydroCAD and the TRCA 6- and 12-hour AES storm distributions. Due to potential blockage or clogging, an orifice control smaller than 75 mm is not recommended. The model is run with a 75 mm orifice control to determine estimated storage usage and release rates; the maximum quantity determined using the 6- and 12-hour distributions is used. The orifice control is to be located downstream of the storage units. Storage is proposed in the form of underground modular cellular storage (ACO StormBrixx or approved equal) lined with permeable geotextile and set on a granular base to promote infiltration; some surface ponding is anticipated in the lower frequency return storms. *Approximately 375 m³ of storage is provided for the residential component, and approximately 189 m³ is provided for the commercial component. Both systems are located within the proposed commercial parking lot; a servicing easement is required for the residential system.*

Tables below summarize the storage and discharge for the residential and commercial components for both the 6- and 12-hour AES distributions. Table 4 below provides a summary of the required storage (maximal usage) while Table 5 provides a summary of the allowable and calculated overall discharge rates.

Refer to Tables 2 and 3 for estimated Residential and Commercial storage usage and discharge with 75 mm orifice control.

**TABLE 2 – RESIDENTIAL COMPONENT
FLOW AND STORAGE SUMMARY
(with 75 mm Orifice)**

Return Period	6-hr TRCA AES		12-hr TRCA AES	
	Allowable Flow (L/s)	Calculated Discharge (L/s)	Storage Used (m ³)	Calculated Discharge (L/s)
100-yr	15	11.3	304.5	11.3
50-yr	12	10.2	257.1	10.3
25-yr	10	9.2	211.0	9.3
10-yr	8	7.6	153.8	7.8
5-yr	6	6.2	112.0	6.5
2-yr	3	3.9	60.3	4.4

**TABLE 3 – COMMERCIAL COMPONENT
 FLOW AND STORAGE SUMMARY
 (with 75 mm Orifice)**

Return Period	6-hr TRCA AES		12-hr TRCA AES	
	Allowable Flow (L/s)	Calculated Discharge (L/s)	Storage Used (m ³)	Calculated Discharge (L/s)
100-yr	8	8.6	180.7	8.6
50-yr	7	7.3	157.2	7.4
25-yr	6	6.3	134.1	6.4
10-yr	4	4.9	105.3	5.1
5-yr	3	3.6	84.5	3.9
2-yr	2	1.0	59.6	2.0

**TABLE 4 – OVERALL STORAGE
 REQUIREMENT (in m³)**

Return Period	Residential	Commercial
100-yr	305.9	180.7
50-yr	259.5	157.2
25-yr	216.1	135.6
10-yr	160.5	108.6
5-yr	120.3	89.3
2-yr	69.2	66.3

**TABLE 5 – OVERALL DISCHARGE
 REQUIREMENT (in L/s)**

Return Period	Allowable	6-hr TRCA AES	12-hr TRCA AES
		Calculated	Calculated
100-yr	23	19.5	19.4
50-yr	19	17.6	17.6
25-yr	16	15.5	15.7
10-yr	12	12.5	12.9
5-yr	9	9.9	10.5
2-yr	5	4.9	6.4

3.3 Minor Storm System

Storm drainage is to be collected by a series of inlets and conveyed to the underground storage for both the residential and commercial developments. *Both the residential and commercial storm systems are sized to capture and convey the 100-year Town of Caledon storm.* A Stormceptor oil/grit separator is proposed upstream of the modular storage units to reduce the sediment settling within the storage units. Drainage is controlled by way of control structures complete with 75 mm orifice at the downstream end of the storage units; inspection maintenance holes are provided on each storm service at the property line. The proposed storm outlet for the residential component is a 200 mm connection to a catch basin manhole at the south-east corner of the site. A storm sewer easement over the commercial lands will be required. The commercial component will be serviced by a 200 mm connection to the existing ditch on Airport Road at the south-east corner of the site.

3.4 Major Storm System

In the event of an extreme rainfall event or failure of the storm sewer system, emergency overland flow for the residential development is directed to *Walker Road West by way of a driveway connection through the commercial component.* The condominium road will serve as the primary conduit for overland flow, directing flow south.

The commercial development has been graded to limit ponding *in the parking area to 0.25 m.* The overland flow for the commercial development is north to south via the parking lot to Walker Road West.

3.5 Water Quality

An oil/grit separator is proposed upstream of the underground storage units for both the residential and commercial components. This is to help minimize the settlement of sediment in the modules. Both units are sized to provide MOECC "Enhanced" level of protection (80% long-term removal of suspended solids). A Stormceptor STC 2000 is proposed for the residential development, and a Stormceptor STC 750 is proposed for the commercial development. Sizing calculations are included in Appendix 'D'. *It should be noted that exaggerated impervious areas were used in the sizing of the units.*

3.6 Erosion and Sediment Control

Typical sediment and erosion control measures will be required during construction. These include silt fencing, mud mat, and check dams with provisions for re-vegetating the site as soon as construction is completed. *Although more detailed information will be provided at the detailed*

design stage, it is recognized that special care must be taken to protect the wetland from sediment laden runoff and erosion potential.

3.7 Feature Based Water Balance

The subject lands are bounded to the west by a small portion of the bottom end of the Provincially Significant Caledon East Wetlands Complex. The wetlands catchment basin is illustrated in Appendix 'E', along with a highlighted portion of the subject lands that is to be redirected to Airport Road as a result of development. The overall catchment area for the wetland complex is approximately 506.7 ha; the area to be redirected away from the wetland as a result of the proposed development is approximately 0.22 ha, representing roughly 0.045% of the total catchment area for the wetland. Because of the proximity of the proposed development to the adjacent wetland (named Wetland #2, refer to Appendix 'E'), Toronto and Region Conservation Authority requires a Feature Based Water Balance analysis to be undertaken to assess the potential impact as a result of development of the subject lands.

An analysis of the existing and proposed conditions is undertaken using a continuous hydrologic model of both cases, comparing the pre- and post-development evaporation, infiltration, and runoff for the areas that are tributary to the wetland. The same rainfall and temperature data, time steps, analysis duration, and soil parameters are used in both cases. Refer to Figures 3 and 4 in Appendix 'E' for the pre- and post-development drainage area boundary delineation referenced in this text.

3.7.1. Hydrologic Model and Parameters

A continuous hydrologic model using EPA SWMM is setup using estimated parameters. One hour rainfall data for the period 23/05/1986–01/11/2007 measured at Toronto Buttonville Airport was provided by TRCA to be used in the model. Daily temperature data for the same period was also provided by TRCA and is used by the model to estimate evaporation using the Hargreaves method. Further detail on model parameters and subcatchment definition is provided below.

3.7.1.1. Data Limitations

It is noted that the total volume of rainfall provided in the hourly data (9,471 mm) does not equal the total volume of rainfall provided in the daily record (15,283 mm). The hourly rainfall data includes a number of erroneous (flagged as missing) readings; when cross-checked against the daily rainfall records, it is noted that the missing readings correspond to days with higher rainfall totals. Additionally, it should be noted that there are no rainfall events recorded for the months of December–March. Notwithstanding the foregoing, we believe the impact is insignificant.

3.7.1.2. Infiltration

The Green-Ampt infiltration method is selected based on the availability of known soil parameters. The hydraulic conductivity for the soil was calculated by R.J. Burnside based on in-situ testing at two monitoring wells; an average hydraulic conductivity between the two monitoring wells of 4.3×10^{-4} cm/s is converted to 15.5 mm/hr and used for the simulation. Other required parameters for the Green-Ampt method are Suction Head and Initial Deficit, both of which are obtained from published tables and interpolated based on the hydraulic conductivity. The parameters used in the model are discussed in further detail in Appendix 'E'.

It is worthy to note that the prevailing interpreted groundwater flow direction is north to south; most of the infiltration volume estimated for the development area is not tributary to the wetland. Refer to the "Interpreted Groundwater Flow" figure prepared by R.J. Burnside included in Appendix 'E'. Accordingly, there is no discussion of any net change in infiltration volume in the results.

3.7.1.3. Subcatchment Areas

Two subcatchment areas have been delineated and modelled to determine the existing evaporation, infiltration, and runoff that drains to the wetland. The two subcatchment areas are identified as "Buffer" and "Dev-Area", and are illustrated on the attached subcatchment area plans (Figures 3 and 4, Appendix 'E'). The development area ("Dev-Area") is delineated to the east based on the contours provided on the topographic survey prepared by David B. Searles (Appendix 'A'). It is bounded to the north and south by the property boundaries, and to the west by the development limit. The topography of this area is more steeply sloped than the adjacent Buffer subcatchment area. The Buffer area is bounded to the east by the development limit, to the north and south by the property boundaries, and to the west by the surveyed "drip line". The Buffer area is beyond the limits of the development, and will remain unchanged. The development area subcatchment drains to the Buffer subcatchment in the existing condition, but will drain toward Airport Road upon development. The Buffer subcatchment outlets to the wetland in both cases. A summary of the parameters used to define the subcatchments as well as the model input file is provided in Appendix 'E'.

3.7.2. Results

A simulation is performed with the development area outlet directed to the Buffer in order to evaluate the existing condition, and again with the development area disconnected—representing the post development condition. The results indicate a net reduction in total runoff of approximately 2 mm from the Buffer to the wetland over the entire simulation period (approximately 0.1 mm/year). The net change in runoff is negligible, and it can be concluded that

there is no significant impact to the wetland as a result of redirecting the development area toward Airport Road.

Pre- and post-development weekly hydrographs are not provided; the absence of any meaningful runoff in both conditions makes a graphical representation of these values impractical. In the pre-development condition, there are a total of three runoff events over the entire simulation period from the Buffer area to the wetland ranging between 0.002 and 0.019 m³/s; there are also three runoff events in the post-development condition ranging between 0.002 and 0.019 m³/s. Pre- and post-development simulation results over the entire period are tabulated below.

**PRE-DEVELOPMENT SIMULATION RESULTS
 (1986–2007)**

Subcatchment Name	Total Precipitation (mm)	Total Evaporation (mm)	Total Infiltration (mm)	Total Runoff (mm)	Peak Runoff (m ³ /s)
Dev-Area	9471.10	0.96	9466.88	4.36	0.01
Buffer	9471.10	0.99	9468.39	3.83	0.02

**POST-DEVELOPMENT SIMULATION RESULTS
 (1986–2007)**

Subcatchment Name	Total Precipitation (mm)	Total Evaporation (mm)	Total Infiltration (mm)	Total Runoff (mm)	Peak Runoff (m ³ /s)
Buffer	9471.10	0.98	9467.62	3.05	0.01

**NET CHANGE
 (1986–2007)**

Subcatchment Name	Total Precipitation (mm)	Total Evaporation (mm)	Total Infiltration (mm)	Total Runoff (mm)	Peak Runoff (m ³ /s)
Buffer	0.00	-0.01	-0.77	-0.78	-0.01

3.7.3. Summary

The results of continuous hydrologic modelling over the period of 1986–2007 indicate that there is no significant impact to the wetland as a result of redirecting flow from the development area away from the wetland; there is an insignificant net change in total runoff volume to the wetland (in the order of 0.04 mm/year). Rainfall over the simulation period is mostly infiltrated; the prevailing interpreted groundwater flow direction—as determined by R.J. Burnside—is north to south. It is therefore concluded that infiltrated rainfall is not tributary to the wetland.

3.8 LOW IMPACT DEVELOPMENT

Underground stormwater detention is proposed as part of the stormwater quantity control for the development. The detention system is designed to permit infiltration of stormwater in an effort to offset the increase in imperviousness over the site.

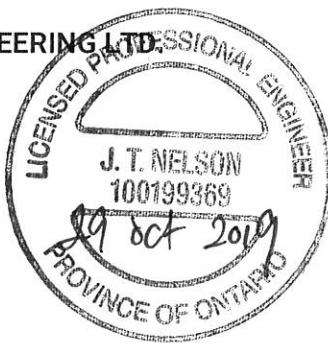
4.0 CONCLUSION

The servicing requirements as discussed in this report are summarized below in support of the proposed rezoning application.

1. Water servicing for both domestic and fire purposes will be from the existing 300 mm watermain on Airport Road and the 300 mm watermain on Walker Road West for the residential component. The commercial component is to be serviced from the 300 mm watermain on Airport Road. A 150 mm diameter main will be per Region standards to provide service to the proposed townhouse units. A 150 mm fire and 50 mm domestic service is proposed for the commercial component.
2. The commercial sanitary drainage will be directed to the existing 250 mm sanitary sewer along Airport Road by way of 150 mm service laterals.
3. The residential sanitary drainage will be directed to the existing 250 mm sanitary sewer on Walker Road West by way of 200 mm sanitary sewer via an easement over the proposed commercial development.
4. Storm drainage is to be controlled as closely as possible to the 2-100 year unit flow rates for sub-basin #6 determined using equation B provided by TRCA for the Humber River watershed; a 75 mm minimum diameter orifice control is proposed. A total of 305.9 m³ of storage is required for the residential development, and 180.7 m³ of storage is required for the commercial development; storage is underground in modular cellular storage. Approximately 375 m³ is provided for the residential component and 189 m³ for the commercial component.
5. There is no significant impact to the adjacent wetland as a result of redirecting flow from a small portion of the wetland catchment area toward Airport Road.

PREPARED BY TRAFALGAR ENGINEERING LTD.

J.T. Nelson, P.Eng.
Manager—Design Services



Appendix 'A'

Topographic Survey

Site Plan

SURVEYOR'S REAL PROPERTY REPORT
 PART 1, PLAN OF
**PART OF LOT 4, CONCESSION 6
 EAST OF HURONTOARIO STREET**
 (GEOGRAPHIC TOWNSHIP OF CALEDON, COUNTY OF PEEL)

TOWN OF CALEDON

REGIONAL MUNICIPALITY OF PEEL

David B. Searles Surveying Ltd.
 ONTARIO LAND SURVEYORS

METRIC
 DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN
 METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

LEGEND

DENOTES MONUMENT FOUND
DENOTES MONUMENT SET
DENOTES CROWN BAR
BAR
DENOTES WINCH
DENOTES THOMPSON OILS.
L.L. THOMPSON OILS.
DENOTES BURTON OILS.
L. BURTON OILS.
DENOTES L. BRISON, OILS, O.L.S.
L. BRISON, OILS, O.L.S.
DENOTES MACAY & PETERS, O.L.S.
M&P
DENOTES PLAN 43R-20393
PLAN 43R-21866
P1
DENOTES REGISTERED PLAN 43R-1481
R2
DENOTES REGISTERED PLAN 43R-1482
R3
DENOTES ANC
ANC
DENOTES BB
BB
DENOTES BC
BC
DENOTES CB
CB
DENOTES CCUT
CCUT
DENOTES CONCRETE PIPE
CPIPE
DENOTES CULVERT (STEEL PIPE)
CULVERT (STEEL PIPE)
DENOTES CURB STONE WALK
CURB STONE WALK
DENOTES DRAINAGE Ditch
DRAINAGE Ditch
DENOTES EDGE OF GRAVEL
EDGE OF GRAVEL
DENOTES EDGE OF PAVEMENT
EDGE OF PAVEMENT
DENOTES END OF ROAD
END OF ROAD
DENOTES FIRE HYDRANT
FIRE HYDRANT
DENOTES GRAVEL DRIVEWAY
GRAVEL DRIVEWAY
DENOTES GAS MEASURE MARKER
GAS MEASURE MARKER
DENOTES GATE
GATE
DENOTES GATE, FENCE
GATE, FENCE
DENOTES HEDGE LINE
HEDGE LINE
DENOTES HEADLAND
HEADLAND
DENOTES MANUFACTURE COVER (SANITARY)
MFG(SAN)
DENOTES MAINTENANCE HOLE COVER (STORM)
MAINTENANCE HOLE COVER (STORM)
DENOTES METER BOX (METER BOX)
METER BOX
DENOTES MINING WELL
MINING WELL
DENOTES MUDHOLE
MUDHOLE
DENOTES NAIL HOLE
NAIL HOLE
DENOTES NORTH LIMIT
NORTH LIMIT
DENOTES OUTLINE
OUTLINE
DENOTES OVERHEAD WIRES
OVERHEAD WIRES
DENOTES CONIFEROUS TREE
CONIFEROUS TREE
DENOTES DECIDUOUS TREE
DECIDUOUS TREE
DENOTES TREE LINE
TREE LINE
DENOTES TOP CASING
TOP CASING
DENOTES TOP PIPE
TOP PIPE
DENOTES TEST WELL
TEST WELL

BEARING NOTE

BEARINGS SHOWN IN GRID ARE GRID AND ARE REFERRED TO THE ELEVATIONS ON PLAN 43R-20293 (PIN: 43R-21866 (P2), 43R-1481 (P3), 43R-1482 (P4)).

BEARINGS SHOWN IN GRID ARE GRID AND ARE REFERRED TO THE ELEVATIONS ON PLAN 43R-20293 (PIN: 43R-21866 (P2), 43R-1481 (P3), 43R-1482 (P4)).

ELEVATIONS ARE REFERRED TO THE TOWN OF CALEDON BENCHMARK NO. 25, BEING ON THE NORTH FACE AT THE WEST CORNER OF A TWO STOREY BLOCK BUILDING #1630 LOCATED IN THE EAST SITE OF AIRPORT ROAD, 11.53M NORTH OF THE NORTHERN LIMIT OF CALEDON EAST, HAVING AN ELEVATION OF 510.16 METRES.

DISTANCE NOTE

DISTANCES SHOWN HEREIN ARE GROUND DISTANCES AND CAN BE CONVERTED TO GRID DISTANCES BY MULTIPLYING BY A COMBINED SCALE FACTOR OF 0.9986073.

BENCH MARK NOTE

ELEVATIONS ARE REFERRED TO THE TOWN OF CALEDON BENCHMARK NO. 25, BEING ON THE NORTH FACE AT THE WEST CORNER OF A TWO STOREY BLOCK BUILDING #1630 LOCATED IN THE EAST SITE OF AIRPORT ROAD, 11.53M NORTH OF THE NORTHERN LIMIT OF CALEDON EAST, HAVING AN ELEVATION OF 510.16 METRES.

NOTE

PROPERTY LIMITS ARE NOT FENCED UNLESS OTHERWISE NOTED ON THE FACE OF THE PLAN.

CAUTION

LOCATIONS OF ALL UTILITIES ARE APPROXIMATE. ALL UTILITIES SHOULD BE CONTACTED PRIOR TO ANY DIGGING OR CONSTRUCTION.

I CERTIFY THAT:
 1. THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEY'S ACT, THE SURVEYOR'S ACT, AND THE REGULATIONS MADE UNDER THEM.
 2. THE SURVEY WAS COMPLETED ON THE 22nd DAY OF SEPTEMBER, 2016.

SURVEYOR'S CERTIFICATE

THE REPRODUCTION, ALTERATION OR USE OF THIS PLAN, IN WHOLE OR IN PART, IS STRICTLY PROHIBITED.

PART 1
 DAVID B. SEARLES SURVEYING LTD.
 4225 Steeles Avenue East, Suite 100, Markham, Ontario, L3R 1V5
 Tel: (905) 273-6640 Fax: (905) 273-6440 Email: info@searles.ca
 Calculation File: 104-16CAL.DWG Drawing File: 104-0-16.DWG

PART 2
 DAVID B. SEARLES SURVEYING LTD.
 4225 Steeles Avenue East, Suite 100, Markham, Ontario, L3R 1V5
 Tel: (905) 273-6640 Fax: (905) 273-6440 Email: info@searles.ca
 Calculation File: 104-16CAL.DWG Drawing File: 104-0-16.DWG

PRELIMINARY

AUSTER SANKEY
 ONTARIO LAND SURVEYOR




Appendix 'B'

Grading Plan (G1)

Servicing Plan (S1)

Erosion and Sediment Control Plan (E1)



MH	N	NE	E	SE	S	SW	W	NW	TOP	DIA	OPSD	COVER	SANITARY SEWER DESIGN TABLE			
													RESIDENTIAL DEVELOPMENT	COMMERCIAL DEVELOPMENT	COMMENTS	
1A	288.49			288.23		288.20			1200	701.010	401.010					
2A	288.49			289.66					1200	701.010	401.010					
3A				289.05		289.15			292.82	1200	701.010	401.010				
4A	289.41			289.31					293.14	1200	701.010	401.010				
5A	289.49			289.66					294.37	1200	701.010	401.010				
6A				290.69		290.59			294.22	1200	701.010	401.010				
7A				290.89		290.79			294.31	1200	701.010	401.010				
8A						291.31			294.61	1200	701.010	401.010				
9A	290.33			290.31		290.43			292.68	1200	701.010	401.010				
10A				290.71					290.81	293.06	1200	701.010	401.010			
11A	290.15			290.13		290.52			292.25	1200	701.010	401.010				
12A				290.80		290.90			292.69	1200	701.010	401.010				

CATCH BASIN TABLE											
CB	INV	TOP	OPSD	COVER	COMMENTS	RESIDENTIAL DEVELOPMENT	COMMERCIAL DEVELOPMENT	COMMENTS	RESIDENTIAL DEVELOPMENT	COMMERCIAL DEVELOPMENT	COMMENTS
1	292.10	293.47	705.010	STD.503	BEDHIVE, STYLE TOP						
2	291.80	293.25	705.010	STD.503	BEDHIVE, STYLE TOP						
3	292.20	293.65	705.010	STD.503	BEDHIVE, STYLE TOP						
4	292.80	294.05	705.010	STD.503	BEDHIVE, STYLE TOP						
5	292.65	294.10	705.010	STD.503	BEDHIVE, STYLE TOP						
6	292.80	294.44	705.010	STD.503	BEDHIVE, STYLE TOP						
7	291.43	294.32	705.010	400.010							
8	292.80	294.35	705.010	400.010							
9	292.60	294.06	705.010	400.010							
10	292.60	294.06	705.010	400.010							
11	292.60	294.05	705.010	400.010							
12	292.20	293.65	705.010	400.010							
13	292.16	293.61	705.010	400.010							
14	291.84	293.29	705.010	400.010							
15	291.81	293.28	705.010	400.010							
16	291.55	293.00	705.010	400.010							
17	291.58	293.03	705.010	400.010							
18	291.34	292.36	705.010	400.010							
19	291.66	292.56	705.010	400.010							
20	291.67	292.57	705.010	400.010							
21	291.55	292.85	705.010	STD.503	BEDHIVE, STYLE TOP						

GENERAL NOTES

1. CONSTRUCTION FOR THIS PROJECT TO COMPLY WITH THE MOST CURRENT VERSION OF THE DEVELOPMENT STANDARDS, POLICIES AND GUIDELINES, PREPARED BY THE TOWN OF CALEDON INFRASTRUCTURE DEPARTMENT AND THE ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS.

2. ALL PROPOSED CONSTRUCTION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.

3. A MINIMUM OF FORTY-EIGHT (48) HOURS PRIOR TO COMMENCING CONSTRUCTION WITHIN THE MUNICIPAL RIGHT OF WAY THE CONTRACTOR MUST CONTACT THE FOLLOWING: THE TOWN OF CALEDON PUBLIC WORKS AND ENGINEERING DEPARTMENT 905-584-2272, THE REGION OF PEEL, ENBRIDGE CONSUMERS GAS, HYDRO ONE, BELL CANADA, ROGERS CABLE, FIRE AND EMERGENCY SERVICES

4. ALL DRAINAGE TO BE SELF-CONTAINED AND DISCHARGED TO A LOCATION APPROVED BY THE PUBLIC WORKS AND ENGINEERING DEPARTMENT AND CONSERVATION AUTHORITY PRIOR TO THE ISSUANCE OF A BUILDING PERMIT.

5. SEDIMENT CONTROL DEVICES ARE TO BE PROVIDED PRIOR TO ANY CONSTRUCTION ON THE SITE AND SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD TO THE SATISFACTION OF THE TOWN AND THE APPLICABLE CONSERVATION AUTHORITY.

6. A MINIMUM OF 1.2M CLEARANCE IS TO BE PROVIDED FROM LIMITS OF ALL SIDEWALKS AND DRIVEWAYS TO EXISTING UTILITY STRUCTURES WITHIN THE MUNICIPAL RIGHT OF WAY. IF THIS CLEARANCE IS NOT MAINTAINED THEY SHALL BE RELOCATED AT THE APPLICANT'S EXPENSE.

7. STREET CURBS ARE TO BE CONTINUOUS THROUGH ALL ENTRANCES TO THE SITE AND THE CURB SHALL BE TAPERED BACK 600mm. SIDEWALKS SHALL BE COMPLETELY REMOVED AND REPLACED BY 180mm MINIMUM CONCRETE THICKNESS, 30MPa AND 5 to 7% GRADE ANTRAMENT AT ALL PROPOSED INDUSTRIAL, COMMERCIAL AND INSTITUTIONAL ENTRANCES.

8. ANY CHANGES TO GRADES OR SERVICES FROM THE ORIGINAL APPROVED SITE PLAN MUST BE SUBMITTED BY THE ENGINEER TO THE TOWN FOR APPROVAL PRIOR TO CONSTRUCTION.

9. ALL BOULEVARDS TO BE RESTORED WITH 150mm MINIMUM OF TOPSOIL AND SOD TO THE SATISFACTION OF THE TOWN OF CALEDON PUBLIC WORKS AND ENGINEERING DEPARTMENT.

10. THE MINIMUM PAVEMENT FOR THE ASPHALT DRIVEWAY APROVN WITHIN THE MUNICIPAL ROAD ALLOWANCE SHALL BE AS FOLLOWS: 40MM HLS ASPHALT 50MM HL8 ASPHALT 150MM GRANULAR "A" 300MM GRANULAR "B" THE CONSULTANT SHOULD REVIEW THE ABOVE WITH RESPECT TO THE EXPECTED USAGE.

11. ALL DIMENSIONS AND ELEVATIONS TO BE VERIFIED PRIOR TO CONSTRUCTION AND ANY DISPARANCES FOUND PRIOR TO OR DURING CONSTRUCTION SHALL BE CLARIFIED WITH THE ENGINEER.

12. THE LOCATION OF ALL EXISTING UTILITIES AND SERVICES TO BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.

13. THIS PLAN SHOULD BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANT'S PLANS. ANY DISPARANCES SHALL BE CLARIFIED PRIOR TO CONSTRUCTION. INFORMATION RELATED TO DIMENSIONS FOR PRIVATE ROAD, PARKING AND SETBACKS SHALL BE TAKEN FROM THE SITE PLAN PREPARED BY THE ARCHITECT.

14. ALL SERVICES AND APPURTENANCE MATERIAL AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH THE LATEST TOWN OF CALEDON, REGIONAL MUNICIPALITY OF PEEL, ONTARIO PLUMBING CODE AND ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS.

15. O.P.S.D. REFERS TO ONTARIO PROVINCIAL STANDARD DRAWING.

16. ALL REMOVED OR DAMAGED CURBS, SIDEWALK AND SOD RESULTING FROM SERVICE INSTALLATION SHALL BE REINSTATED BY SERVICING CONTRACTOR TO THE TOWN OF CALEDON STANDARDS.

17. ALL SEWER TRENCH BACKFILL WITHIN SITE SHALL BE NATIVE MATERIAL FREE OF ORGANIC MATERIAL AND COMPAKED TO 95% SMID OR AS OTHERWISE DIRECTED BY THE GEOTECHNICAL ENGINEER.

WATERMAINS

1. ALL MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO THE CURRENT PEEL PUBLIC WORKS STANDARDS AND SPECIFICATIONS.

2. WATERMAINS OR PIPING SYSTEMS FOR MATERIALS 150mm (4") AND LARGER MUST BE AWN C-900-16 POLYVINYL CHLORIDE (PVC) DR16 INSTALLED WITH A 12 GAUGE TIN COATED COPPER LIGHT COATED PVC COATED TRACER WIRE BROUGHT TO THE SURFACE AT EACH VALVE BOX. 32mm (2") AND SMALLER MUST BE COPPER TYPE "K" SOFT COPPER TUBING.

3. WATERMAINS AND / OR WATER SERVICES ARE TO HAVE A MINIMUM LENGTH OF 1.7m (5') WITH A MINIMUM HORIZONTAL SPACING OF 1.2m (4') FROM THEMSELVES AND ALL OTHER UTILITIES.

4. PROVISIONS FOR FLUSHING WATER PRIOR TO TESTING, ETC. MUST BE PROVIDED WITH AT LEAST A 50mm (2") OUTLET ON 100mm (4") AND LARGER UNITS. CONNECT TO A FLUSHING POINT AT THE END, THE SAME SIZE AS THE LINE, THAT MUST ALSO BE MOVED OR PISSED TO ALLOW THE WATER TO DRAIN INTO A PARKING LOT OR DOWN A DRAIN. ON FIRE LINES, FLUSHING OUTLET TO BE 100mm (4") DIAMETER MINIMUM ON A HYDRANT.

5. ALL CURB STOPS TO 3.0m (10') OFF THE FACE OF THE BUILDING UNLESS OTHERWISE NOTED.

6. HYDRANT AND VALVE SET TO REGION STANDARD 1-6-1. DIMENSION A & B, 3.7m (2') AND 0.9m (3') AND TO HAVE PUMPER NOZZLE.

7. WATERMANS TO BE INSTALLED TO GRADES AS SHOWN ON THE APPROVED SITE PLAN. COPY OF GRADE SHEET MUST BE SUPPLIED TO INSPECTOR PRIOR TO COMMENCEMENT OF WORK, WHERE REQUESTED BY INSPECTOR.

8. WATERMANS MUST HAVE A MINIMUM VERTICAL CLEARANCE OF 0.30m (12") OVER / 0.50m (20") UNDER SEWERS AND ALL OTHER UTILITIES WHEN CROSSING.

9. ALL PROPOSED WATER PIPING MUST BE ISOLATED FROM EXISTING LINES IN ORDER TO ALLOW INDEPENDENT PRESSURE TESTING AND CHLORINATING FROM EXISTING SYSTEMS.

10. ALL OPERATIONS AND OPERATION OF REGION OF WATER VALVES SHALL BE ARRANGED THROUGH THE REGIONAL INSPECTOR ASSIGNED OR BY CONTACTING THE OPERATIONS AND MAINTENANCE DIVISION.

11. ALL PROPOSED WATER PIPING MUST BE ISOLATED THROUGH A TEMPORARY CONNECTION THAT SHALL INCLUDE AN APPROPRIATE CROSS-CONNECTION CONTROL DEVICE, CONSISTENT WITH THE DEGREE OF HAZARD, FOR BACKFLOW PREVENTION OF THE ACTIVE DISTRIBUTION SYSTEM, CONFORMING TO REGION OF PEEL STANDARDS 1-7-7 OR 1-7-8.

SANITARY SEWERS

1. ALL SANITARY SEWER BEDDING AS PER STD. 2-3.

2. MINLINE SANITARY SEWER PIPE SIZE SHALL BE MINIMUM 250mm IN DIAMETER INSTALLED AT THE APPROVED DESIGN GRADE. PIPE CLASS AND APPURTENANCES AS PER REGION'S SPECIFICATIONS.

3. ALL SEWERS CONSTRUCTED WITH GRADES 0.5% OR LESS SHALL BE APPROVED BY THE ENGINEER AND THE AGENCY PROJECT MANAGER OR DESIGNEE AND BE INSTALLED WITH LASER AND CHECKER PRIOR TO BACKFILL.

4. MINIMUM SANITARY SEWER PIPE SLOPE FOR LAST LEG SHALL BE 1% AND DESIRABLE SLOPE 2%.

5. ALL MANHOLES SHALL BE AS PER REGION STD. DWG. 2-5-1, 2-5-2, AND 2-5-3, WITH BENCHING AS PER STD 2-5-20. SEE NOTE 9.c. FOR IN-FLOW AND INfiltration.

6. FRAME AND COVERS SHALL BE AS PER REGION STD. DWG. 2-6-1.

7. MANHOLE STEPS SHALL BE AS PER REGION STD. DWG. 2-6-11.

8. MANHOLES DEEPER THAN 5.0m MUST BE EQUIPPED WITH SAFETY PLATFORMS, AS PER STD 2-6-13, 2-6-14, AND 2-6-15

9. SANITARY SERVICE LATERALS SHALL BE MINIMUM 150mm DIAMETER.

a. SANITARY SERVICE SHALL BE LOWER AND TO THE RIGHT OF THE STORM SERVICE AT THE PROPERTY LINE WHEN FACING THE LOT FROM THE STREET.

b. CONNECTIONS TO SEWERS SHALL BE MADE WITH MANUFACTURED TEES OR WYES WHERE APPLICABLE AND SHALL BE COLOUR CODED AS NON-WHITE, AS PER STD. DWG. 2-4-1, 2-4-2, AND 2-4-3.

c. ANY SANITARY CONNECTION OR STRUCTURE LEADING TO IN-FLOW AND INFILTRATION WILL HAVE TO BE ADDRESSED AND WILL BE ENFORCED IN THE FIELD BY A FIELD INSPECTOR.

STORM SEWERS

1. ALL STORM SEWERS 450mm DIAMETER AND LESS SHALL BE PVC DR-35 SEWER PIPE & FITTINGS IN ACCORDANCE WITH CSA B19.2.2 WITH BEDDING PER O.P.S.D. 802.010. STORM SEWER 525mm DIAM. AND LARGER SHALL BE REINFORCED CONCRETE PIPE 65-D, CSA A257.2 COMPLETE WITH BEDDING PER O.P.S.D. 802.030 (UNLESS OTHERWISE SHOWN).

2. STORM SEWERS 150mm DIAMETER AND LESS SHALL BE IN ACCORDANCE WITH G.P.S.C. 725.05. WITH 250mm DIAMETER (UNLESS OTHERWISE SHOWN) PVC LEAD AT 1.05 MIN. COVER AND PER O.P.S.D. 705.02 WITH 300mm DIAMETER (UNLESS OTHERWISE SHOWN). PVC LEAD AT 1.05 MIN. FRAME AND COVER PER O.P.S.D. 400.02 (UNLESS OTHERWISE SHOWN). ALL CB LEADS TO HAVE A MINIMUM COVER OF 1.2m AND A MAXIMUM COVER OF 1.8m BELOW THE SURFACE (UNLESS OTHERWISE SHOWN). SWINGING GATES SHALL BE PROVIDED IN ACCORDANCE WITH CITY OF MISSISSAUGA STD 2114.05.

3. STORM CONNECTIONS SHALL BE 150mm DIAMETER, WHITE COLOUR ONLY PVC PIPE DR-28 AS A MIN. GRADE OF 2.0X PER CITY STD. 2115.05.

4. MINIMUM CLEARANCE SHALL BE 150mm OR AS REQUIRED FOR BEDDING WHICHEVER IS GREATER.

5. CATCHBASINS AT ROAD SIDES SHALL HAVE 270mm GORETEX MATERIAL PLACED BETWEEN FRAME AND COVER TO CONTROL SEDIMENT. MAINTENANCE OF THE GORETEX SHALL BE ROUTINELY DONE TO ENSURE ADEQUATE DRAINAGE.

6. STORM PIPE WITH LESS THAN 1.2m COVER TO BE PROVIDED WITH INSULATION IN ACCORDANCE WITH OBC SECTION 7.3.5.4. APPENDIX A.

ADDITIONAL NOTES

1. LOCATION OF ALL EXISTING UTILITIES IN THE FIELD



Appendix 'C'

Estimated Domestic Water Demand

Estimated Sanitary Flow

TRAFAVGAR ENGINEERING LTD.

ESTIMATED SANITARY FLOW

Project: Shacca Caledon
Desc: Second submission Rezoning

Project No.: 1599
Prepared By: JN
Checked By: SP

Residential

<u>Land Use / Occupancy Type</u>	<u>Site Area (ha)</u>	<u>Population Density (pers/unit)</u>	<u>Eq. Population (cap.)</u>	<u>Per Cap. Demand (L/cap. Day)</u>	<u>Average Daily Dry Weather Flow (L/s)</u>
Proposed Townhouse Condominium	32	3.2	102	302.8	13.0

*SEE PEEL STD. 2-5-2

TOTAL	102	13.0
--------------	------------	-------------

Industrial / Commercial / Institutional

<u>Land Use / Occupancy Type</u>	<u>GFA</u>	<u>Population Density (pers/ha)</u>	<u>Eq. Population (cap.)</u>	<u>Per Cap. Demand (L/Ha. Day)</u>	<u>Average Daily Dry Weather Flow (L/s)</u>
					0.0
					0.0

TOTAL	0	0.0
--------------	----------	------------

Residential Peaking Factor: 1.00 *PEAKING IS INCLUDED IN REGION FLOW (STD 2-5-2)

ICI Peaking Factor: 4.50

Include ICI Peaking?

No

Site Area: 0.94 (ha)

Infiltration Allowance: 0.20 (L/s ha)

Manhole Allowance 2.24 (L/s)

Residential Average Flow: 15.3 (L/s)

ICI Average Flow: 0.0 (L/s)

Groundwater Discharge: 0.0 (L/s)

Total Average Flow: 15.3 (L/s)

Residential Peak Flow: 15.3 (L/s)

ICI Peak Flow: 0.0 (L/s)

Groundwater Discharge: 0.0 (L/s)

Total Peak Flow: 15.3 (L/s)

TRAFALGAR ENGINEERING LTD.

ESTIMATED SANITARY FLOW

Project: Shacca Caledon
Desc: Second submission Rezoning

Project No.: 1599
Prepared By: JN
Checked By: SP

Residential

Land Use / Occupancy Type	Site Area (ha)	Population Density (pers/ha)	Eq. Population (cap.)	Per Cap. Demand (L/cap. Day)	Average Daily Dry Weather Flow (L/s)
Proposed Commercial Development	0.557	50	28	*	13.0
*SEE PEEL STD. 2-5-2					
TOTAL			28		13.0

Industrial / Commercial / Institutional

Land Use / Occupancy Type	GFA	Population Density (pers/ha)	Eq. Population (cap.)	Per Cap. Demand (L/Ha. Day)	Average Daily Dry Weather Flow (L/s)
					0.0
					0.0
TOTAL			0		0.0

Residential Peaking Factor: 1.00 *PEAKING IS INCLUDED IN REGION FLOW (STD 2-5-2)

ICI Peaking Factor: 4.50

Include ICI Peaking?

No

Site Area: 0.56 (ha)

Infiltration Allowance: 0.20 (L/s ha)

Manhole Allowance: 1.12 (L/s)

Residential Average Flow: 13.7 (L/s)

ICI Average Flow: 0.0 (L/s)

Groundwater Discharge: 0.0 (L/s)

Total Average Flow: 13.7 (L/s)

Residential Peak Flow: 13.7 (L/s)

ICI Peak Flow: 0.0 (L/s)

Groundwater Discharge: 0.0 (L/s)

Total Peak Flow: 13.7 (L/s)

TRAFALGAR ENGINEERING LTD.

ESTIMATED DOMESTIC WATER DEMAND

Project: Shacca Caledon
Desc: Second submission Rezoning

Project No.: 1599
Prepared By: JN
Checked By: SP

Land Use / Occupancy Type	Units	Occupancy Data			Peaking Factors			Demand Flow			
		Population Density (pers/unit)	Eq. Population (cap.)	Per Cap. Demand (L/cap. Day)	Average Daily Demand (L/min)	Min. Hour	Peak Hour	Max. Daily	Min. Hour Demand (L/min)	Max. Hour Demand (L/min)	Max. Daily Demand (L/min)
Proposed Townhouse Condominium	32	3.2	102	280	20	1.00	3.00	2.00	20	60	40
									0	0	0
									0	0	0
									0	0	0
TOTAL			102		20				20	60	40

Fire Flow

Using Fire Underwriters Survey Methodology:

1. An estimate of the fire flow is given by the formula $F = 220C\sqrt{A}$

Where:

F = The required fire flow in litres per minute

C = Coefficient related to the type of construction

A = The total floor area in square metres (including all storeys but excluding basements at least 50% below grade)

Type of Construction: Ordinary

Coefficient: 1.00

Total Floor Area: 1200 (m²)

Average Daily Demand: 20 (L/min)

Minimum Hourly Demand: 20 (L/min)

Maximum Hourly Demand: 60 (L/min)

Maximum Daily Demand: 40 (L/min)

Max. Daily Plus Fire: 9040 (L/min)

$F = 8000$ (L/min)

Adequately Protected Vertical Openings: No

Area Note: For fire resistive buildings, consider the two largest adjoining floors plus 50% of the remaining floors up to eight, when openings are inadequately protected. For adequately protected vertical openings consider only the area of the largest floor plus 25% of each of the two immediately adjoining floors

2. Adjust the value in No. 1 for occupancy surcharge/reduction

Occupancy Contents: Limited Combustible

Factor: -15%

$F = 6800$ (L/min)

3. Adjust the value in No. 2 for sprinkler

NFPA 13 Sprinkler: No

Reduction: 0%

Standard Water Supply: Yes

Reduction: 0%

Fully Supervised: No

Reduction: 0%

4. Adjust the value in No. 2 for exposure

Separation (m)	Charge
North	45
East	3
South	18
West	N/A

Total Charge: 35%
Exposure Charge: 2380 (L/min)

5. Estimated Fire Flow is value in No. 2 less Sprinkler Reduction plus Exposure Charge, rounded to the nearest 1000

$F = 9000$ (L/min)

TRAFalgar ENGINEERING LTD.

ESTIMATED DOMESTIC WATER DEMAND

Project: Shacca Caledon
Desc: Second submission Rezoning

Project No.: 1599
Prepared By: JN
Checked By: SP

Land Use / Occupancy Type	Occupancy Data				Peaking Factors			Demand Flow			
	Site Area (ha)	Population Density (pers/ha)	Eq. Population (cap.)	Per Cap. Demand (L/cap. Day)	Average Daily Demand (L/min)	Min. Hour	Peak Hour	Max. Daily	Min. Hour Demand (L/min)	Max. Hour Demand (L/min)	Max. Daily Demand (L/min)
Proposed Commercial Development	0.557	50	28	280	5	1.00	3.00	1.40	5	16	8
									0	0	0
									0	0	0
									0	0	0
TOTAL			28		5				5	16	8

Fire Flow

Using Fire Underwriters Survey Methodology:

1. An estimate of the fire flow is given by the formula $F = 220C\sqrt{A}$

Where:

F = The required fire flow in litres per minute

C = Coefficient related to the type of construction

A = The total floor area in square metres (including all storeys but excluding basements at least 50% below grade)

Type of Construction: Ordinary

Coefficient: 1.00

Total Floor Area: 905 (m²)

Average Daily Demand: 5 (L/min)

Minimum Hourly Demand: 5 (L/min)

Maximum Hourly Demand: 16 (L/min)

Maximum Daily Demand: 8 (L/min)

Max. Daily Plus Fire: 8008 (L/min)

$$F = 7000 \text{ (L/min)}$$

Adequately Protected Vertical Openings: No

Area Note: For fire resistive buildings, consider the two largest adjoining floors plus 50% of the remaining floors up to eight, when openings are inadequately protected. For adequately protected vertical openings consider only the area of the largest floor plus 25% of each of the two immediately adjoining floors

2. Adjust the value in No. 1 for occupancy surcharge/reduction

Occupancy Contents: Combustible

Factor: 0%

$$F = 7000 \text{ (L/min)}$$

3. Adjust the value in No. 2 for sprinkler

NFPA 13 Sprinkler: No

Reduction: 0%

Standard Water Supply: Yes

Reduction: 0%

Fully Supervised: No

Reduction: 0%

Total Reduction: 0%
 Sprinkler Reduction: 0 (L/min)

4. Adjust the value in No. 2 for exposure

	Separation (m)	Charge
North	35	5%
East	N/A	
South	N/A	
West	35	5%
Total Charge:		10%
Exposure Charge:		700 (L/min)

5. Estimated Fire Flow is value in No. 2 less Sprinkler Reduction plus Exposure Charge, rounded to the nearest 1000

$$F = 8000 \text{ (L/min)}$$

Appendix 'D'

TRCA SWM Requirements (Email)

Pre-Development Drainage Area Plan (Figure 1)

Post-Development Drainage Area Plan (Figure 2)

Storm Sewer Design Sheet

HydroCAD Output

Oil/Grit Separator Sizing

James Nelson

From: Anant Patel <APatel@trca.on.ca>
Sent: June-22-16 10:16 AM
To: Stephen Potter
Subject: 16114 Airport Road, Caledon

Follow Up Flag: Follow up
Flag Status: Flagged

Hi Steve,

As discussed this morning, please see below TRCA's Stormwater Management requirements.

The stormwater management requirements for the proposed development at 16114 Airport Road, Caledon is:

- **Quantity Control:** TRCA's quantity control criteria for the site is that post development peak flows to be controlled to pre-development peak flows for 2 - 100 year storms. The pre-development peak flows for 2 - 100 year storms should be established based on the unit flow rates for Sub-basin # 6 (Equation B) and typically with drainage areas under existing conditions. The unit flow relationships for the Humber River Watershed are provided in the TRCA Stormwater Management Criteria document which can be found in the following link: <http://www.trca.on.ca/dotAsset/26183.pdf>. Please use 6 hr AES and 12 hr AES to estimate storage requirements for the post-development flows to achieve the targets and then select the option that require greater storage requirements.
- **Quality Control:** Level 1/Enhanced protection such that 80% TSS removal is achieved.
- **Erosion Control:** due to the size of the site, an erosion assessment is required to identify the necessary controls required to mitigate any potential downstream erosion issues. However, the minimum control required is to detain runoff from a 25 mm storm for at least 48 hours.
- **Water Balance:** As the site is located on the Oak Ridges Moraine, post-development infiltration should be maintained to pre-development infiltration by implementing mitigation measures to aid in the infiltration.
- **Feature Based Water Balance Analysis:** Given that the Ministry of Natural Resources and Forestry (MNRF) has identified the wetland to the rear of the property as a Provincially Significant Wetland (PSW), a feature based water balance will be required in support of the application. Scoping of the FBWB will depend on the conceptual alterations to the wetland catchment area as well as the sensitivity of the wetland complex. Please refer the applicant to Appendix D of the SWM Criteria Document for further guidance.

TRCA staff recommend that you also refer to TRCA Low Impact Development Stormwater Management Planning & Design Guide, which can be found in the following link: https://trca.ca/wp-content/uploads/2016/04/LID-SWM-Guide-v1.0_2010_1_no-appendices.pdf.

Should you have any further questions, please do not hesitate to contact me.

Thank you,
Anant

Anant Patel, Acting Planner II | Planning and Development |
Toronto and Region Conservation Authority|
P: 416.661.6600 x5618 | F: 416.661.6898 | apatel@trca.on.ca | www.trca.on.ca |
Head Office location & courier address: 101 Exchange Avenue, Vaughan, ON, L4K 5R6 |

LEGEND



PROPOSED OVERLAND FLOW DIRECTION

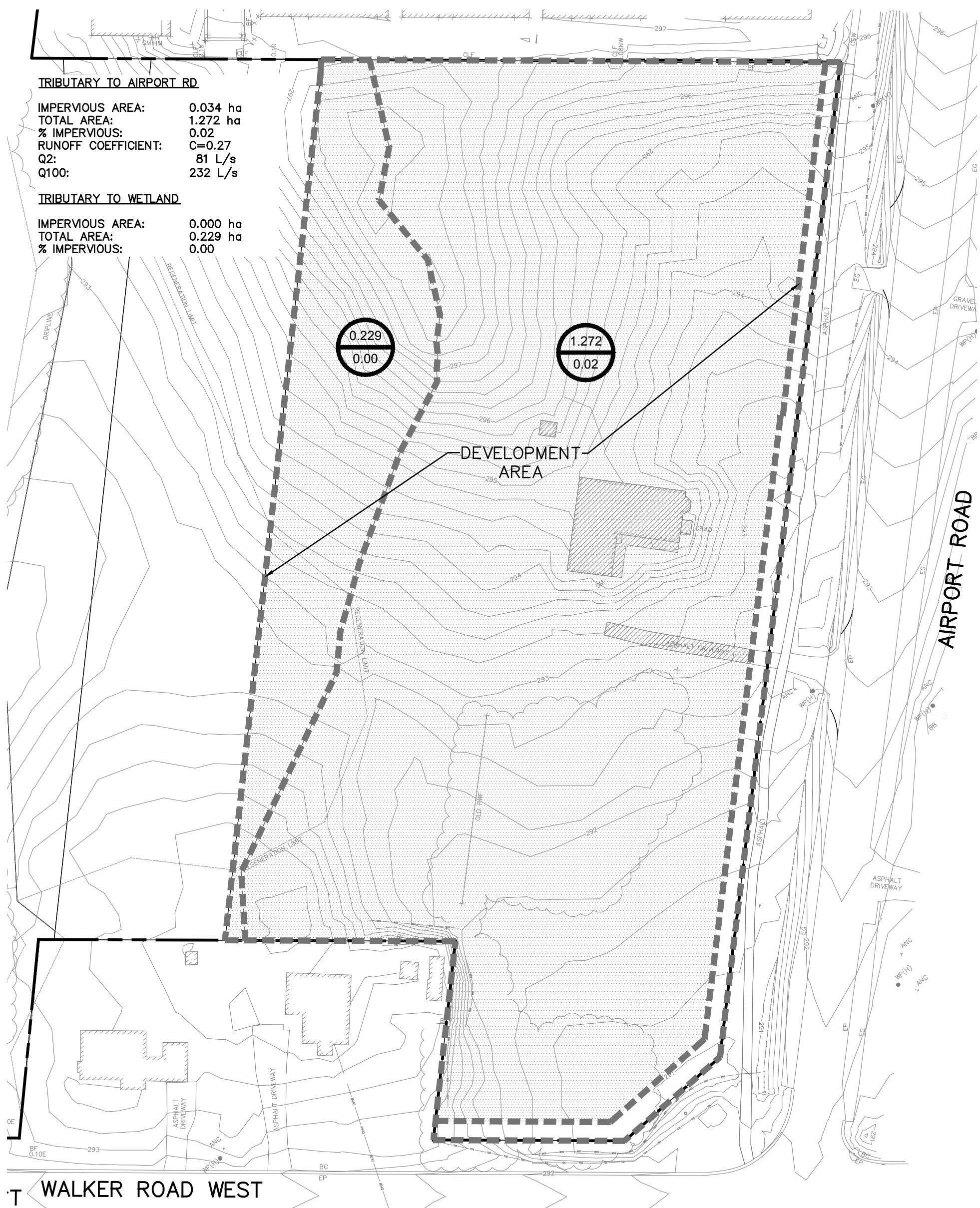
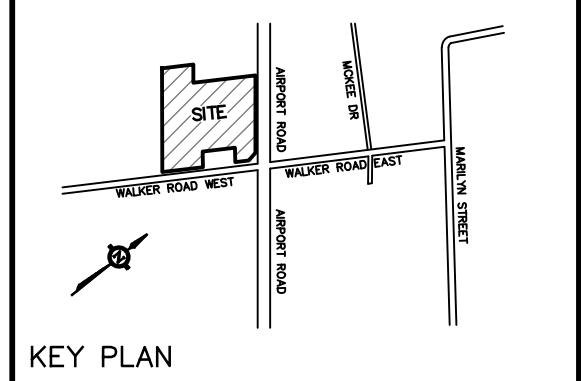


PROPOSED DRAINAGE AREA



DRAINAGE AREA (ha)

IMPERVIOUSNESS



PROJECT TITLE

16114 AIRPORT ROAD
TOWN OF CALEDON

TRAFalgar ENGINEERING LTD.
TEL: (905) 338-3366 FAX: (905) 338-7734
tel@trafalgareng.com

DRAWING TITLE

PRE-DEVELOPMENT DRAINAGE AREA PLAN

DESIGN BY

JN/SP

SCALE

1:1

DRAWING NO.

FIGURE 1

DRAWN BY

JN

DATE

06/03/2017

CAD FILE:

1599S.DWG

PLOT SCALE:

1:1

PLOT DATE: 06/02/2018

LEGEND



PROPOSED OVERLAND FLOW DIRECTION

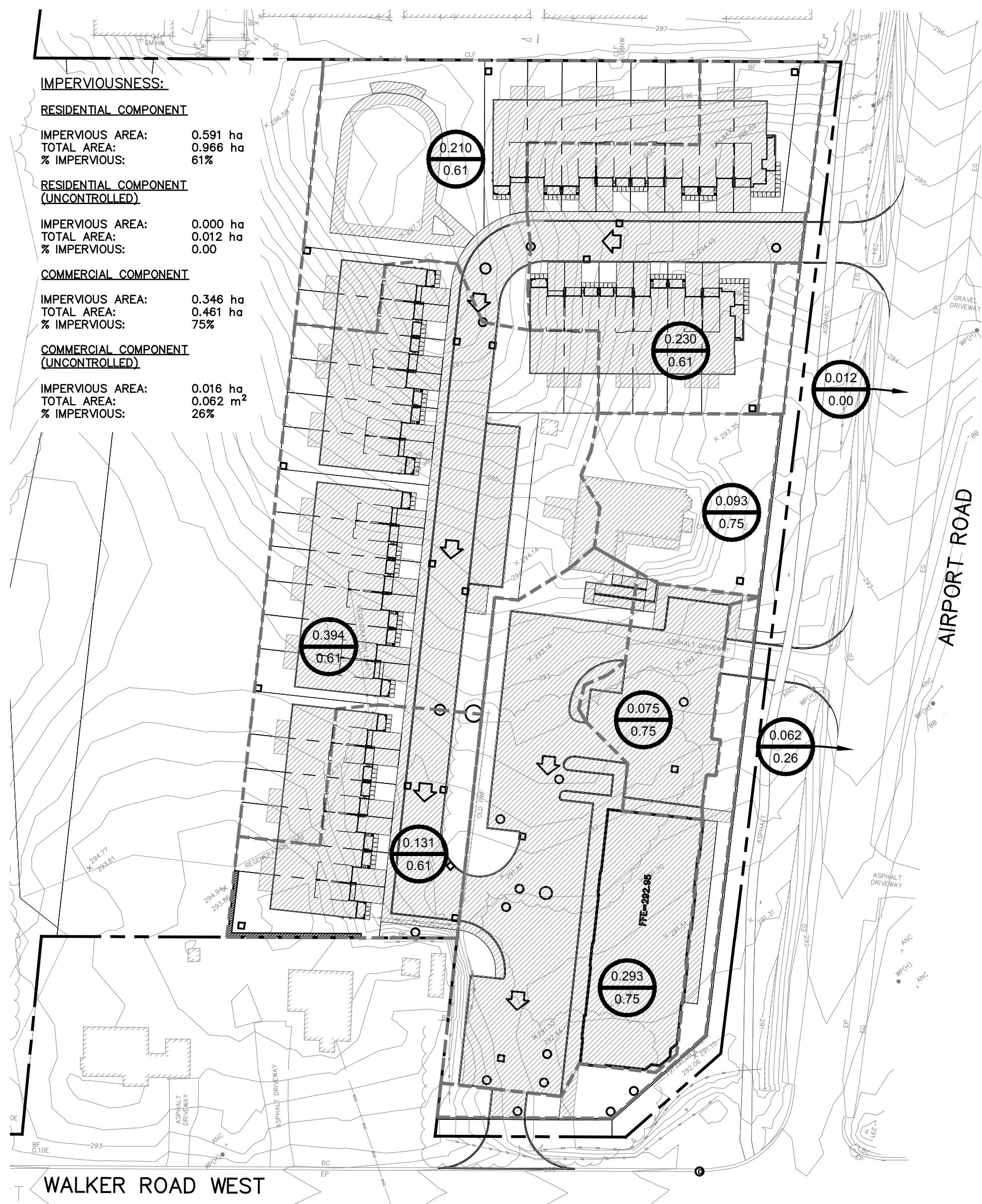
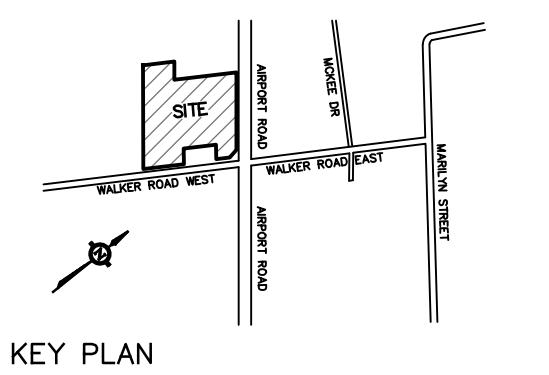


PROPOSED DRAINAGE AREA



DRAINAGE AREA (ha)

IMPERVIOUSNESS



PROJECT TITLE

16114 AIRPORT ROAD
TOWN OF CALEDON

TRAfalgar Engineering Ltd.
TEL: (905) 338-3366 FAX: (905) 338-7734
tel@trafalgareng.com

DRAWING TITLE

POST-DEVELOPMENT DRAINAGE AREA PLAN

DESIGN BY

JN/SP

SCALE

1:1

DRAWING NO.

FIGURE 2

DRAWN BY

JN

DATE

06/03/2017

CAD FILE:

1599S.DWG

PLOT SCALE:

1:1

PLOT DATE: 06/02/2018

TRAFalgar Engineering Ltd.
Consulting Engineers

Project Name : 16114 Airport Road
Project No. : 1599
Date : 06-Sep-19

STORM SEWER DESIGN SHEET

100-year Storm

Town of Caledon

$$\text{Intensity} = A / (T_d + B)^C$$

A= 4688

B= 17

C= 0.96

Area	LOCATION	FROM	TO	DRAINAGE AREA				RUNOFF			PROPOSED SEWER						Fraction Full	
				Area	Runoff Coeff.	A x R	Accum. A x R	T of C	Intensity	Expected Flow	Length	Gradient	Pipe Size	Manning's Coeff.	Capacity	Velocity	Time of Flow	
No.		MH	MH	(ha)	('c')	(ha x 'c')	(ha x 'c')	(min)	(mm/hr)	(m³/s)	(m)	(%)	(mm)	(n)	(m³/s)	(m/s)	(min)	(%)
	Residential Development																	
1		13	12	0.230	0.76	0.176	0.176	10.00	198.10	0.097	43.50	0.50	375	0.013	0.130	1.14	0.64	75
			12	11			0.176	10.64	193.71	0.095	9.00	0.50	375	0.013	0.130	1.14	0.13	73
2		11	10	0.210	0.76	0.160	0.336	10.77	192.83	0.180	9.50	0.50	450	0.013	0.211	1.28	0.12	85
3		10	8	0.394	0.76	0.301	0.637	10.89	192.01	0.340	69.00	0.60	525	0.013	0.348	1.56	0.74	97
4		9	8	0.131	0.76	0.100	0.100	10.00	198.10	0.055	39.50	0.50	300	0.013	0.072	0.98	0.67	77
		8	7			0.736	11.63	187.26	0.383	4.50	1.00	525	0.013	0.450	2.01	0.04	85	
		7	STOR			0.736	11.67	187.02	0.383	3.00	1.00	525	0.013	0.450	2.01	0.02	85	
		STOR	6			0.736	11.69	186.87	0.382	1.50	2.00	525	0.013	0.636	2.85	0.01	60	
		6	5						0.011	15.50	0.50	200	0.013	0.024	0.75	0.35	45	
		5	4						0.011	31.00	0.50	200	0.013	0.024	0.75	0.69	45	
		4	3						0.011	7.50	0.50	200	0.013	0.024	0.75	0.17	45	
		3	2						0.011	16.50	0.50	200	0.013	0.024	0.75	0.37	45	
		2	1						0.019	19.00	0.50	250	0.013	0.044	0.87	0.36	43	
		1									250							
	Commercial Development																	
5		CB8	20	0.093	0.94	0.087	0.087	10.00	198.10	0.048	43.00	1.00	250	0.013	0.062	1.23	0.58	77
6		20	19	0.075	0.94	0.070	0.158	10.58	194.07	0.085	26.00	0.50	375	0.013	0.130	1.14	0.38	66
		19	17			0.158	10.96	191.53	0.084	20.50	0.50	375	0.013	0.130	1.14	0.30	65	
7		18	17	0.293	0.94	0.274	0.274	10.00	198.10	0.151	5.00	0.50	450	0.013	0.211	1.28	0.06	72
		17	STORC			0.432	11.27	189.58	0.227	1.00	2.00	450	0.013	0.422	2.57	0.01	54	
		STORC	16			0.432	11.27	189.54	0.227	1.50	2.00	450	0.013	0.422	2.57	0.01	54	
		16	15						0.007	5.00	0.50	200	0.013	0.024	0.75	0.11	29	
		15	14						0.007	16.00	0.50	200	0.013	0.024	0.75	0.36	29	
		14	1						0.007	7.50	0.50	250	0.013	0.044	0.87	0.14	16	

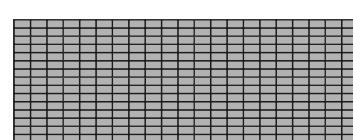
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Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrickx 1 (ACO StormBrickx®)
Inlet= 603 mmW x 67 mmH => 0.350 m³ / 1.2 ml = 0.44 m³
Outside= 603 mmW x 62 mmH => 0.369 m³ / 1.2 ml = 0.44 m³

14 Chambers/Row x 21 m Long x 1.62 m Row Length
8 Rows x 603mm Wide x 4.82 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m³ = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m³ = 49.76 m³ Displacement
Chamber Storage = 47.20 m³ 0.647 Mi
Overall System Efficiency = 95.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m



T\TRCA AES 12-hr 2-Year Rainfall=42 mm
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Summary for Pond RS: Residential Storage

Inflow Area = 0.9656 ha, 61.1259 Impervious, Inflow Depth = 0.125 m for 2-Year event
Inflow = 0.0240 m³s @ 5.03 hrs, Volume= 0.125 Mi, Atene= 82%, Lag= 123.6 mm
Outflow = 0.0344 m³s @ 7.08 hrs, Volume= 0.169 Mi
Routing by Slope Ind method, Time step= 0.06480 Mi, dt= 0.05 hrs, 2 / 2 Plugs/Poly detention time= 256.4 min calculated for 0.039 Mi (87% of inflow)
Center of Mass set, time= 222.4 min (384.3 hrs) 2.38 m³/min
Volume = 104.5 m³, Invert = 120.0 m³ Average Storage Description
ACO StormBrickx 2 x 144 Inside #1
Inside= 603 mmW x 1.224 mmH => 0.701 m³ / 1.21 ml = 0.84 m³
Outside= 603 mmW x 1.226 mmH => 0.737 m³ / 1.21 ml = 0.89 m³

9.04 m³ NW x 24.0 m L x 1.22 m H Field B
266.6 m³ Overall - 266.6 m³ Embedded - 0.0 m³ x 0.0% Voids
Inside= 603 mmW x 1.224 mmH => 0.701 m³ / 1.21 ml = 0.84 m³
Outside= 603 mmW x 1.226 mmH => 0.737 m³ / 1.21 ml = 0.89 m³

#2A 291.000 m 121.6 m³ Total Available Storage
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

Device Routing Invert Outlet Devices
#1 Primary 291.056 m 75 mm Vert. OfficelGrade Cc = 0.600
Primary Outflow Max=0.044 m³s @ 7.09 hrs /29.226 m (Free Discharge)

#2-1 Outflow/Grade (Orifice Controls) 0.0044 m³s @ 0.99 m/s

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Pond CS: Commercial Storage - Chamber Wizard Field B

Chamber Model = ACO StormBrickx 1 (ACO StormBrickx®)
Inlet= 603 mmW x 67 mmH => 0.350 m³ / 1.2 ml = 0.44 m³
Outside= 603 mmW x 62 mmH => 0.369 m³ / 1.2 ml = 0.44 m³

14 Chambers/Row x 21 m Long x 1.62 m Row Length
8 Rows x 603mm Wide x 4.82 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m³ = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m³ = 49.76 m³ Displacement
Chamber Storage = 47.20 m³ 0.647 Mi
Overall System Efficiency = 95.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

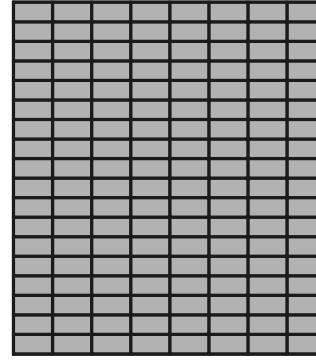


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Pond RS: Residential Storage - Chamber Wizard Field B

Chamber Model = ACO StormBrickx 2 (ACO StormBrickx®)
Inlet= 603 mmW x 1.224 mmH => 0.701 m³ / 1.21 ml = 0.84 m³
Outside= 603 mmW x 1.226 mmH => 0.737 m³ / 1.21 ml = 0.89 m³

20 Chambers/Row x 12 m Long x 24.10 m Row Length
15 Rows x 603 mm Wide x 9.04 m Base Width
1.224 mm Chamber Height = 1.22 m Field Height
300 Chambers x 0.84 m³ = 252.26 m³ Chamber Storage
300 Chambers x 0.86 m³ = 253.59 m³ Displacement
Chamber Storage = 253.26 m³ = 0.253 Mi
Overall Storage Efficiency = 96.0%
Overall System Size = 24.10 m x 9.04 x 1.22 m



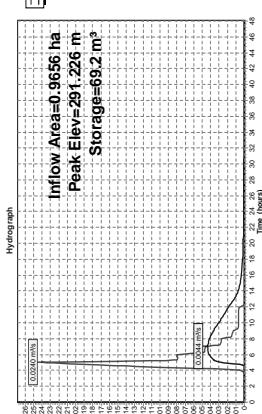
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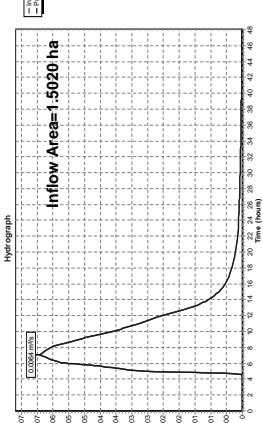
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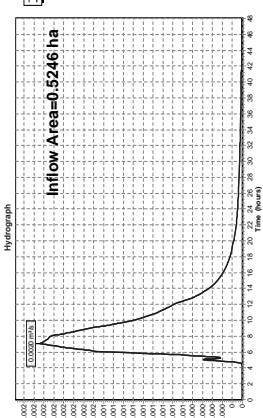
Pond RS: Residential Storage



Link 21: Development Total



Link 21: Development Total



Summary for Link 21: Development Total

Inflow Area = 1.5020 ha, 63.48% Impervious, Inflow Depth > 10 mm for 2-Year event
Inflow = 0.0353 m³s @ 0.10 hrs, Volume= 0.152 Mi, Atten= 0%, Lag= 0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 21: Development Total

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Link 41: Residential Total



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Summary for Link 41: Residential Total

Inflow Area = 0.9774 ha, 60.19% Impervious, Inflow Depth > 11mm for 2-Year event
Inflow = 0.0344 m³s @ 7.08 hrs, Volume= 0.109 Mi, Atten= 0% min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 41: Residential Total

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Summary for Link 31: Commercial Total

Inflow Area = 0.5246 ha, 68.8% Impervious, Inflow Depth > 8 mm for 2-Year event
Inflow = 0.0320 m³s @ 7.10 hrs, Volume= 0.083 Mi, Atten= 0%, Lag= 0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link 31: Commercial Total

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Summary for Subcatchment 1R: Residential Lands

Runoff = 0.0386 m³s @ 5.03 hrs, Volume= 0.028 Mi, Depth= 22 mm
TRCA AES 12-hr 5-Year Rainfall=54 mm
Area = 0.152 Mi, Description= Paved parking, HSG D
3.742.0 98 Paved parking, HSG D
3.742.0 81 27.5% Grass Cover, Good, HSG B
9.656.0 84 Weighted Average
3.742.0 61 38.75% Pavement Area
5.914.0 98 61.25% Impervious Area

Subcatchment 1R: Residential Lands

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Summary for Subcatchment 1C: Commercial Lands

Runoff = 0.0233 m³s @ 5.01 hrs, Volume= 0.013 Mi, Depth= 20 mm
TRCA AES 12-hr 5-Year Rainfall=54 mm
Area = 0.032 Mi, Description= Paved parking, HSG D
3.467.0 98 Paved parking, HSG D
1.168.0 61 27.5% Grass Cover, Good, HSG B
4.626.0 88 Weighted Average
3.168.0 61 25.27% Pavement Area
3.457.0 98 74.73% Impervious Area

Subcatchment 1C: Commercial Lands

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Summary for Subcatchment 1B: Residential Lands

Runoff = 0.0233 m³s @ 5.01 hrs, Volume= 0.013 Mi, Depth= 22 mm
TRCA AES 12-hr 5-Year Rainfall=54 mm
Area = 0.032 Mi, Description= Paved parking, HSG D
3.467.0 98 Paved parking, HSG D
1.168.0 61 27.5% Grass Cover, Good, HSG B
4.626.0 88 Weighted Average
3.168.0 61 25.27% Pavement Area
3.457.0 98 74.73% Impervious Area

Subcatchment 1B: Residential Lands

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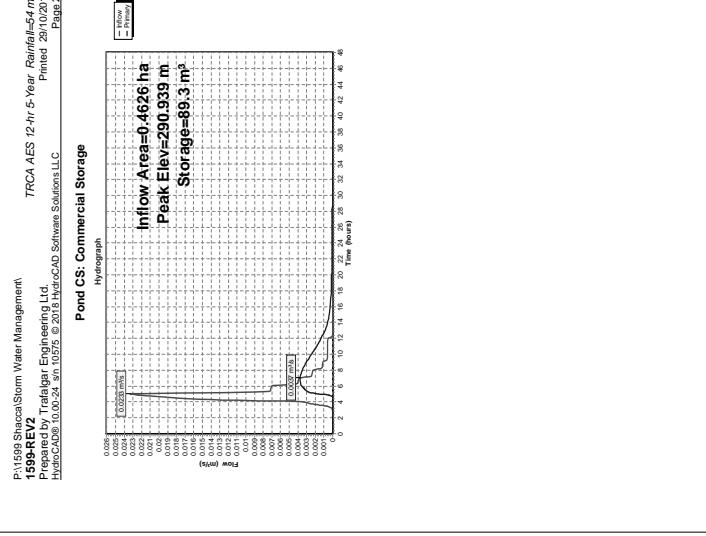
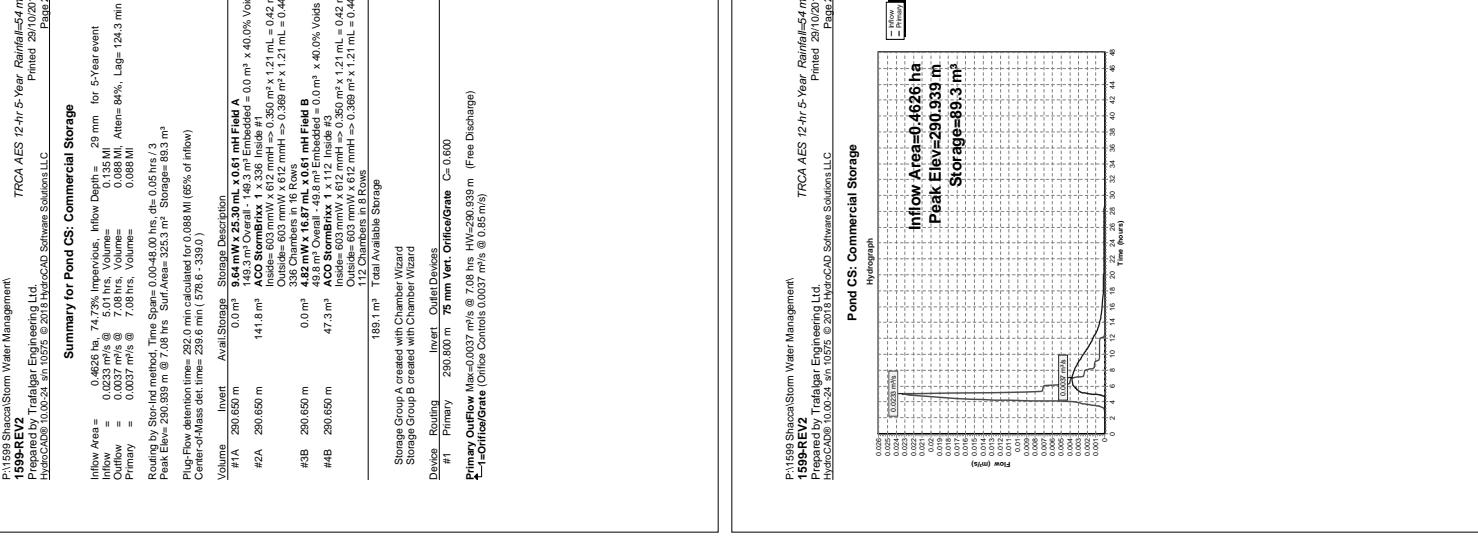
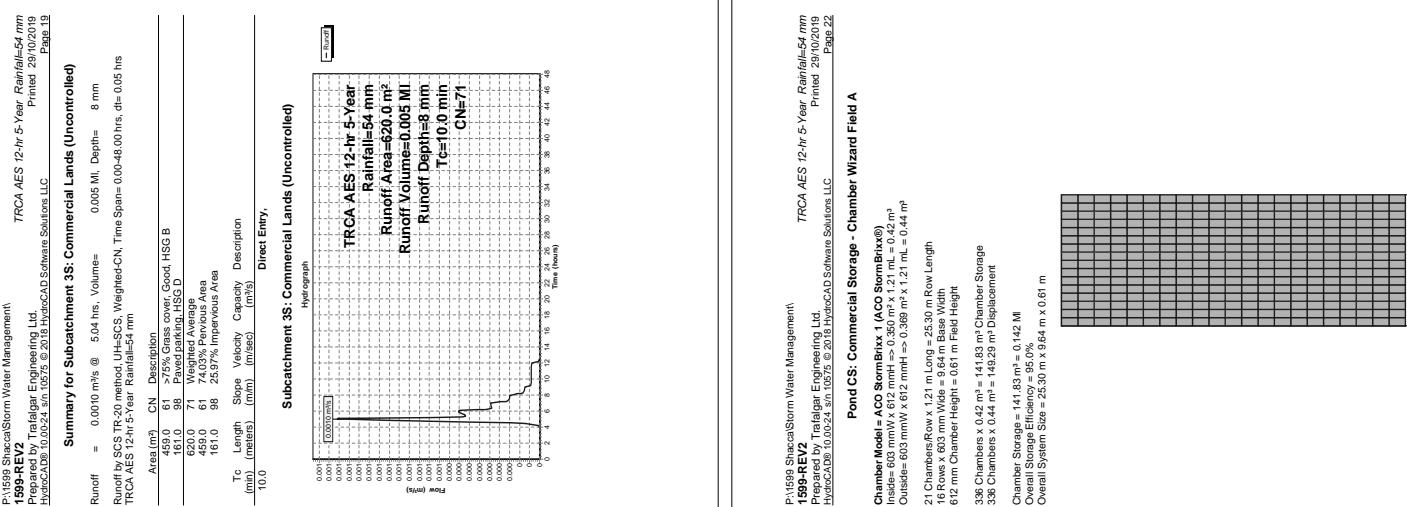
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Summary for Subcatchment 1B: Residential Lands

Runoff = 0.0233 m³s @ 5.01 hrs, Volume= 0.013 Mi, Depth= 22 mm
TRCA AES 12-hr 5-Year Rainfall=54 mm
Area = 0.032 Mi, Description= Paved parking, HSG D
3.467.0 98 Paved parking, HSG D
1.168.0 61 27.5% Grass Cover, Good, HSG B
4.626.0 88 Weighted Average
3.168.0 61 25.27% Pavement Area
3.457.0 98 74.73% Impervious Area

Subcatchment 1B: Residential Lands

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Summary for Pond RS: Residential Storage

Inflow Area = 0.03856 ha, Rainfall=54 mm
Outflow = 0.03856 m³ @ 0.122 m/s, Volume= 7.035 m³, Atten= 33%, Lag= 0.122 MI
Primary = 0.0065 m³ @ 7.035 hrs, Volume= 0.0065 m³

Routing by Sto-Ind method, Time Span= 0.00-48.00 hrs, cte= 0.05 hrs / 2

Peak Elv@ 18.393 m @ 0.08 hrs, Surf.Area= 322.3 m²
PluF-Flow retention time= 265.8 min calculated for 0.192 MI Chamber Storage
Center-of-Mass det. time= 246.4 min (397.0 - 360.7)

Volume, Invert, Avail.Storage, Storage Description

#A	291.000 m	0.0 m	1085 mm x 934 m ² x 122 mm Field A
#2A	291.000 m	121.6 m ³	2 x 144 Inside ft Inside= 603 mm/W x 1224 mm/H => 0.701 m ² x 121 ml = 0.84 m ³ Outside= 603 mm/W x 1224 mm/H => 0.737 m ² x 121 ml = 0.89 m ³
#3B	291.000 m	0.0 m ³	144 Chambers in 18 Rows 35.6 m ² x 4.0 m ³ = 144.0 m ³
#4B	291.000 m	253.3 m ³	2 x 61.6 Chambers 2 x 30 Inside ft Inside= 603 mm/W x 1224 mm/H => 0.701 m ² x 121 ml = 0.84 m ³ Outside= 603 mm/W x 1224 mm/H => 0.737 m ² x 121 ml = 0.89 m ³

300 Chambers in 15 Rows
374.8 m² Total Available Storage

Storage Group A created with Chamber Wizard

Storage Group B created with Chamber Wizard

Device Routing Invert, Outlet Devices

#1 Primary 291.050 m 75 mm Vert. Orifice/Grate C= 0.600

↓-1 Orifice/Rate (Orifice Controls 0.0005 m³/s @ 1.47 m/s)

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

Peak Elv=291.393 m

Storage=120.3 m³

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Pond RS: Residential Storage

Inflow Area = 150201.0a, Rainfall=54 mm
Outflow = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 m³
Primary = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 m³

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

Peak Elv=291.393 m

Storage=120.3 m³

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Summary for Pond RS: Residential Storage

Inflow Area = 150201.0a, Rainfall=54 mm
Outflow = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 m³
Primary = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 m³

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

Peak Elv=291.393 m

Storage=120.3 m³

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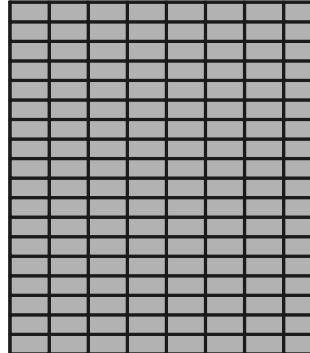
Pond RS: Residential Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrix® 2 (ACO StormBrix®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 ml = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 ml = 0.89 m³

8 Chambers/Row x 12.1 m Long = 24.10 m Row Length
18 Rows x 603 mm/Surf.Widt = 9.04 m Base Width
1.224 mm Chamber Height = 1.22 m Field Height

144 Chambers x 0.84 m² = 121.57 m² Chamber Storage
144 Chambers x 0.88 m² = 127.96 m² Displacement

Chamber Storage = 253.26 ml = 253.26 m³
Overall System Efficiency = 95%
Overall System Size = 24.10 m x 9.04 m x 1.22 m



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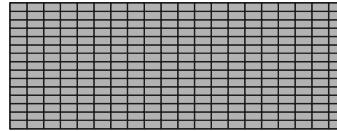
Pond RS: Residential Storage - Chamber Wizard Field B

Chamber Model = ACO StormBrix® 2 (ACO StormBrix®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 ml = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 ml = 0.89 m³

18 Chambers/Row x 12.1 m Long = 24.10 m Row Length
18 Rows x 603 mm/Surf.Widt = 9.04 m Base Width
1.224 mm Chamber Height = 1.22 m Field Height

300 Chambers x 0.84 m² = 253.26 m² Chamber Storage
300 Chambers x 0.88 m² = 265.59 m² Displacement

Chamber Storage = 253.26 ml = 253.26 m³
Overall System Efficiency = 95%
Overall System Size = 24.10 m x 9.04 m x 1.22 m



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TRCA AES 12-hr 5-Year Rainfall=54 mm

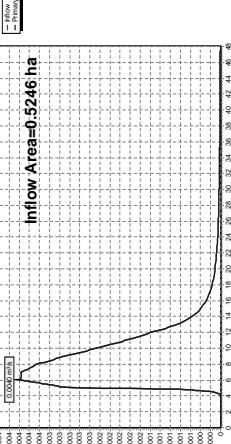
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TRCA AES 12-hr 5-Year Rainfall=54 mm
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Summary for Link 3L: Commercial Total

Inflow Area = 0.5246 ha, Rainfall=54 mm
Inflow = 0.0040 m³ @ 6.07 hrs, Volume= 0.003 MI
Primary = 0.0040 m³ @ 6.07 hrs, Volume= 0.003 MI, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

Link 3L: Commercial Total

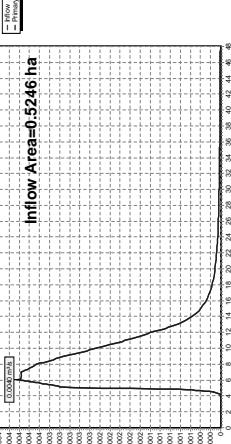


TRCA AES 12-hr 5-Year Rainfall=54 mm
1599-REV2
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Summary for Link 2L: Development Total

Inflow Area = 150201.0a, Rainfall=54 mm
Inflow = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 MI
Primary = 0.0105 m³ @ 7.033 hrs, Volume= 0.0105 MI, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

Link 2L: Development Total

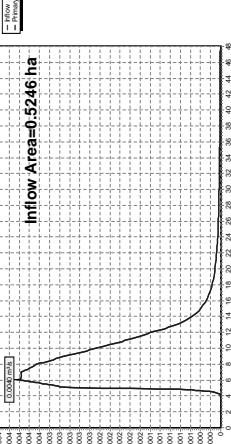


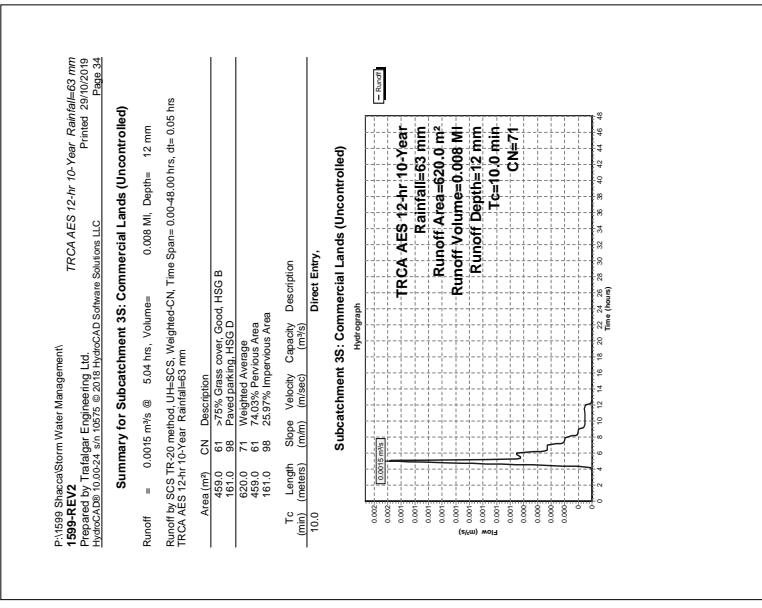
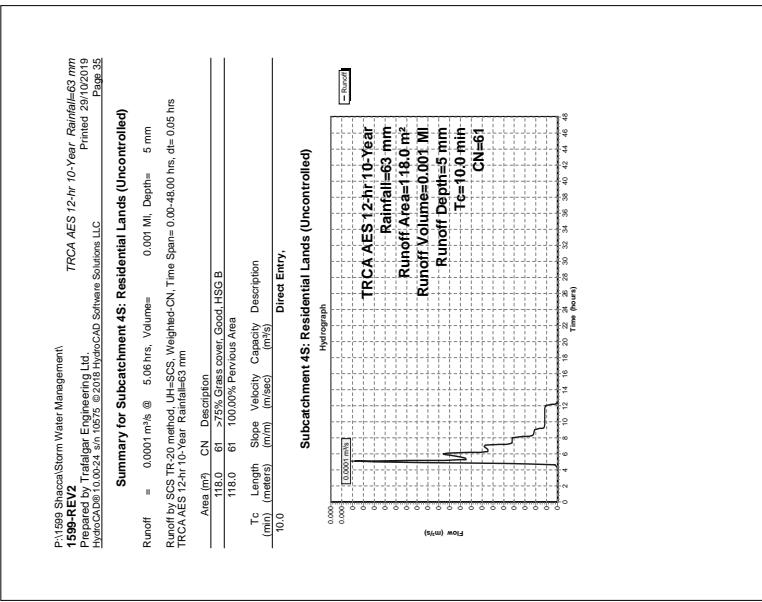
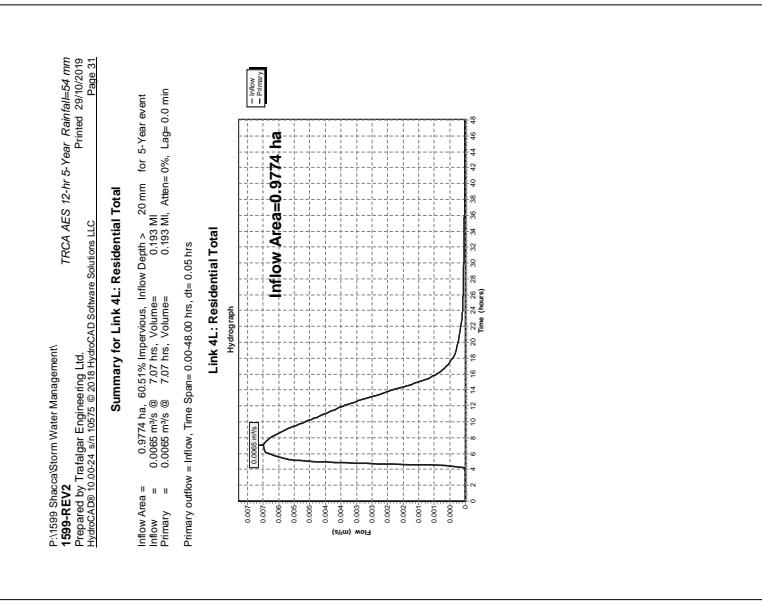
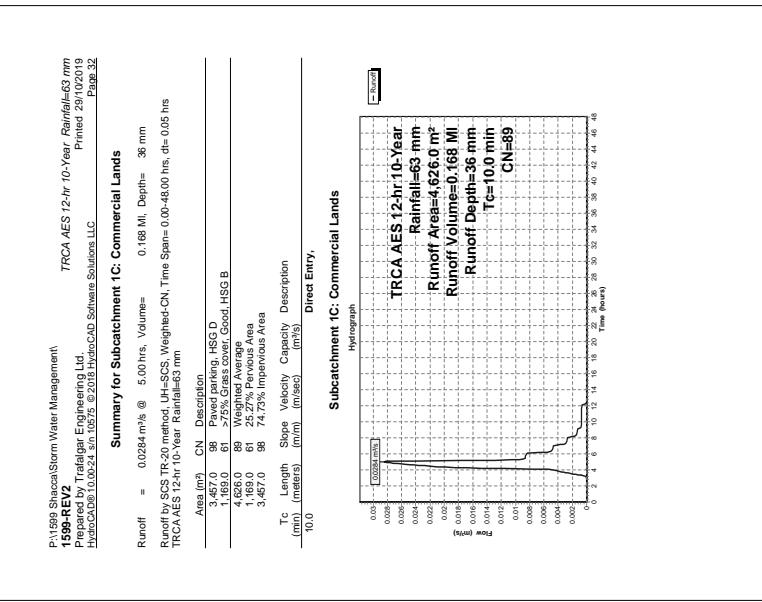
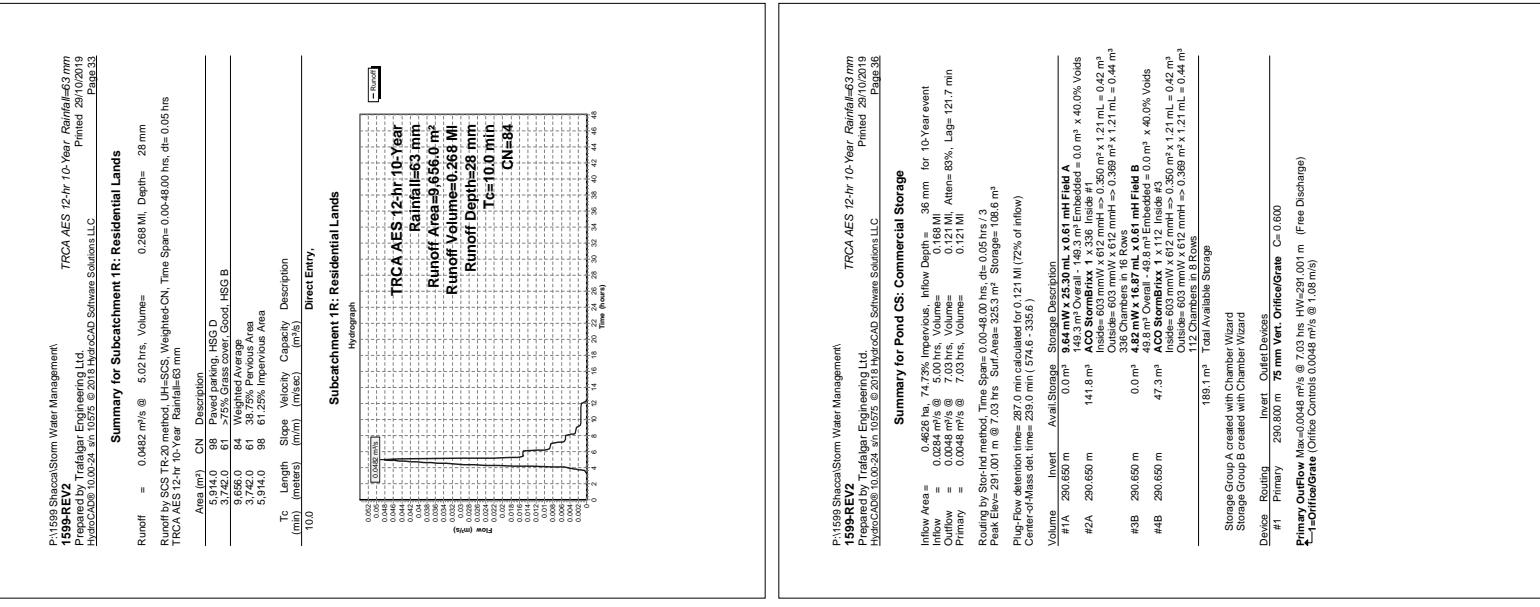
TRCA AES 12-hr 5-Year Rainfall=54 mm
1599-REV2
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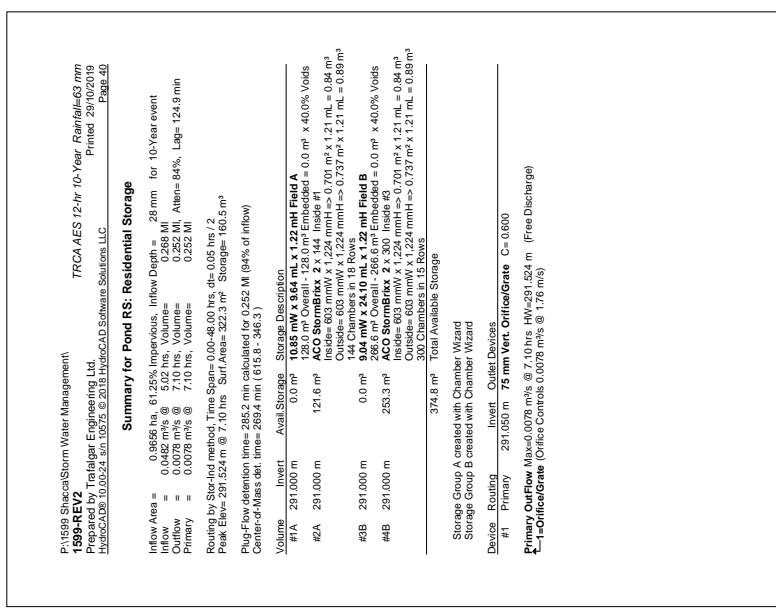
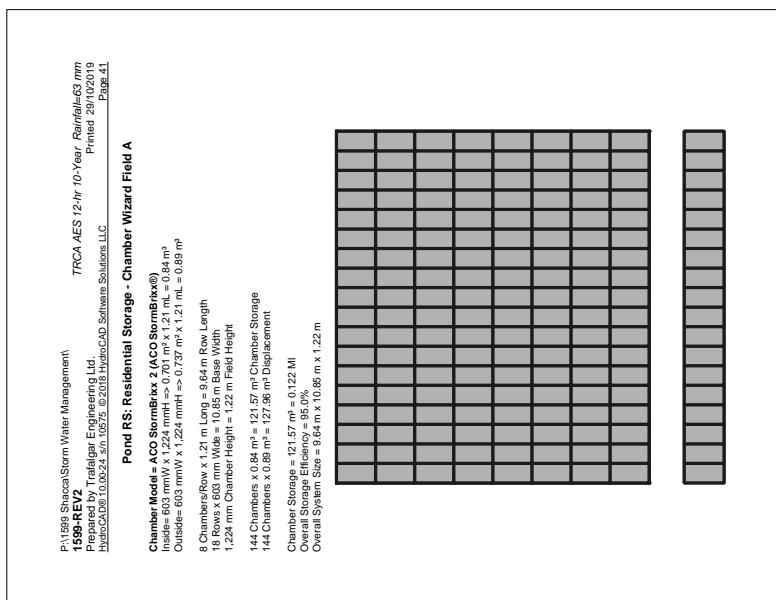
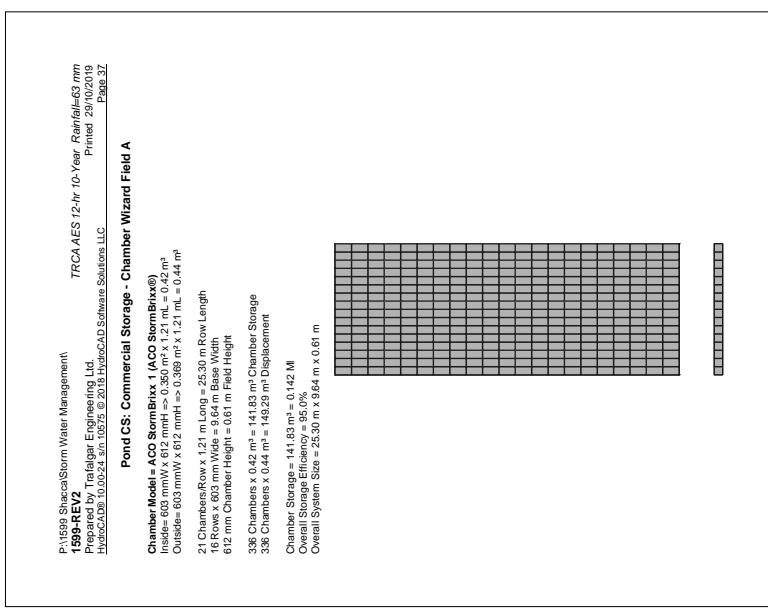
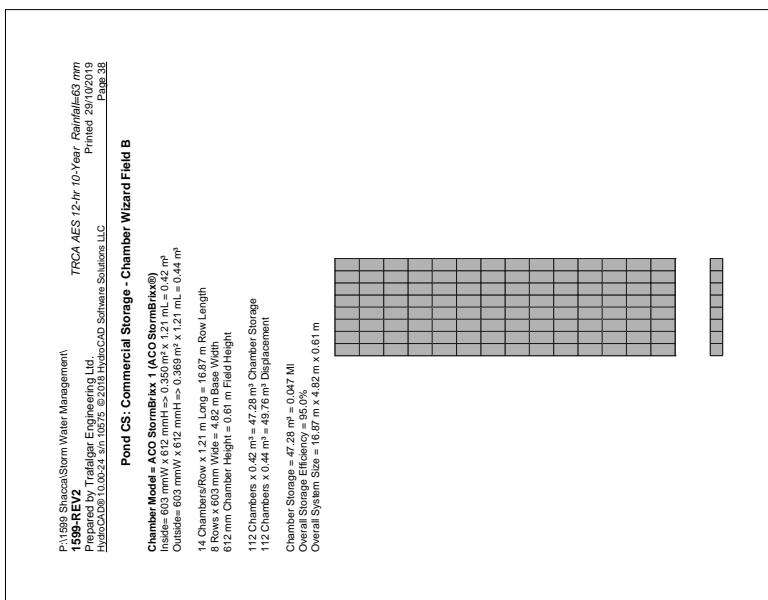
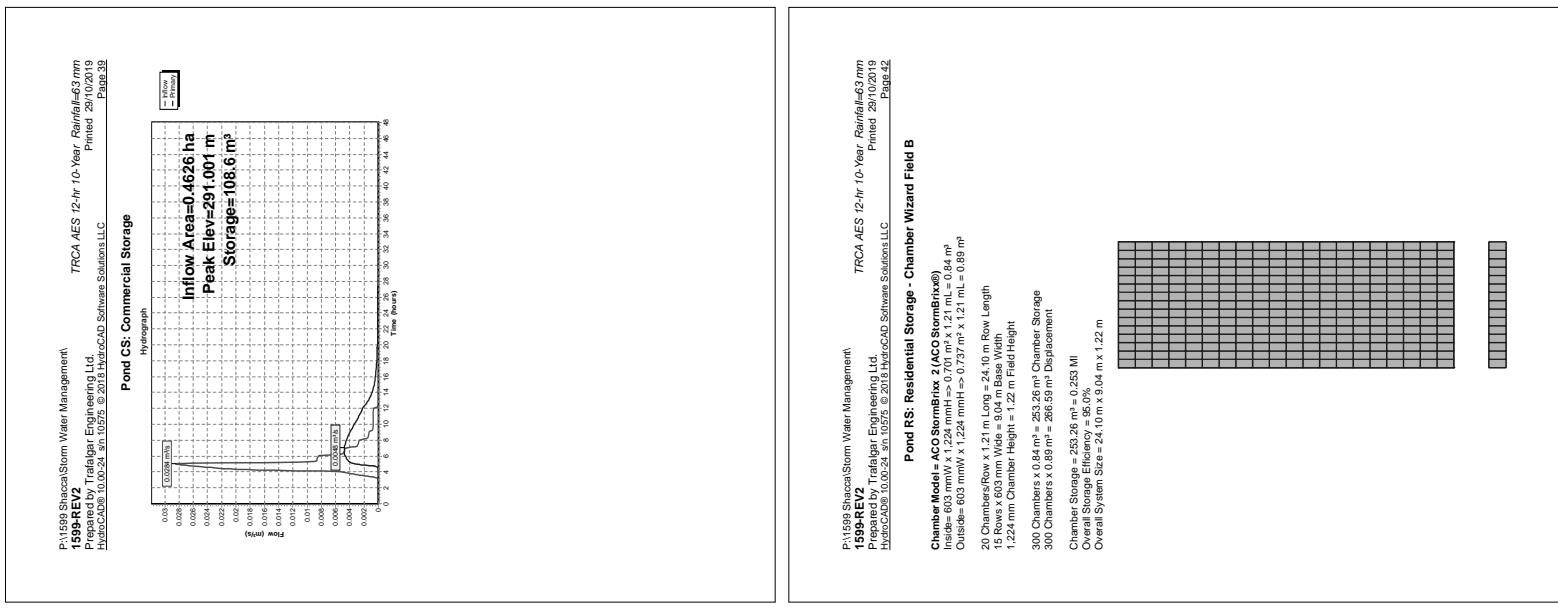
Summary for Link 3L: Commercial Total

Inflow Area = 0.5246 ha, Rainfall=54 mm
Inflow = 0.0040 m³ @ 6.07 hrs, Volume= 0.003 MI
Primary = 0.0040 m³ @ 6.07 hrs, Volume= 0.003 MI, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, de= 0.05 hrs

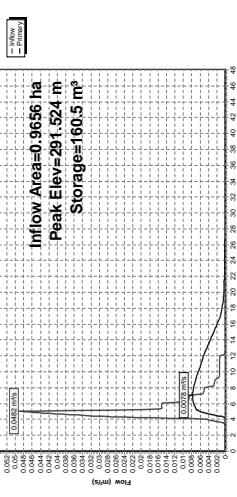
Link 3L: Commercial Total



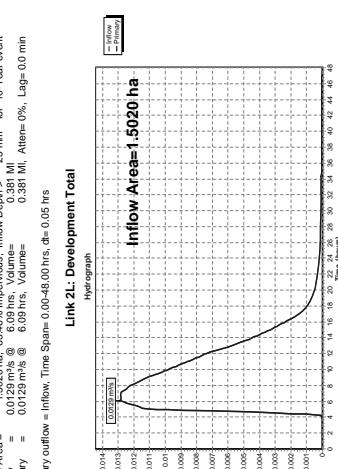




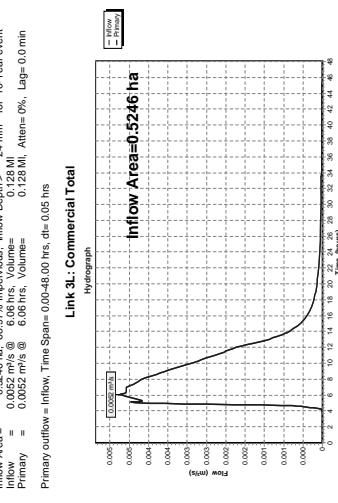
Pond RS: Residential Storage



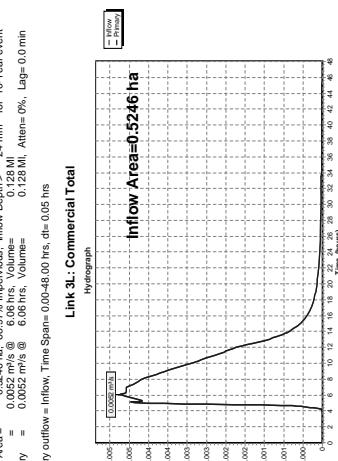
Pond RS: Residential Storage



Pond RS: Residential Storage

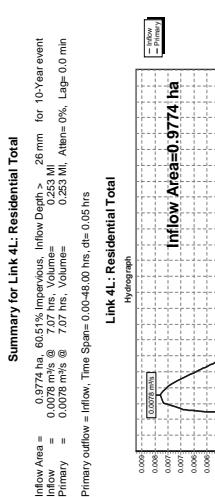


Pond RS: Residential Storage

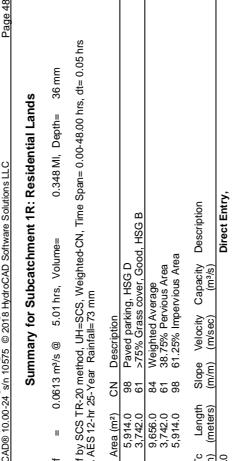
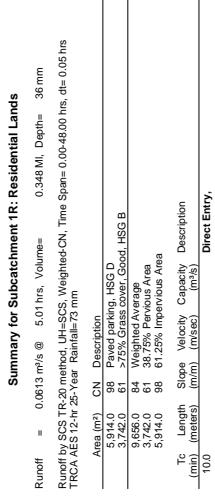


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Link 4L: Residential Total

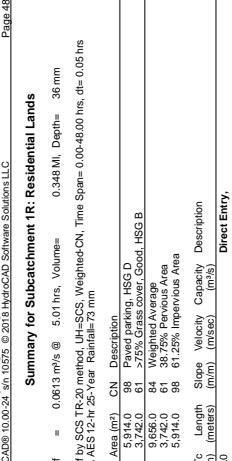
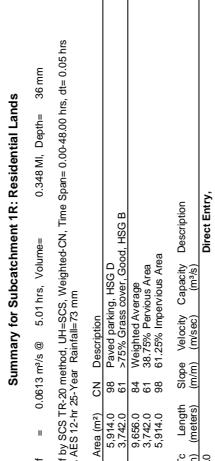


Link 4L: Residential Total



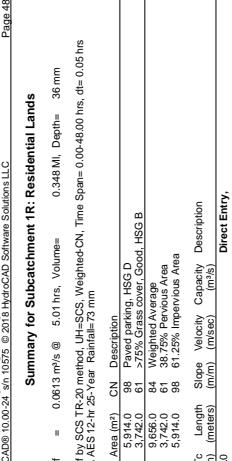
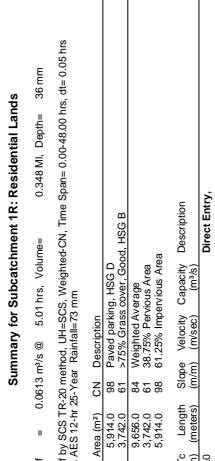
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Summary for Subcatchment 1C: Residential Lands



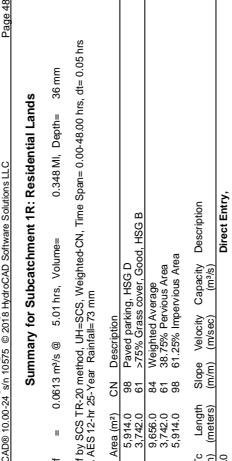
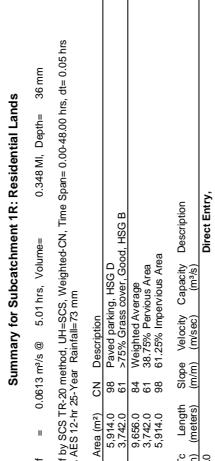
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Summary for Link 3: Commercial Total

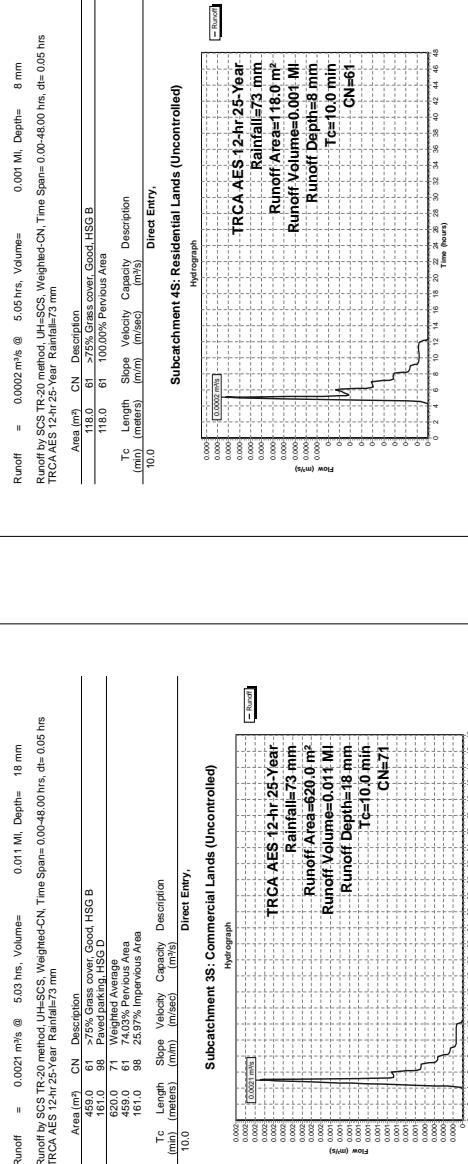


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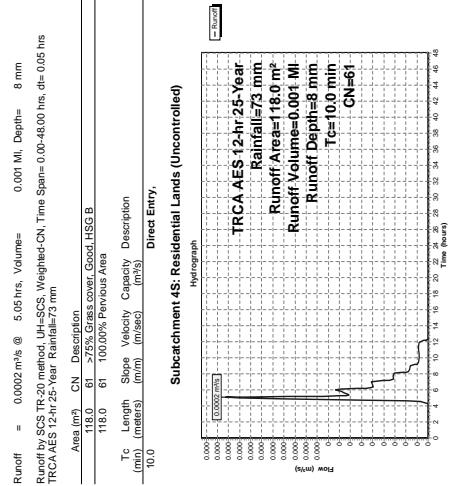
Summary for Link 4: Residential Total



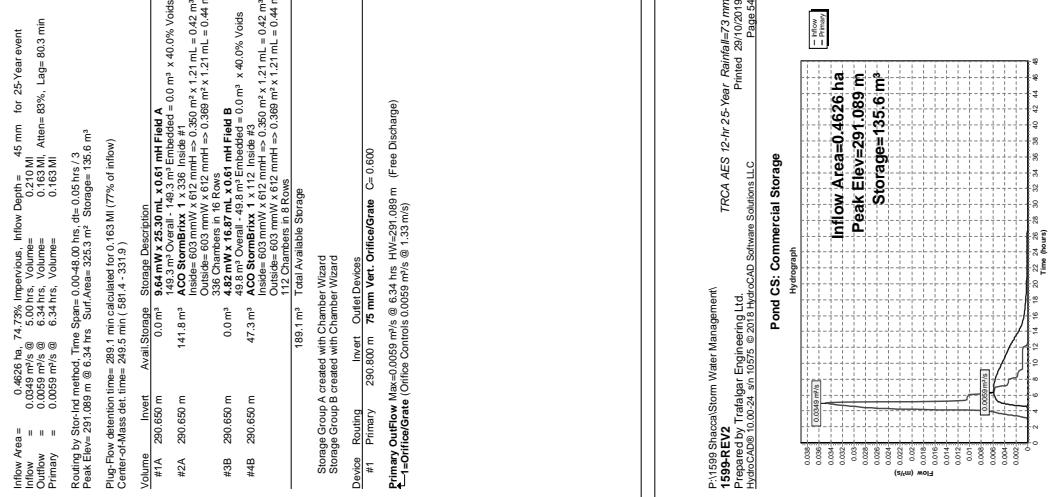
Summary for Subcatchment 35: Commercial Lands (Uncontrolled)



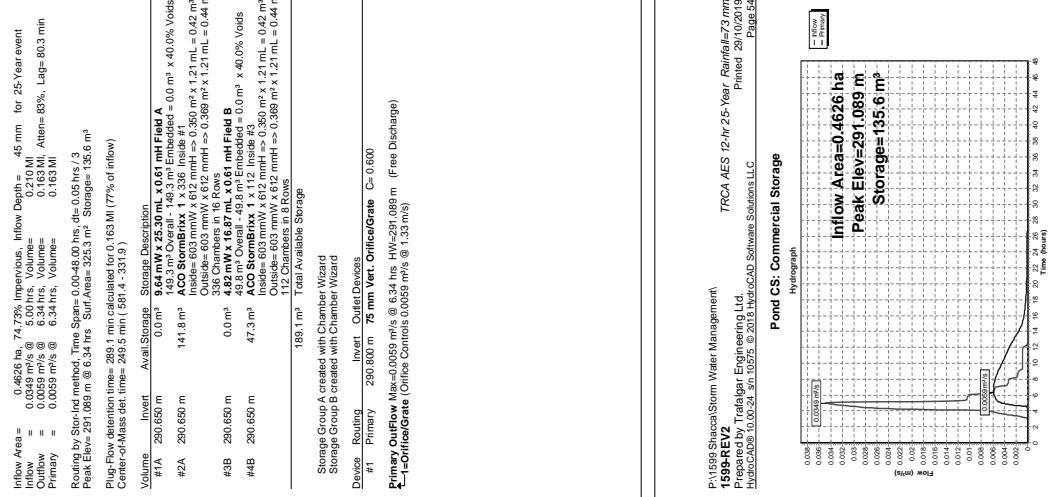
Summary for Subcatchment 35: Commercial Lands (Uncontrolled)



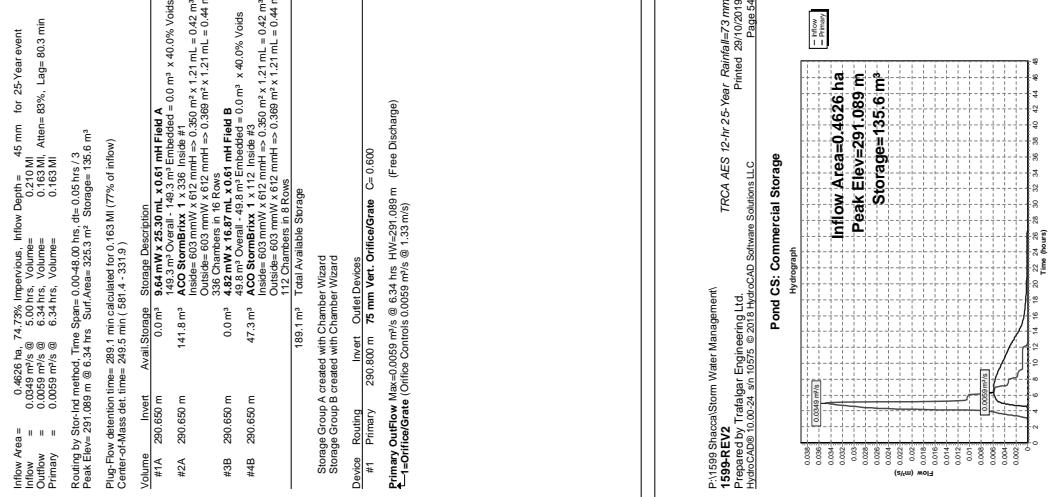
Summary for Pond CS: Commercial Storage



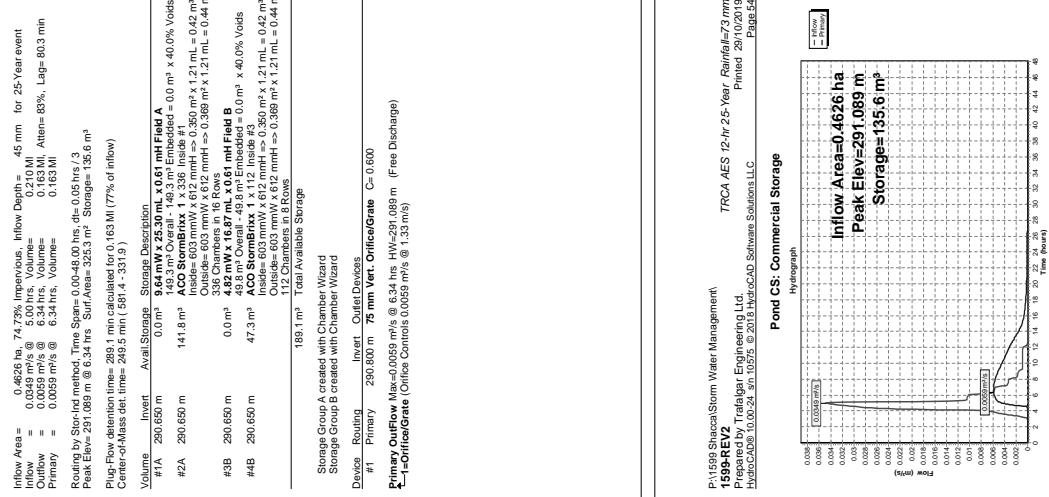
Summary for Pond CS: Commercial Storage



Pond CS: Commercial Storage - Chamber Wizard Field A



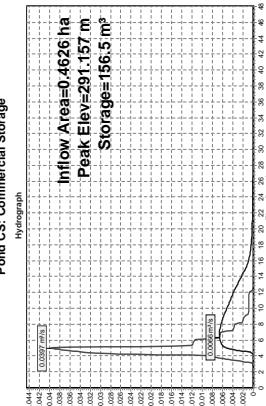
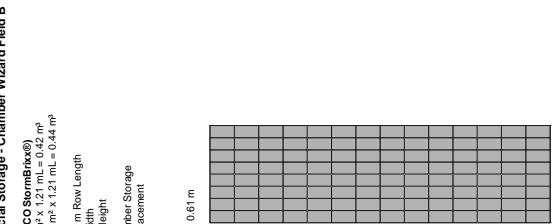
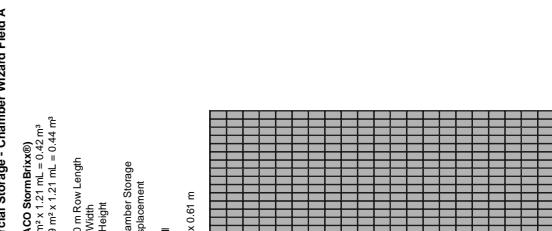
Pond CS: Commercial Storage - Chamber Wizard Field B



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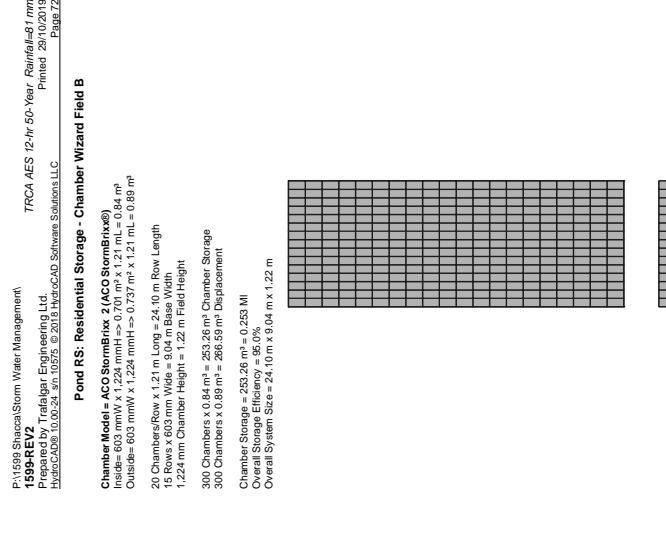
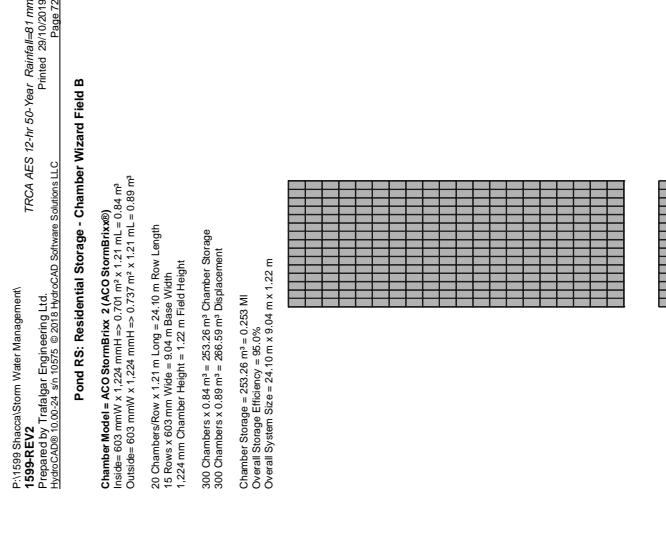
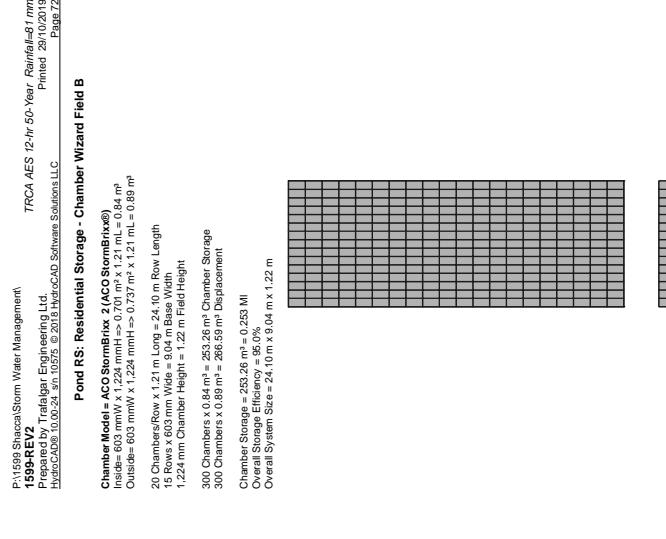
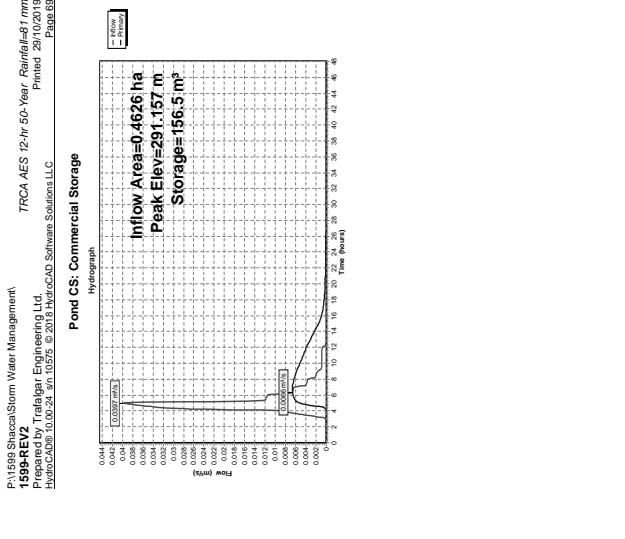
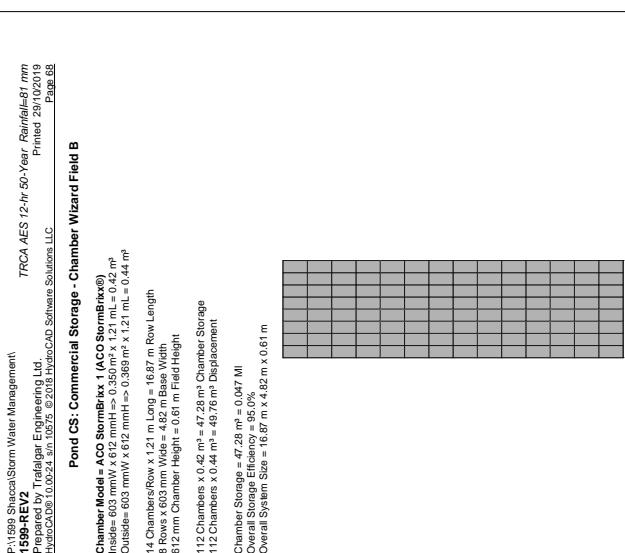
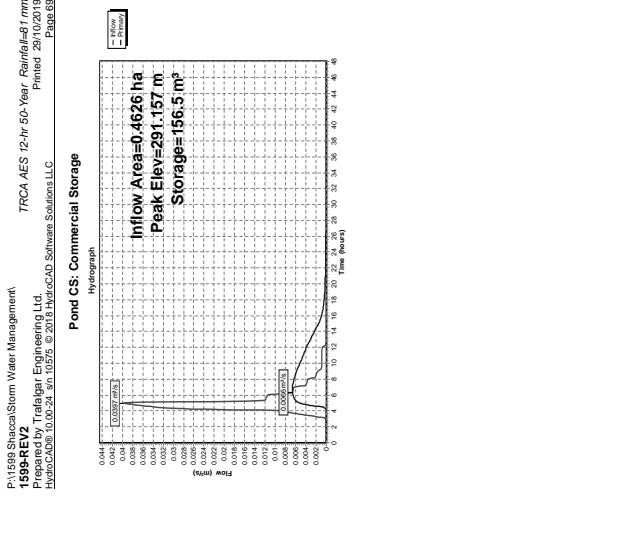
Summary for Pond RS: Residential Storage

Volume	Avg. Storage Description
#A 291,000 m ³	10.05 m ³ x 944 m ³ x 11.22 mH Field A
#A 291,000 m ³	121.6 m ³ Overall - 120.0 m ³ Embedded - 0.0 m ³ x 40.0% Voids
Inside: 603 mmW x 1.224 mmH => 0.701 m ³ x 1.21 mL = 0.84 m ³	
Outside: 603 mmW x 1.224 mmH => 0.737 m ³ x 1.21 mL = 0.89 m ³	
#B 291,000 m ³	9.04 m ³ x 11.22 mH Field B
#B 291,000 m ³	266.6 m ³ Overall - 266.6 m ³ Embedded - 0.0 m ³ x 40.0% Voids
Inside: 603 mmW x 1.224 mmH => 0.701 m ³ x 1.21 mL = 0.84 m ³	
Outside: 603 mmW x 1.224 mmH => 0.737 m ³ x 1.21 mL = 0.89 m ³	

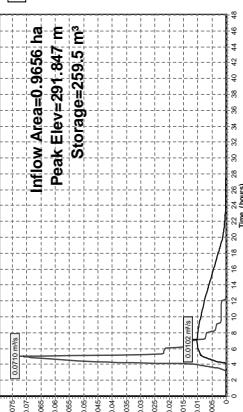
Total Available Storage: 374.8 m³

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

Device Route Inlet Outlet Devices
#1 Primary 291,050 m 75 mm Vert. Officel/Grade Cc: 0.600
_1-Officel/Grade (Orifice Controls 0.0102 m³/s @ 7.13 hrs Hh:29.847 m (Free Discharge)

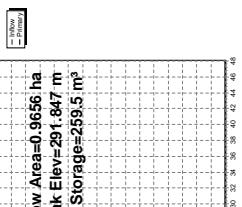


Pond RS: Residential Storage



Summary for Link 21: Development Total

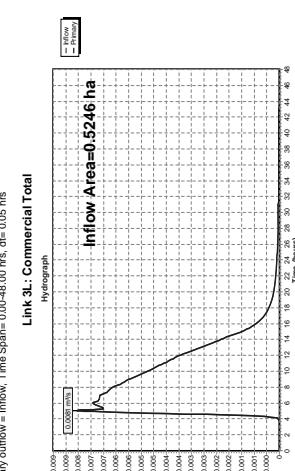
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Link 4L: Residential Total

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Inflow Area = 0.9774 ha, 66.1% impervious, Inflow Depth > 40 mm for 50-Year event
Inflow = 0.0103 m³/s @ 7.07 hrs, Volume= 0.394 Mi, Atten= 0% Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-40.00 hrs, del= 0.05 hrs



Inflow Area = 0.5246 ha, 66.1% impervious, Inflow Depth > 40 mm for 50-Year event
Inflow = 0.0081 m³/s @ 0.86 hrs, Volume= 0.239 Mi, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, del= 0.05 hrs

Link 3L: Commercial Total

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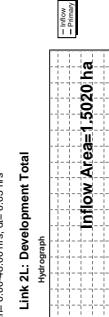
Hydrograph

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Subcatchment 1C: Commercial Lands

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Inflow Area = 1.5020 ha, 63.46% impervious, Inflow Depth > 40 mm for 50-Year event
Inflow = 0.0178 m³/s @ 0.86 hrs, Volume= 0.652 Mi, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-8.00 hrs, del= 0.05 hrs



Link 2L: Development Total

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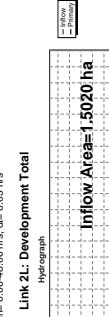
Hydrograph

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Subcatchment 1B: Residential Lands

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Inflow Area = 0.5246 ha, 66.1% impervious, Inflow Depth > 40 mm for 50-Year event
Inflow = 0.0081 m³/s @ 0.86 hrs, Volume= 0.239 Mi, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, del= 0.05 hrs



Link 3L: Commercial Total

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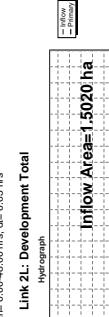
Hydrograph

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Subcatchment 1B: Residential Lands

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Inflow Area = 0.5246 ha, 66.1% impervious, Inflow Depth > 40 mm for 50-Year event
Inflow = 0.0081 m³/s @ 0.86 hrs, Volume= 0.239 Mi, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, del= 0.05 hrs

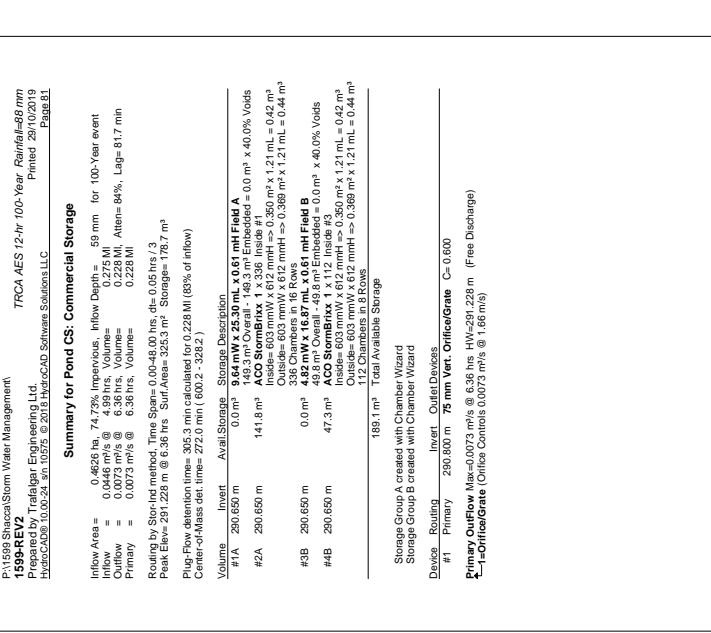
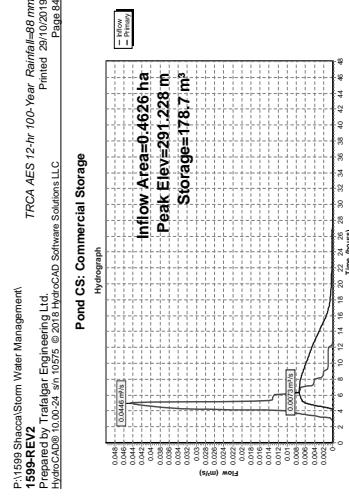
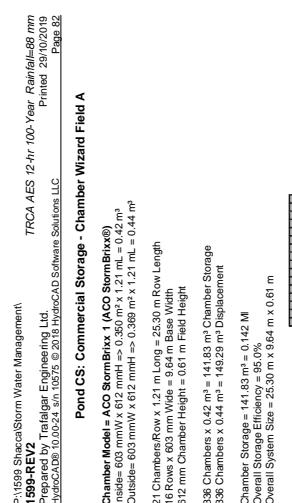
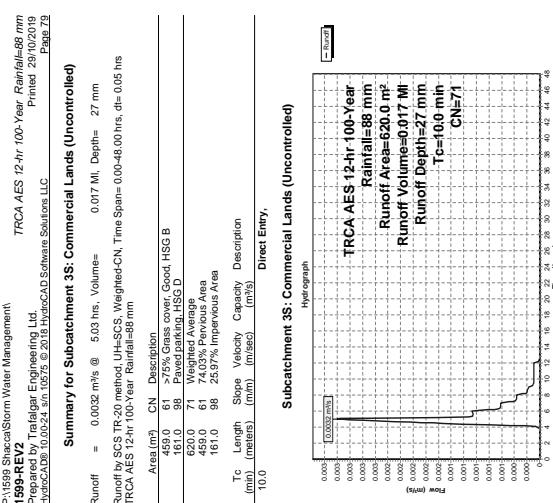


Link 3L: Commercial Total

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Hydrograph

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Summary for Pond RS: Residential Storage

Inflow Area = 0.03956 ha Rain=88 mm for 100-Year event
Inflow = 0.013 m³/s @ 0.125 hrs, Volume= 7.71 m³
Outflow = 0.012 m³/s @ 7.15 hrs, Volume= 0.012 m³
Primary = 0.012 m³/s @ 7.15 hrs, Volume= 0.0455 m³
Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, cte= 0.05 ins./2
Peak Elbow= 29.999 m @ 7.15 hrs, Surf.Area= 322.3 m²
PluF-Flow retention time= 353.8 min calculated for 0.055 M (95% of inflow)
Center-of-Mass det. time= 344.2 min (881.3 - 337.2)
Volume, Inflow, Available Storage
#1A 291.000 m 0.0 m 1085 m³ x 9.64 m³ x 122 m³ Field A
ACO StormBrix 2 x 144 Inside ft
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 1.21 m³ = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
#3B 291.000 m 0.0 m³ 286.6 m³ Field B
ACO StormBrix 2 x 144 Inside ft
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 1.21 m³ = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
#4B 291.000 m 253.3 m³ 300 Chambers
ACO StormBrix 2 x 300 Inside ft
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 1.21 m³ = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
300 Chambers in 15 Rows.
374.8 m³ Total Available Storage

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Routing Invent. Outlet Devices
#1 Primary 291.050 m 75 mm Vert. Orifice/Grate C= 0.600
↓-1-Orifice/Rate (Orifice Controls 0.012 m/s @ 7.13 hrs, Hl=291.999 m (Free Discharge)

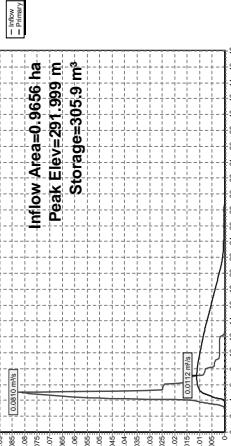
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TRCA AES 12-hr 100-Year Rainfall=88 mm
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TRCA AES 12-hr 100-Year Rainfall=88 mm
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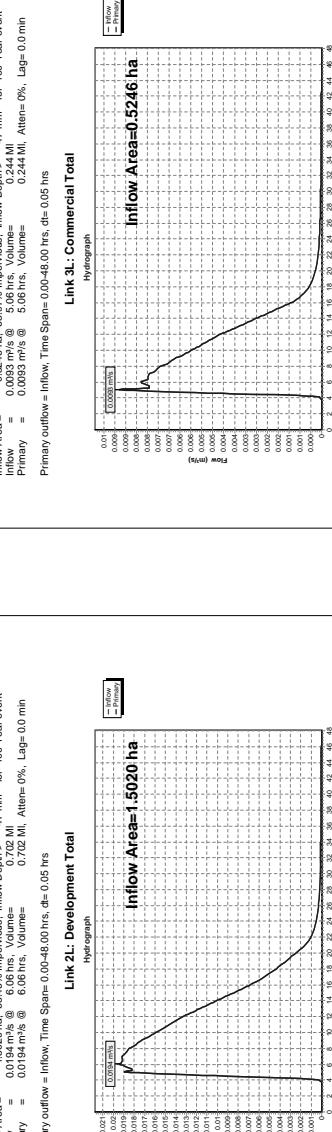
Pond RS: Residential Storage



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TRCA AES 12-hr 100-Year Rainfall=88 mm
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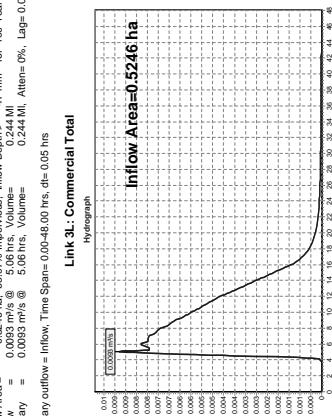
Link 2L: Development Total



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TRCA AES 12-hr 100-Year Rainfall=88 mm
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Link 3L: Commercial Total

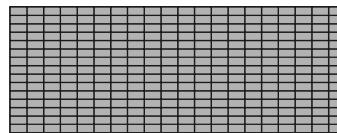


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Pond RS: Residential Storage - Chamber Wizard Field B

Chamber Model = ACO StormBrix 2 (ACO StormBrix(Brixey))
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 1.21 m³ = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
15 Chambers/Row x 12.1 m Long = 24.10 m Row Length
18 Rows x 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
1.224 mm Chamber Height = 0.04 m Base Width
1.224 mm Chamber Height = 0.22 m Field Height
300 Chambers x 0.84 m³ = 252.61 m³ Chamber Storage
300 Chambers x 0.88 m³ = 268.59 m³ Displacement
Chamber Storage = 252.61 m³ = 0.235 ML
Overall System Efficiency = 95%
Overall System Size = 24.10 m x 0.84 m x 1.22 m

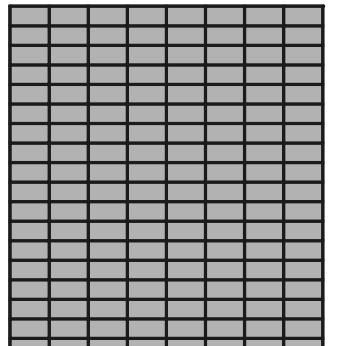


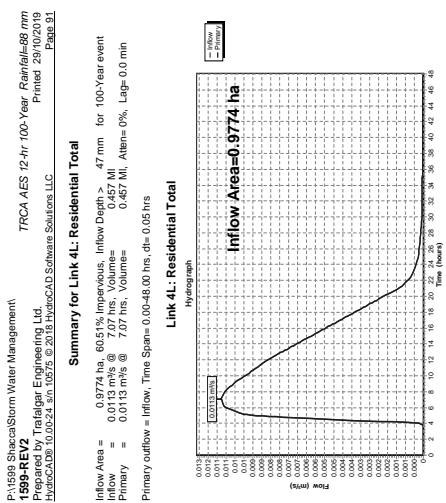
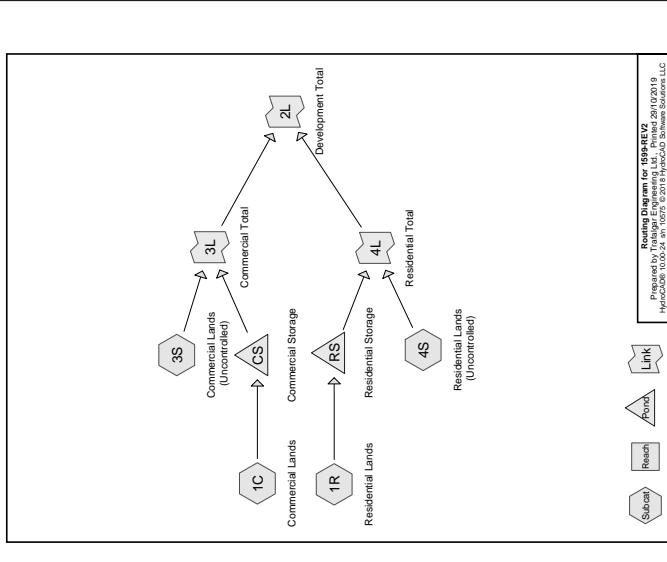
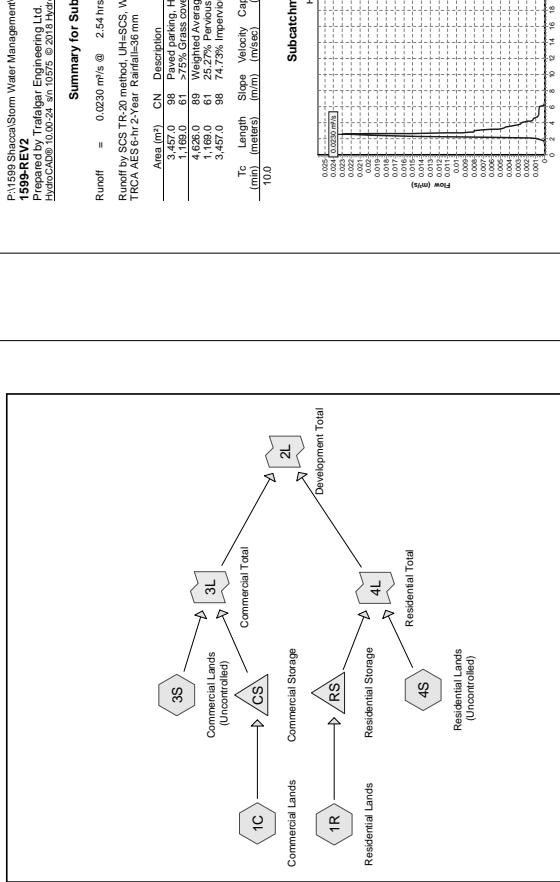
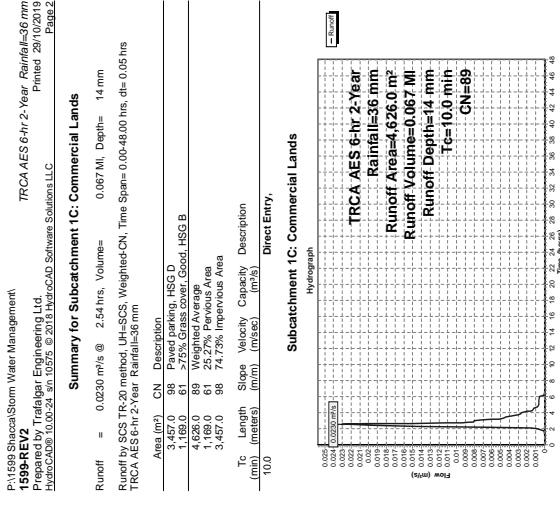
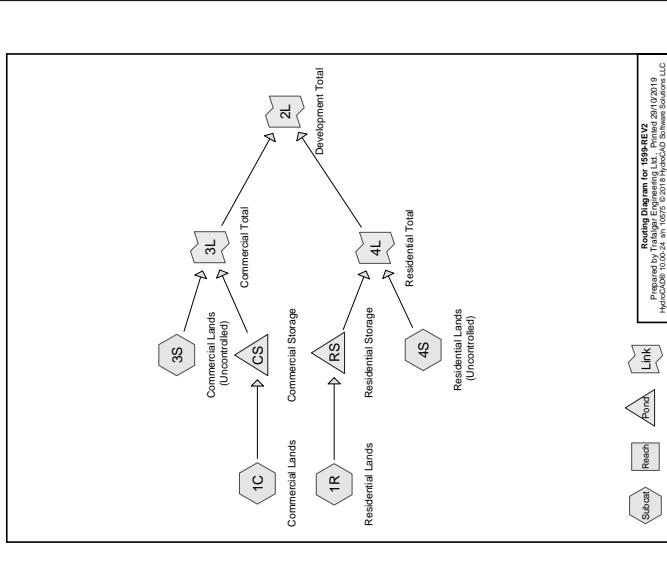
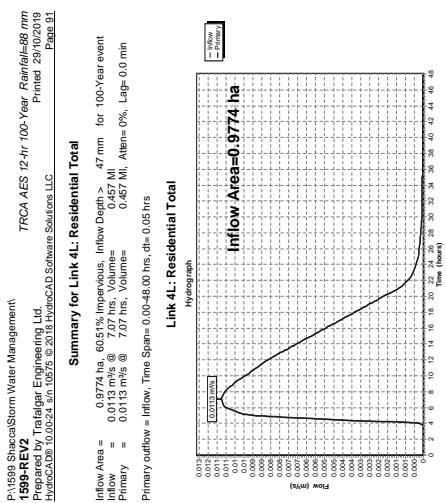
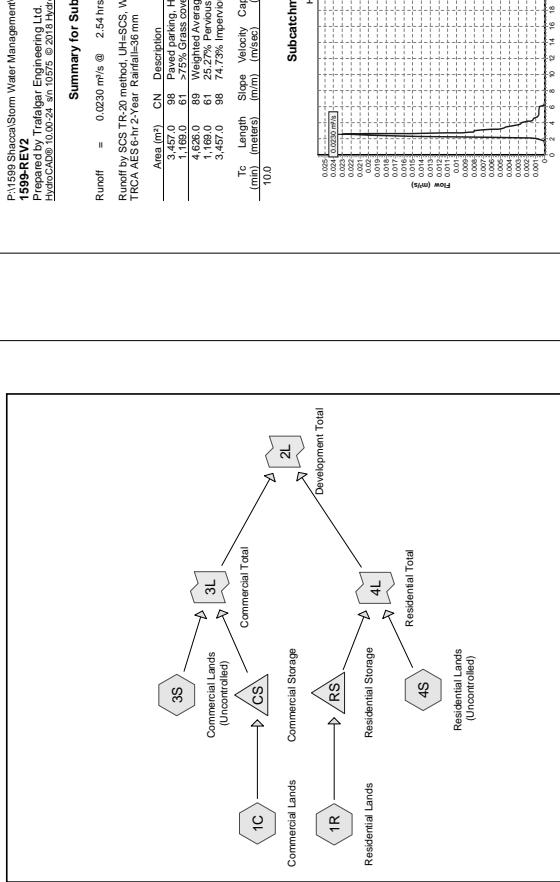
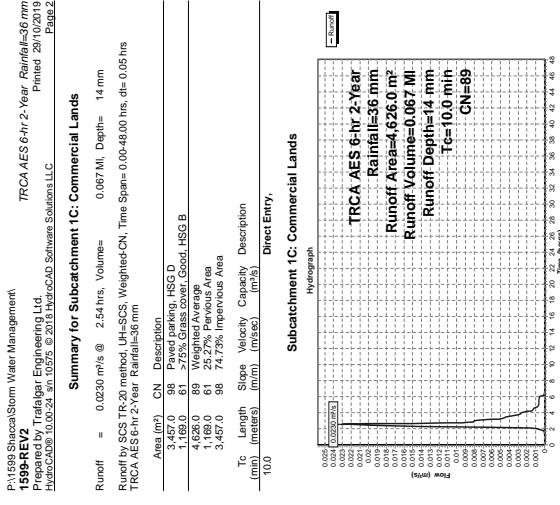
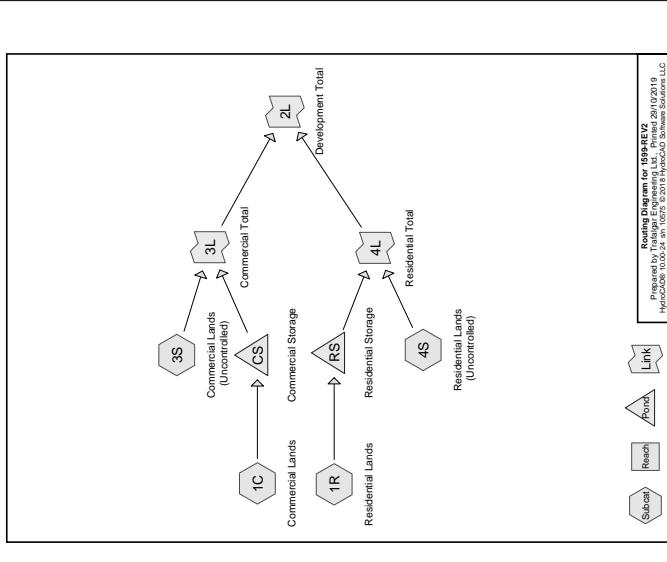
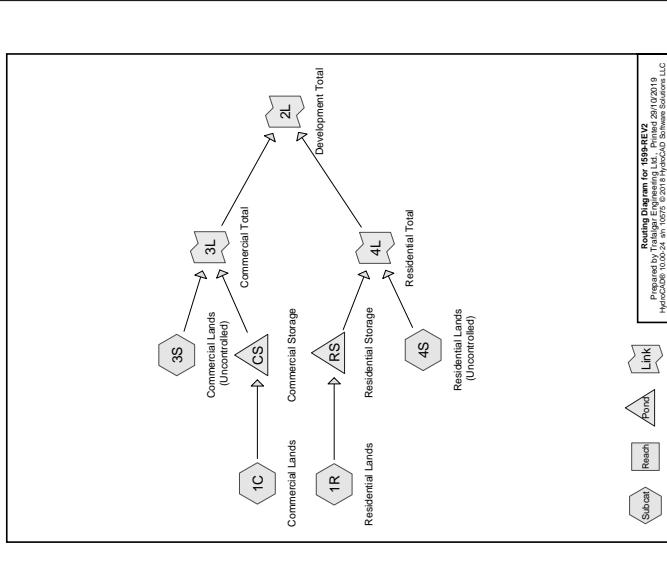
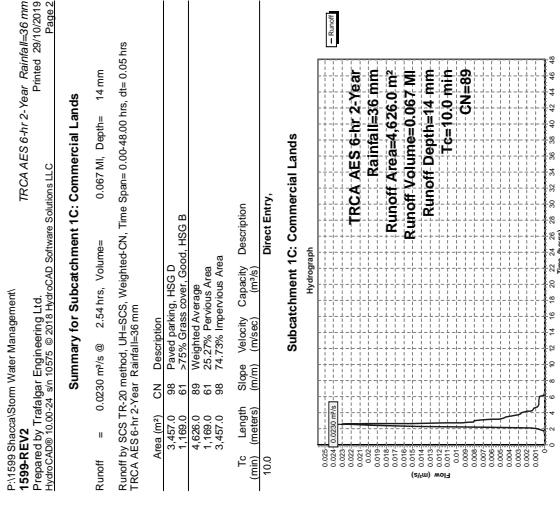
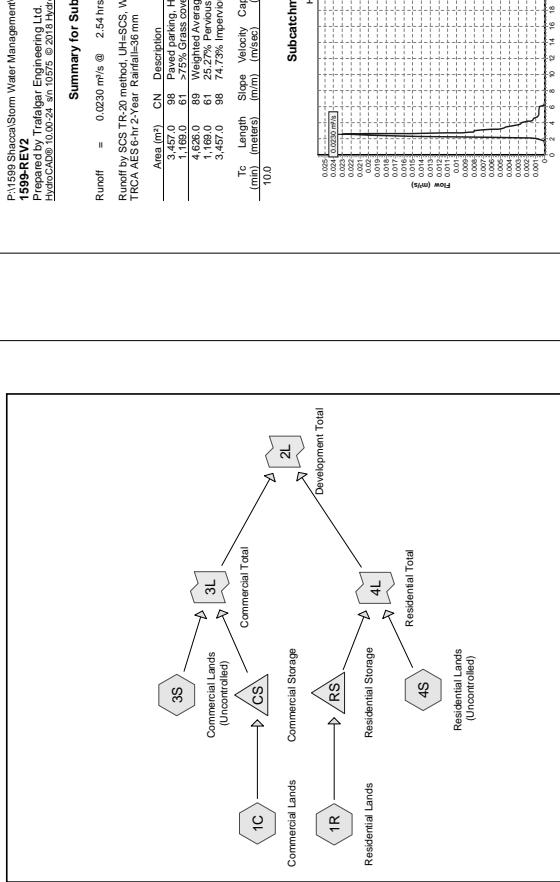
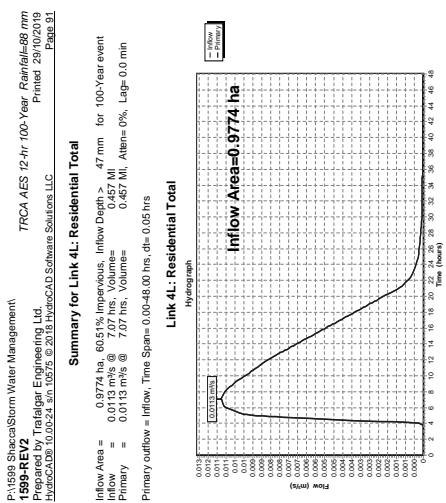
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Pond RS: Residential Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrix 2 (ACO StormBrix(Brixey))
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 1.21 m³ = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
8 Chambers/Row x 12.1 m Long = 9.64 m Row Length
18 Rows x 603 mm/W x 1224 mm/H => 0.737 m² x 1.21 m³ = 0.89 m³
1.224 mm Chamber Height = 0.04 m Base Width
1.224 mm Chamber Height = 0.22 m Field Height
144 Chambers x 0.84 m³ = 121.57 m³ Chamber Storage
144 Chambers x 0.88 m³ = 127.56 m³ Displacement
Chamber Storage = 121.57 m³ = 0.122 ML
Overall System Efficiency = 95%
Overall System Size = 9.64 m x 10.85 m x 1.22 m





Summary for Pond CS: Commercial Storage

Inflow Area = 0.4268 ha 74.3% impervious, inflow Depth = 14 mm for 2-Year event
 Outflow = 0.010 m³/s @ 4.68 hrs. Volume= 0.020 m³
 Primary = 0.0010 m³/s @ 4.68 hrs. Volume= 0.020 m³
 Routing by Sto-1nd method, Time Span = 0.00-48.00 hrs, cte=0.05 hrs / 3
 Peak Elv@ 29.43 m @ 16.4 hrs. Surf.Area= 322.3 m² Storage= 39.6 m³
 Plug-Flow retention time= 407.4 min (calculated for 0.20 M ft inflow)
 Center-of-Mass det. time= 370.0 min (349.4 - 179.4)
 Volume Available Storage Description
 #1A 290/650 m 0.0 m x 143.9 m³ x 0.142 m³
 Overall Storage Efficiency = 0%
 Overall System Size = 29.30 m x 9.64m x 0.61 m

A1 189.1 m³ Total Available Storage
 Storage Group A created with Chamber Wizard
 Storage Group B created with Chamber Wizard
 Device Routing Invert Outlet Devices
 #1 Primary 290/650 m 75 mm Vert. Orifice/Grate C= 0.600
 #1-1Orifice/Rate Orifice Controls 0.000 m³/s @ 0.39 m/s



TRCA AES 6-hr 2-Year Rainfall=36 mm
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Page 7

Pond CS: Commercial Storage

A1 189.1 m³ Total Available Storage
 Storage Group A created with Chamber Wizard
 Storage Group B created with Chamber Wizard
 Device Routing Invert Outlet Devices
 #1 Primary 290/650 m 75 mm Vert. Orifice/Grate C= 0.600
 #1-1Orifice/Rate Orifice Controls 0.000 m³/s @ 0.39 m/s

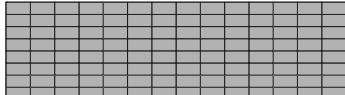
Volume	Invert	Available Storage	Storage Description
#1A	291/100 m	0.0 m³	10.85 m³ x 9.64 m x 121.6 m³ x 12.0 m³ x 40.0% Voids
#2A	291/000 m	121.6 m³	ACO StormBrix 2 x 144 Inside #1 0.08 m³ x 12.0 m³ x 84.0 m³ Inside= 603 mmW x 612 mmH => 0.08 m³ x 12.0 m³ x 84.0 m³ Outside= 603 mmW x 612 mmH => 0.08 m³ x 12.0 m³ x 84.0 m³
#3B	291/1000 m	0.0 m³	9.44 m³ x 24.10 m x 122.0 m³ x 26.6 m³ x 40.0% Voids
#4B	291/0000 m	253.3 m³	ACO StormBrix 2 x 300 Inside #3 0.1 m³ x 12.0 m³ x 84.0 m³ Inside= 603 mmW x 612 mmH => 0.1 m³ x 12.0 m³ x 84.0 m³ Outside= 603 mmW x 612 mmH => 0.1 m³ x 12.0 m³ x 84.0 m³

374.8 m³ Total Available Storage

Storage Group A created with Chamber Wizard
 Storage Group B created with Chamber Wizard
 Device Routing Invert Outlet Devices
 #1 Primary 291/050 m 75 mm Vert. Orifice/Grate C= 0.600
 Primary Outflow Max=0.039 m³/s @ 0.09 hrs HVA=291.197 m (Free Discharge)
 #1-1Orifice/Rate Orifice Controls 0.0039 m³/s @ 0.08 m/s

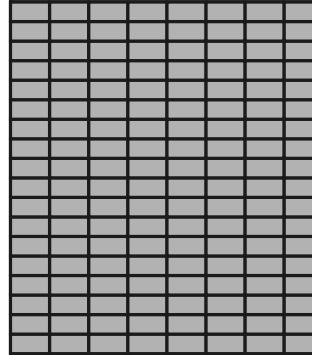
Pond CS: Commercial Storage - Chamber Wizard Field B

Chamber Model = ACO StormBrix(8x8)
 Inside= 603 mmW x 612 mmH => 0.356 m³ x 12.0 m³ x 4.44 m³
 Outside= 603 mmW x 612 mmH => 0.369 m³ x 12.0 m³ x 4.44 m³
 14 Chambers/Row x 12.0 m Long = 16.87 m Row Length
 8 Rows x 603 mm Wide = 4.82 m Base Width
 612 mm Chamber Height = 0.61 m Field Height
 112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
 112 Chambers x 0.44 m = 49.76 m³ Displacement
 Chamber Storage = 47.28 m³ x 0.67 m
 Chamber Storage Efficiency = 88.0%
 Overall System Size = 16.87 m x 4.82 m x 0.61 m



Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrix(8x8)
 Inside= 603 mmW x 612 mmH => 0.356 m³ x 12.0 m³ x 4.44 m³
 Outside= 603 mmW x 612 mmH => 0.369 m³ x 12.0 m³ x 4.44 m³
 14 Chambers/Row x 12.0 m Long = 16.87 m Row Length
 8 Rows x 603 mm Wide = 4.82 m Base Width
 612 mm Chamber Height = 0.61 m Field Height
 112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
 112 Chambers x 0.44 m = 49.76 m³ Displacement
 Chamber Storage = 47.28 m³ x 0.67 m
 Chamber Storage Efficiency = 88.0%
 Overall System Size = 16.87 m x 4.82 m x 0.61 m

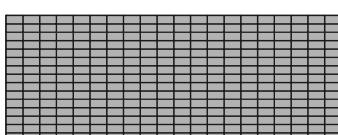


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7 TRCA A/E/S 6-hr 2-Year Rainfall=36 mm
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Pond RS: Residential Storage - Chamber Wizard Field B

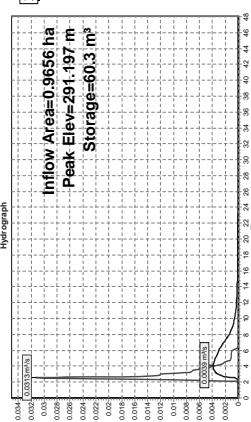
Chamber Model = ACO StormBrixx 2 (ACO StormBrixx®)
Inlet= 0.03 mm/s x 1.25 mm/h => 0.071 m³ / 1.21 ml = 0.064 m³
Outflow= 0.03 mm/W x 1.25 mm/h => 0.737 m³ / 1.21 ml = 0.6 m³
20 Chambers/Rox x 1.21 m Long = 24.0 m Row length
15 Rows x 603 mm Wide = 9.04 m Base Width
1.224 mm Chamber Height = 1.22 m Field Height
300 Chambers x 0.064 m³ = 286.59 m³ Chamber Storage
300 Chambers x 0.064 m³ = 286.59 m³ Displacement
Chamber Storage = 253.26 m³ + 0.253 m³
Overall Storage Efficiency = 90.0%
Overall System Size = 24.0 m x 9.04 m x 1.22 m



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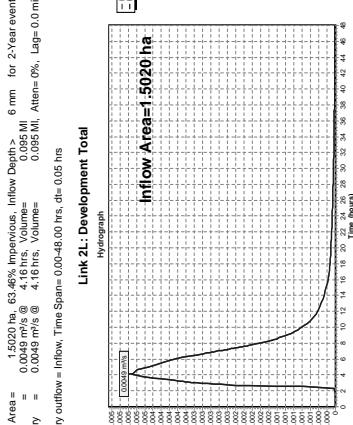
Pond RS: Residential Storage - Residential Total



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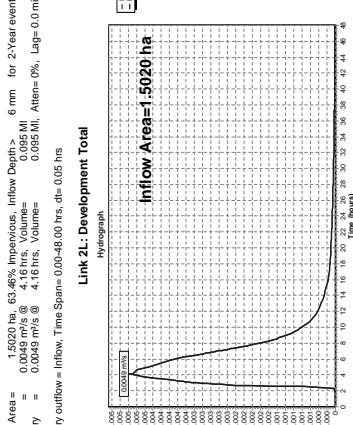
Summary for Link 2L: Development Total



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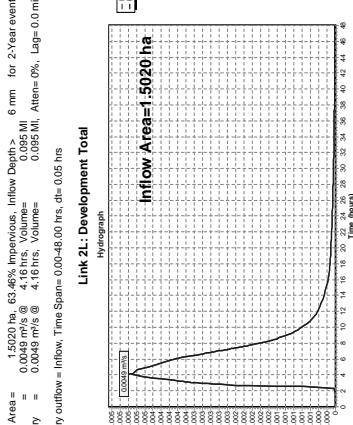
Summary for Link 2L: Development Total



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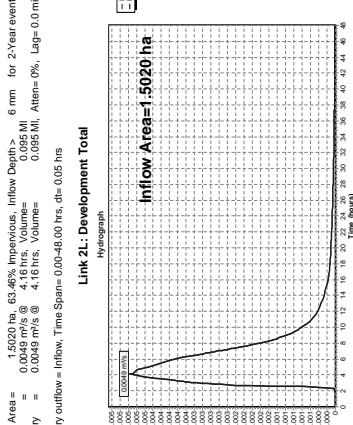
Summary for Subcatchment 1C: Commercial Lands



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TRCA A/E/S 6-hr 5-Year Rainfall=48 mm
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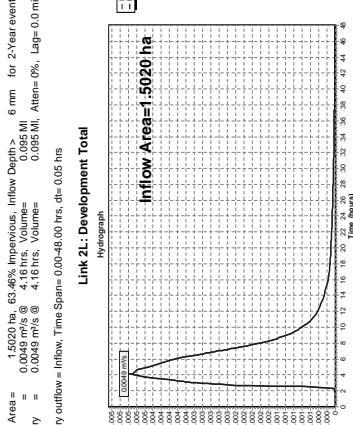
Summary for Subcatchment 1C: Commercial Lands



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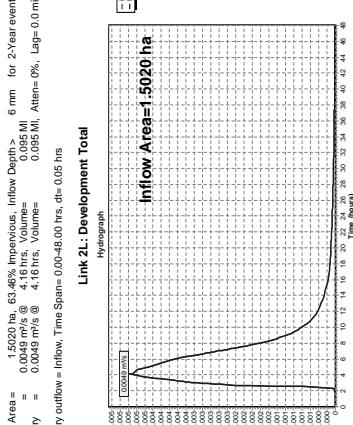
Link 4L: Residential Total



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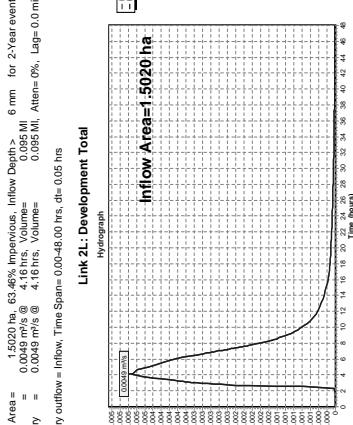
Link 4L: Residential Total



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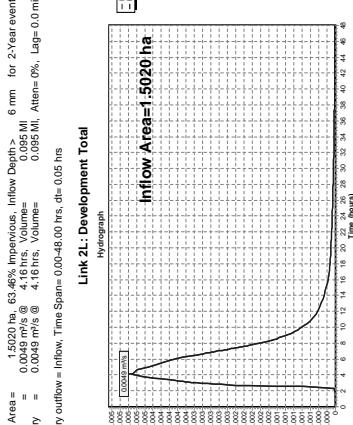
Link 3L: Commercial Total



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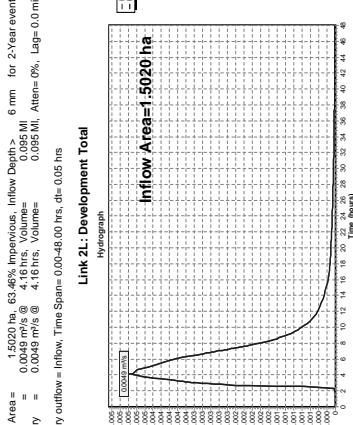
Link 3L: Commercial Total



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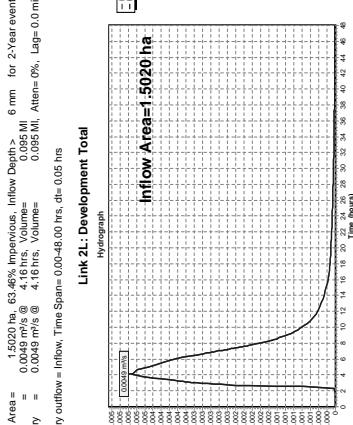
Inflow Area=0.9774 ha



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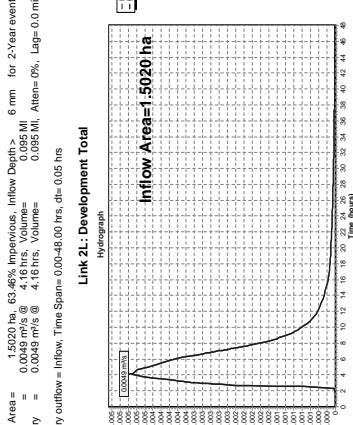
Inflow Area=0.5246 ha



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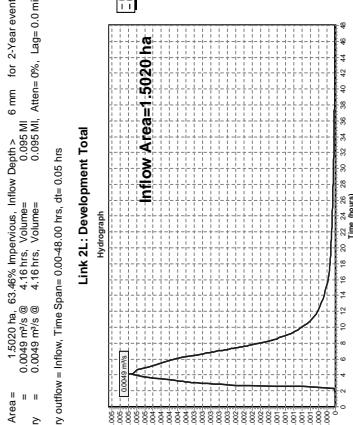
Subcatchment 1C: Commercial Lands



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Subcatchment 1C: Commercial Lands

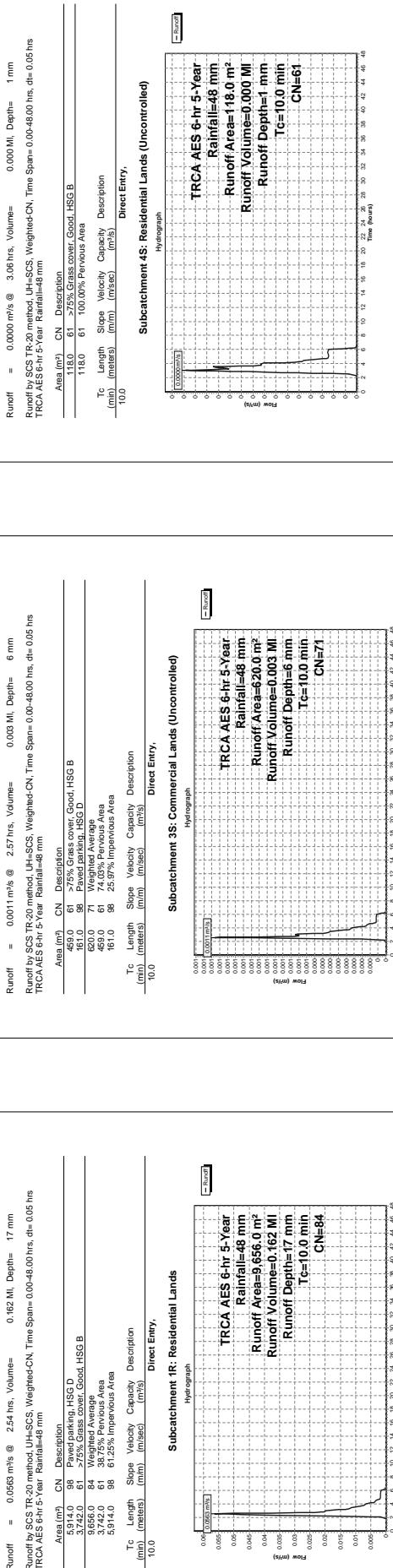


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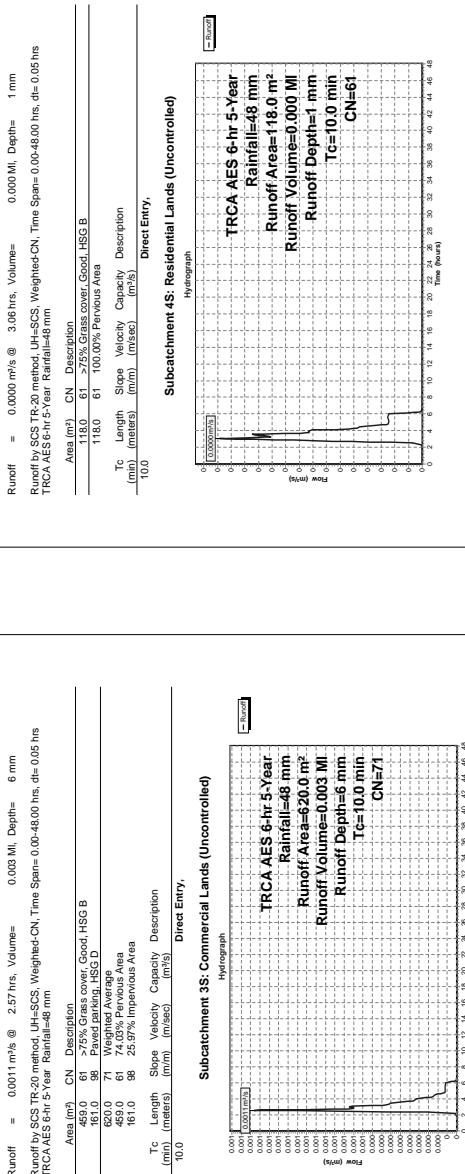
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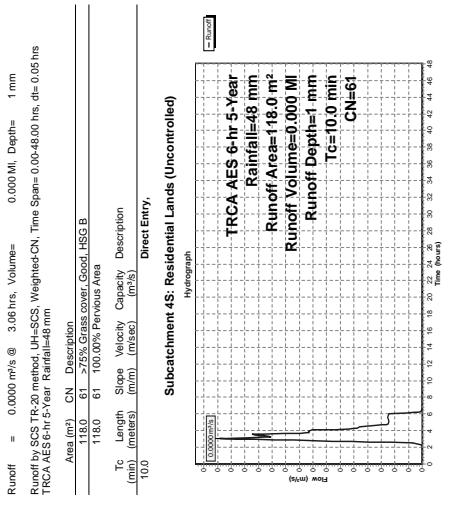
Summary for Subcatchment 1R: Residential Lands



Summary for Subcatchment 3S: Commercial Lands (Uncontrolled)



Summary for Subcatchment 4S: Residential Lands (Uncontrolled)



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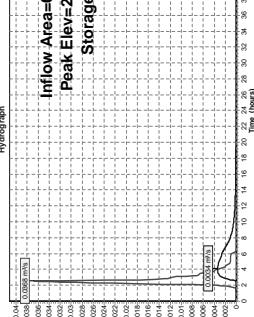
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Summary for Pond CS: Commercial Storage - Chamber Wizard Field A

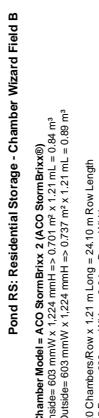
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Route Invert Outlet Devices
#1 Primary 290.650 m 75 mm Vert. OfficGate Cc: 0600
Primary Outflow Max=0.0034 m³s @ 4.10 hrs HW=298.923 m (Free Discharge)
_1Orifice/Drate Orifice Controls 0.0034 m³s @ 0.78 m/s

Pond CS: Commercial Storage

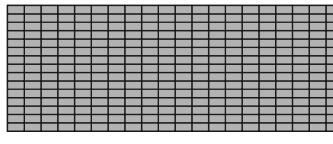


Time (hours)

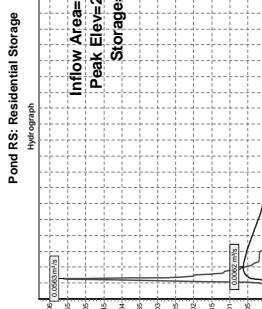
Pond CS: Residential Storage



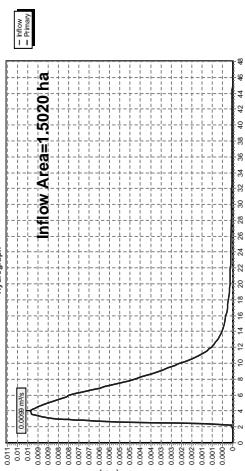
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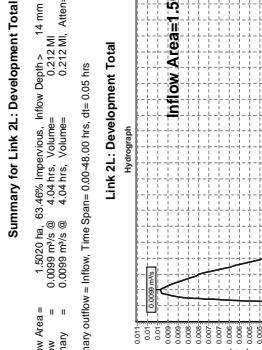
Pond RS: Residential Storage



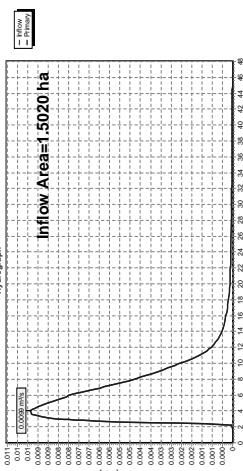
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Pond RS: Residential Storage



Time (hours)



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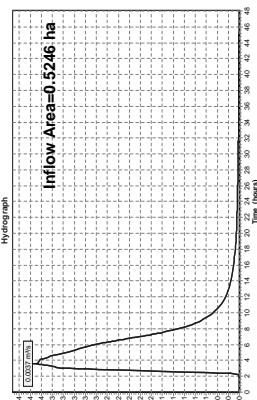
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Summary for Link 3L: Commercial Total

Inflow Area = 0.5546 ha, 68.70% Impervious, Inflow Depth > 13 mm for 5-Year event
Inflow = 0.0327 m³/s @ 3.55 hrs, Volume= 0.086 Ml, Attenu= 0%, Lag= 0.0 min
Primary = 0.0082 m³/s @ 4.07 hrs, Volume= 0.066 Ml, Attenu= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs

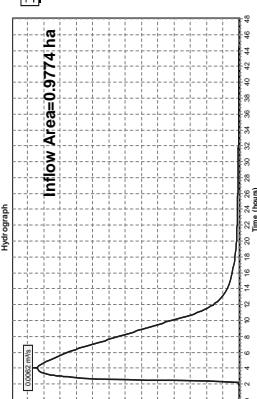
Link 3L: Commercial Total



Summary for Link 4L: Residential Total

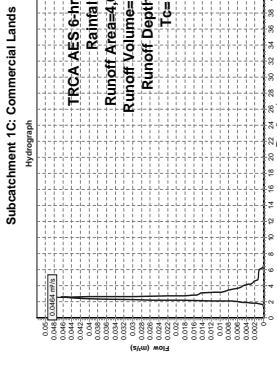
Inflow Area = 0.9774 ha, 60.15% Impervious, Inflow Depth > 15 mm for 5-Year event
Inflow = 0.0432 m³/s @ 2.53 hrs, Volume= 0.147 Ml, Attenu= 0%, Lag= 0.0 min
Primary = 0.0082 m³/s @ 4.07 hrs, Volume= 0.166 Ml, Attenu= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs

Link 4L: Residential Total



Summary for Subcatchment 1C: Commercial Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



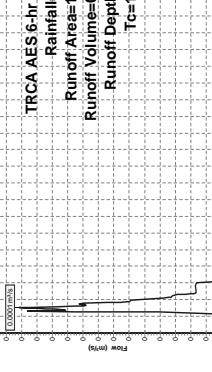
Summary for Subcatchment 1C: Residential Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



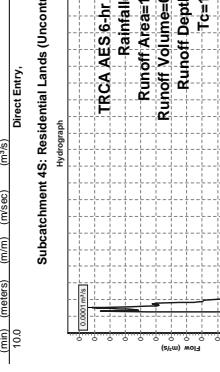
Summary for Subcatchment 1C: Commercial Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



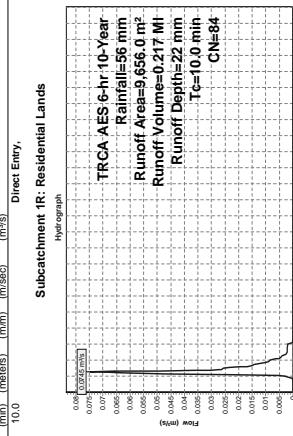
Summary for Subcatchment 1C: Residential Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



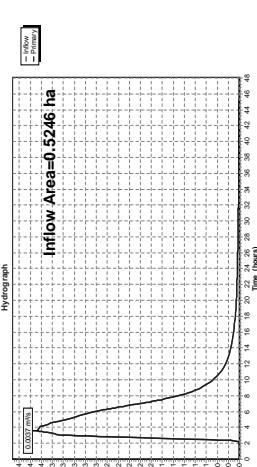
Summary for Subcatchment 1R: Residential Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



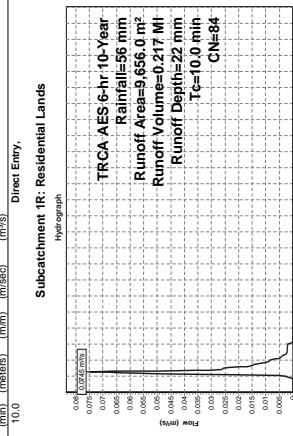
Summary for Subcatchment 3S: Commercial Lands (Uncontrolled)

Runoff = 0.0018 m³/s @ 2.56 hrs, Volume= 0.006 Ml, Depth= 9 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
1.180 0 61 75% Grass, cover, Grd, HSG B
1.180 0 61 100.00% Permeous Area



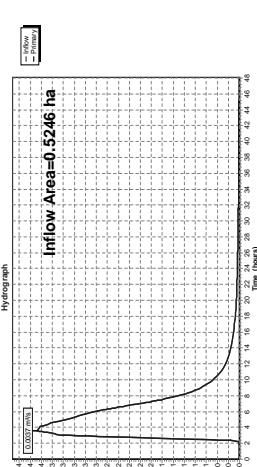
Summary for Subcatchment 1R: Residential Lands

Runoff = 0.0464 m³/s @ 2.53 hrs, Volume= 0.140 Ml, Depth= 30 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
3.457 0 98 Paved parking, Grd, HSG D
4.165 0 98 25.27% Permeous Area
1.163 0 98 25.27% Permeous Area
3.457 0 98 74.73% Impervious Area



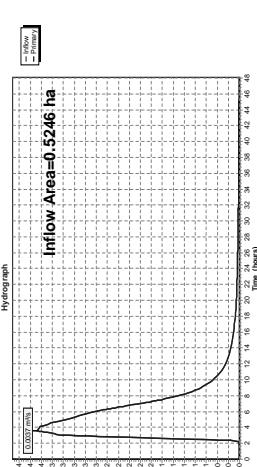
Summary for Subcatchment 3S: Commercial Lands (Uncontrolled)

Runoff = 0.0018 m³/s @ 2.56 hrs, Volume= 0.006 Ml, Depth= 9 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
1.180 0 61 75% Grass, cover, Grd, HSG B
1.180 0 61 100.00% Permeous Area



Summary for Subcatchment 1S: Residential Lands

Runoff = 0.0018 m³/s @ 2.56 hrs, Volume= 0.006 Ml, Depth= 9 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dts= 0.05 hrs
TRCA AES-6-hr 10-Year Rainfall=56 mm
Area (ha) : CN : Description
1.180 0 61 75% Grass, cover, Grd, HSG B
1.180 0 61 100.00% Permeous Area



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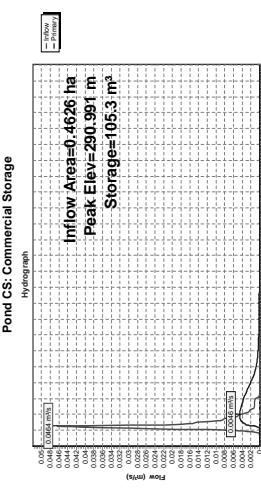
Summary for Pond CS: Commercial Storage

TFRCA AES 6-hr 10-Year Rainfall=56 mm Printed 29/02/2019 Page 38		
Inflow Area = 0.04262 ha, 74,120 m ² , Impervious, Inflow Depth = 30 mm, for 10-year event 0.046 m ³ @ 1.06 m/s, Volume= 41.05 m ³ , Atnet= 0%, ag= 31.1 min Outflow Primary = 0.046 m ³ @ 4.08 m/s, Volume= 0.093 m ³		
Routing by Slope-Ind method, Time Span= 0.00-48.00 hrs, cfs/ftArea= 0.005 in/s, 3		
Peak Elv@29.391 m @ 4.06 hrs, Surf.Area= 323.3 m ² , Storage= 105.3 m ³		
Plug-Flow detention time= 257.3 min calculated for 0.933 m ³ of inflow		
Center-of-Mass det. time= 238.6 min (404.3 - 173.7)		
Volume Inflow Avail.Storage Storage Description		
#A 290.650 m 0.0 m 94.5 mW x 24.30 mL x 61.61 mH Field A Overall Storage Efficiency= 95.0%		
#2A 290.650 m 141.8 m ³ 1.336 Inside ft ³ 0-in ft ³ x 40.0% Voids		
ACO StormBrix 1 336 Inside ft ³ 0-in ft ³ x 40.0% Voids		
Inside= 603 mmW x 612 mmH => 0.350 m ² x 1.21 mL = 0.44 m ³ Outside= 603 mmW x 612 mmH => 0.369 m ² x 1.21 mL = 0.44 m ³		
#3B 290.650 m 0.0 m ³ 142.6 mW x 61.61 mH Field B Overall Storage Efficiency= 95.0%		
#4B 290.650 m 47.3 m ³ 1.667 Inside ft ³ 0-in ft ³ x 40.0% Voids		
ACO StormBrix 1 112 Inside ft ³ 0-in ft ³ x 40.0% Voids		
Inside= 603 mmW x 612 mmH => 0.350 m ² x 1.21 mL = 0.44 m ³ Outside= 603 mmW x 612 mmH => 0.369 m ² x 1.21 mL = 0.44 m ³		
12 Chambers in Rows		
189.1 m ² Total Available Storage		
Storage Group A created with Chamber Wizard		
Storage Group B created with Chamber Wizard		
Device Routing Invert Outfit Devices		
#1 Primary 290.650 m 75 mm Vert. Orifice/Grate C= 0.600		
Primary Outflow Max=0.046 m ³ /s @ 4.06 hrs, Hfl=290.991 m (Free Discharge) ↓=1-Orifice/Rate (Orifice Controls 0.0046 m ³ /s @ 1.04 m/s)		



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Pond CS: Commercial Storage



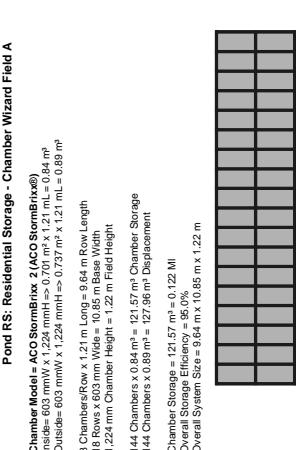
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Summary for Pond RS: Residential Storage

TFRCA AES 6-hr 10-Year Rainfall=56 mm Printed 29/02/2019 Page 40		
Inflow Area = 0.07856 ha, 61.25 m ² , Impermeable, Inflow Depth = 22 mm for 10-Year event Inflow = 0.00745 m ³ /s @ 4.101 hrs, Volume= 0.217 m ³ , Attnet= 90%, Lag= 93.7 min Primary = 0.00776 m ³ /s @ 4.101 hrs, Volume= 0.201 m ³		
Routing by Slope-Ind method, Time Span= 0.00-48.00 hrs, dh= 0.05 hrs, 12		
Peak Elv@291.502 m @ 4.101 hrs, Surf.Area= 322.3 m ² , Storage= 153.8 m ³		
Plug-Flow detention time= 269.5 min calculated for 0.201 m ³ of inflow		
Center-of-Mass det. time= 281.5 min (440.1 - 175.4)		
Volume Invert Avail.Storage Storage Description		
#1A 291.000 m 0.0 m ³ 10.85 mW x 9.64 mL x 121.6 mH Field A ACO StormBrix 2 144 Inside ft ³ 0-in ft ³ x 40.0% Voids		
Inside= 603 mmW x 124 mmH => 0.0701 m ² x 1.21 mL = 0.084 m ³ Outside= 603 mmW x 124 mmH => 0.0737 m ² x 1.21 mL = 0.089 m ³		
#2A 291.000 m 9.44 mW x 24.10 mL x 121.6 mH Field B ACO StormBrix 2 300 Inside ft ³ 0-in ft ³ x 40.0% Voids		
Inside= 603 mmW x 124 mmH => 0.0701 m ² x 1.21 mL = 0.084 m ³ Outside= 603 mmW x 124 mmH => 0.0737 m ² x 1.21 mL = 0.089 m ³		
374.8 m ² Total Available Storage		
Storage Group A created with Chamber Wizard		
Storage Group B created with Chamber Wizard		
Device Routing Invert Outfit Devices		
#1 Primary 291.000 m 75 mm Vert. Orifice/Grate C= 0.600		
Primary Outflow Max=0.0076 m ³ /s @ 1.0 hrs Hfl=291.502 m (Free Discharge) ↓=1-Orifice/Rate (Orifice Controls 0.0076 m ³ /s @ 1.71 m/s)		

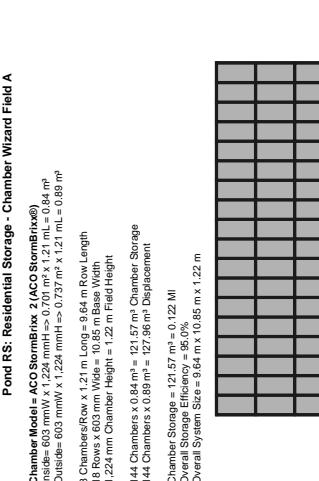
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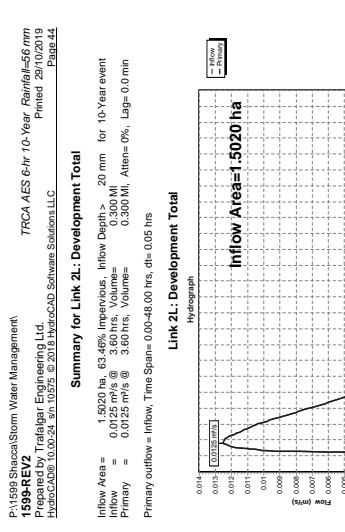
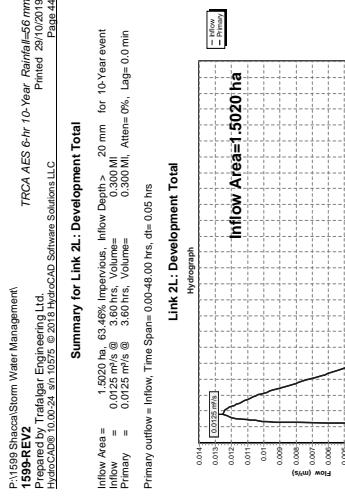
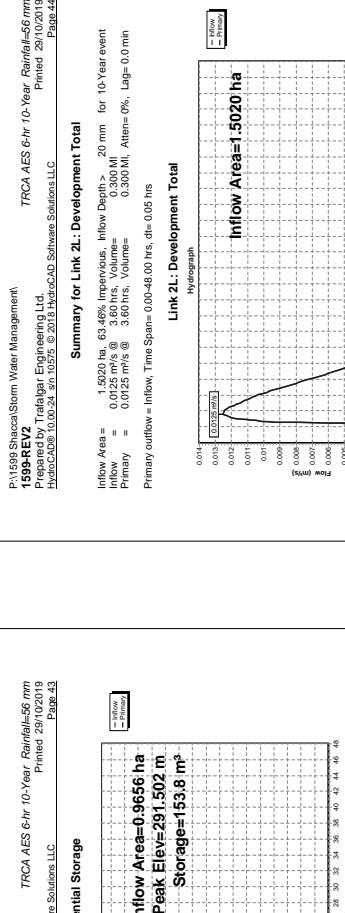
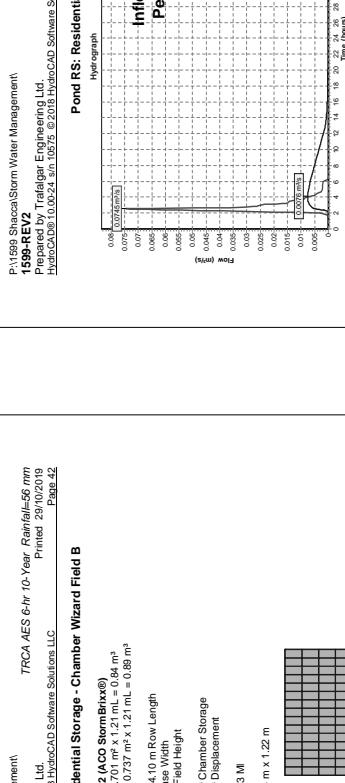
Pond RS: Residential Storage - Chamber Wizard Field A



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Pond RS: Residential Storage - Chamber Wizard Field B



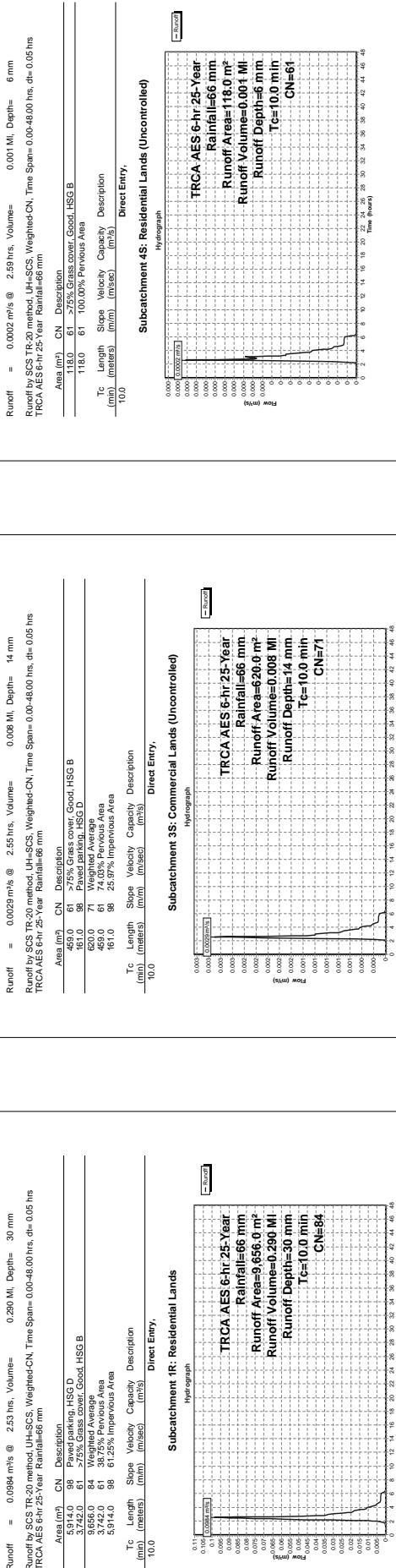


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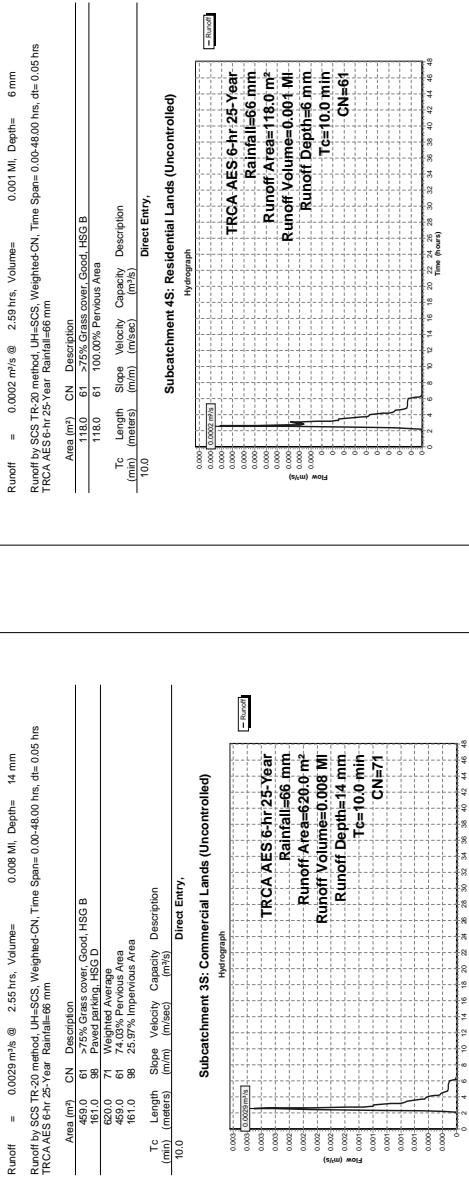
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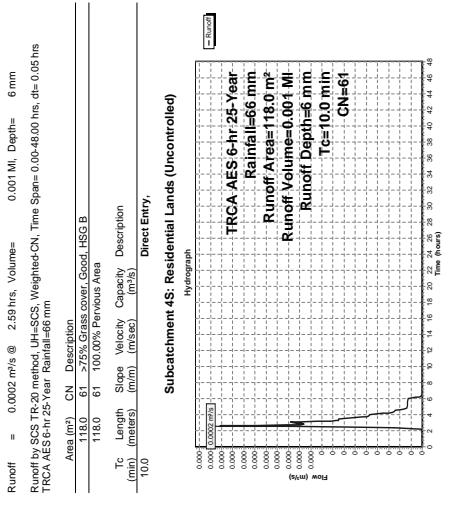
Summary for Subcatchment 1R: Residential Lands



Summary for Subcatchment 3S: Commercial Lands (Uncontrolled)



Summary for Subcatchment 4S: Residential Lands (Uncontrolled)



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Summary for Pond CS: Commercial Storage - Chamber Wizard Field A

Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
8 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³ x 0.047 ml
Overall Storage Efficiency = 96.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
16 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
336 Chambers x 0.42 m = 141.83 m³ Chamber Storage
336 Chambers x 0.44 m = 149.76 m³ Displacement
Chamber Storage = 141.83 m³ x 0.142 ml
Overall Storage Efficiency = 95.0%
Overall System Size = 25.30 m x 9.64m x 0.61 m

Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
8 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³ x 0.047 ml
Overall Storage Efficiency = 96.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
8 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³ x 0.047 ml
Overall Storage Efficiency = 96.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

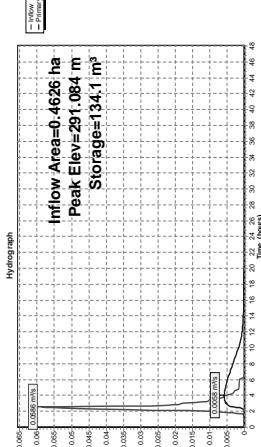
Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
8 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³ x 0.047 ml
Overall Storage Efficiency = 96.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

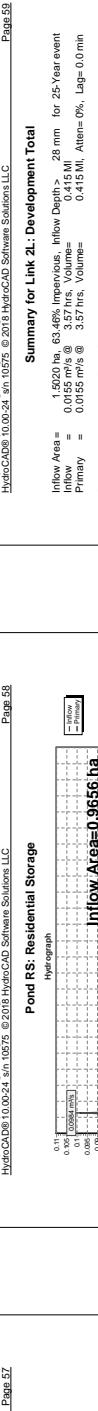
Pond CS: Commercial Storage - Chamber Wizard Field A

Chamber Model = ACO StormBrick 1.1(ACO StormBrick®)
Inside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Outside= 603 mmW x 612 mmH => 0.356 m³ x 121 ml = 0.42 m³
Chamber/Row x 121 m Long x 2.39 m x Row Length
8 Rows x 683 mm Wide x 164 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.42 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³ x 0.047 ml
Overall Storage Efficiency = 96.0%
Overall System Size = 16.87 m x 4.82 m x 0.61 m

Pond CS: Commercial Storage



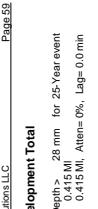
Pond CS: Residential Storage



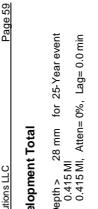
Pond RS: Residential Storage



Pond RS: Residential Storage - Chamber Wizard Field B



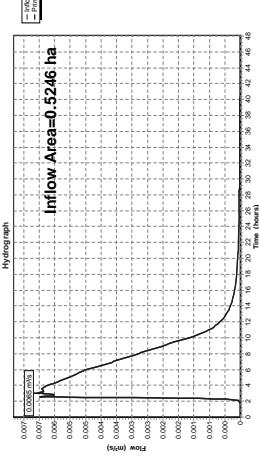
Pond RS: Residential Storage - Chamber Wizard Field A



Summary for Link 3L: Commercial Total

Inflow Area = 0.5546 ha, 68.70% Impervious, Inflow Depth= 27 mm for 25-Year event
Inflow = 0.0052 m³s @ 3.07 hrs, Volume= 0.0141 Ml, Attn= 0%, Lag= 0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs

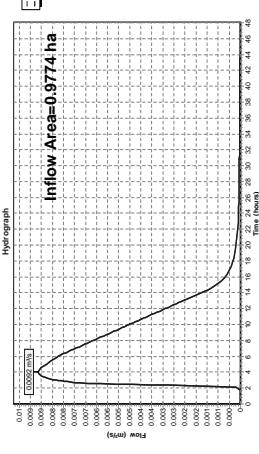
Link 3L: Commercial Total



Summary for Link 4L: Residential Total

Inflow Area = 0.0774 ha, 60.51% Impervious, Inflow Depth= 28 mm for 25-Year event
Inflow = 0.0032 m³s @ 4.07 hrs, Volume= 0.0082 Ml, Attn= 0%, Lag= 0 min
Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs

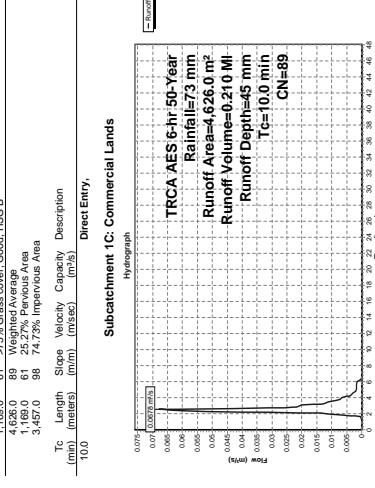
Link 4L: Residential Total



Summary for Subcatchment 1C: Commercial Lands

Runoff = 0.0678 m³s @ 2.53 hrs, Volume= 0.210 Ml, Depth= 45 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs
TRCA AES 6-hr 50-Year Rainfall=73 mm
Area (m²) : CN : Description
3.457.0 98 Paved parking, HSG D
4.635.0 98 75% Grass cover, Good, HSG B
1.168.0 98 25% Pervious Area
3.457.0 98 74.73% Impervious Area
Tc Length Slope Velocity Capacity Description
(min) (metres) (mm) (m/sec) (mm/s) (metres) (mm/sec)
To=10.0 min
CN=89

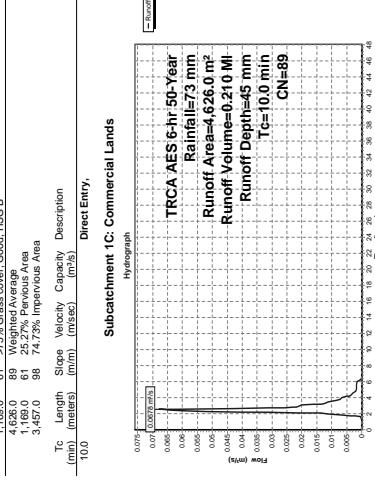
Subcatchment 1C: Commercial Lands



Summary for Subcatchment 3S: Commercial Lands (Uncontrolled)

Runoff = 0.0038 m³s @ 2.55 hrs, Volume= 0.011 Ml, Depth= 18 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs
TRCA AES 6-hr 50-Year Rainfall=73 mm
Area (m²) : CN : Description
18.0 61 75% Grass cover, Good, HSG B
18.0 61 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description
(min) (metres) (mm) (m/sec) (mm/s) (metres) (mm/sec)
To=10.0 min
CN=61

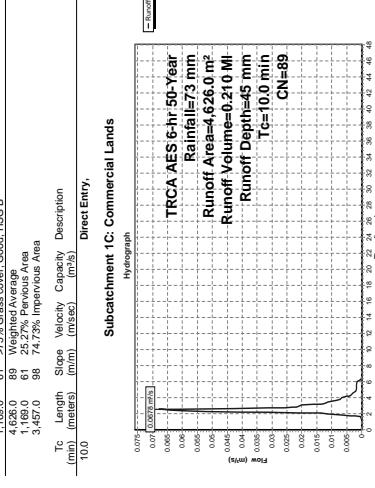
Subcatchment 3S: Commercial Lands (Uncontrolled)



Summary for Subcatchment 4S: Residential Lands (Uncontrolled)

Runoff = 0.0003 m³s @ 2.57 hrs, Volume= 0.001 Ml, Depth= 8 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs
TRCA AES 6-hr 50-Year Rainfall=73 mm
Area (m²) : CN : Description
18.0 61 75% Grass cover, Good, HSG B
18.0 61 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description
(min) (metres) (mm) (m/sec) (mm/s) (metres) (mm/sec)
To=10.0 min
CN=61

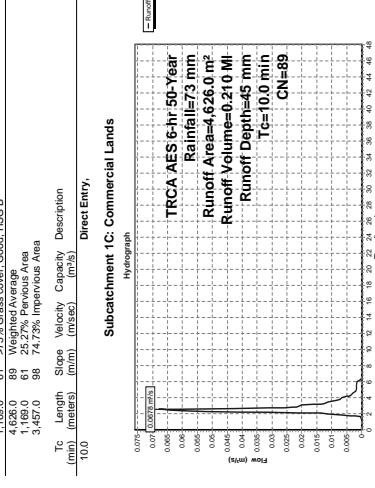
Subcatchment 4S: Residential Lands (Uncontrolled)



Summary for Subcatchment 1R: Residential Lands

Runoff = 0.1167 m³s @ 2.53 hrs, Volume= 0.347 Ml, Depth= 36 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs
TRCA AES 6-hr 50-Year Rainfall=73 mm
Area (m²) : CN : Description
3.742.0 61 275% Grass cover, Good, HSG D
9.656.0 84 Paved Average
3.742.0 61 38.75% Pervious Area
5.914.0 98 61.25% Impervious Area
Tc Length Slope Velocity Capacity Description
(min) (metres) (mm) (m/sec) (mm/s) (metres) (mm/sec)
To=10.0 min
CN=84

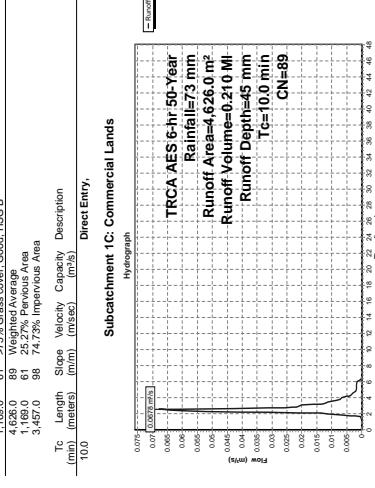
Subcatchment 1R: Residential Lands



Summary for Subcatchment 3C: Commercial Lands (Uncontrolled)

Runoff = 0.0008 m³s @ 2.55 hrs, Volume= 0.001 Ml, Depth= 18 mm
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, ds= 0.05 hrs
TRCA AES 6-hr 50-Year Rainfall=73 mm
Area (m²) : CN : Description
18.0 61 75% Grass cover, Good, HSG B
18.0 61 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description
(min) (metres) (mm) (m/sec) (mm/s) (metres) (mm/sec)
To=10.0 min
CN=61

Subcatchment 3C: Commercial Lands (Uncontrolled)



Summary for Pond CS: Commercial Storage

Inflow Area = 0.03268 ha, 74,320 m², Impervious, Inflow Depth = 10.11 m, 0.55 m, 3.95 hrs, Volume= 16.143 ML, Attent= 0%, ag= 0.75 min
Outflow = 0.0072 m³s @ 16.143 ML, Attent= 0%, ag= 0.75 min
Primary = 0.0067 m³s @ 3.95 hrs, Volume= 16.143 ML
Routing by Sto-4rd method, Time Span= 0.00-48.00 hrs, cte=0.05 hrs, fct= 0.59 hrs, Surface Area= 323.3 m²
Plug-Flow retention time= 272.4 min (calculated for 0.633 M ft³ of inflow)
Center-of-Mass det. time= 251.3 min (422.1 - 170.8)
Volume = Invert Avail Storage
#1A 290/650 m 0.0 m³ 94.5 mW x 24.30 m³ x 16.11 m Field A
Overall System Efficiency = 0.95%
#2A 290/650 m 141.8 m³ 136 Inside ft³
ACO StormBrix 1 x 336 Inside ft³
Inside= 603 mm/W x 612 mm/H => 0.350 m² x 12.1 ml = 0.44 m³
Outside= 603 mm/W x 612 mm/H => 0.369 m² x 12.1 ml = 0.44 m³
#3B 290/650 m 0.0 m³ 94.5 mW x 24.30 m³ x 16.11 m Field B
ACO StormBrix 1 x 336 Inside ft³
Inside= 603 mm/W x 612 mm/H => 0.350 m² x 12.1 ml = 0.44 m³
Outside= 603 mm/W x 612 mm/H => 0.369 m² x 12.1 ml = 0.44 m³
#4B 290/650 m 47.3 m³ 112 Inside ft³
ACO StormBrix 1 x 112 Inside ft³
Inside= 603 mm/W x 612 mm/H => 0.350 m² x 12.1 ml = 0.44 m³
Outside= 603 mm/W x 612 mm/H => 0.369 m² x 12.1 ml = 0.44 m³
12 Chambers in 1 Total Available Storage

189.1 m³ Total Available Storage

Storage Group A created with Chamber Wizard

Storage Group B created with Chamber Wizard

Device Routing Invert Outlet Devices

#1 Primary 290/650 m 75 mm Vert. Orifice/Grate C= 0.600

#2-1 Orifice/Grate Orifice Controls 0.0067 m³/s @ 1.51 m/s

Peak Elev=291.59 m

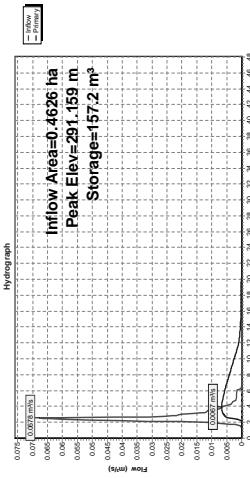
Storage=157.2 m³

Hydrograph

Time (hours)



Pond CS: Commercial Storage



Total Available Storage

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Routing Invert Outlet Devices
#1 Primary 291/050 m 75 mm Vert. Orifice/Grate C= 0.600
#2-1=Orifice/Grate Orifice Controls 0.0102 m³/s @ 1.2 hrs HVA=291.838 m (Free Discharge)

Primary Outflow Max=0.0102 m³/s @ 1.2 hrs

C= 0.600

Overall System Size = 0.102 m³/s

Overall System Efficiency = 98.0%

Overall System Size = 16.07 m x 4.42 m x 0.61 m

Overall System Efficiency = 98.0%

Overall System Size = 16.07 m x 4.42 m x 0.61 m

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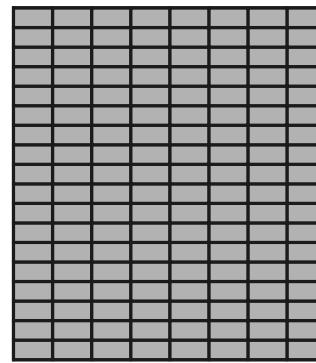
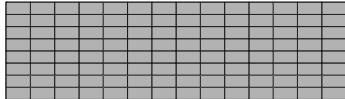
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Pond CS: Commercial Storage - Chamber Wizard Field B

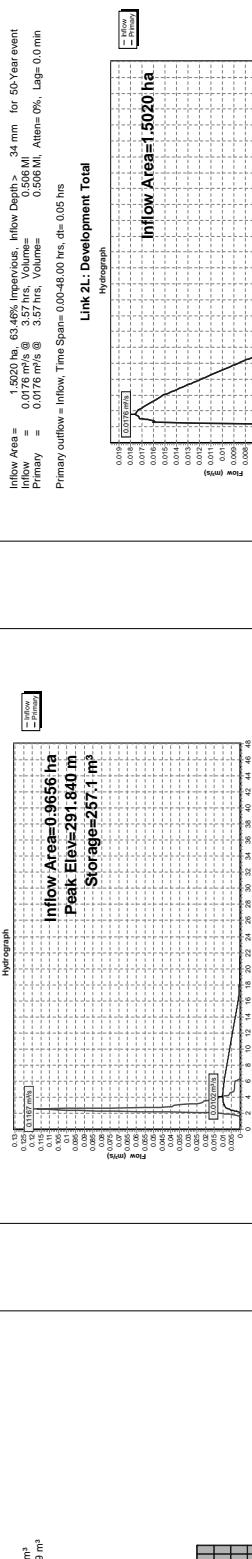
Chamber Model = ACO StormBrix 1 (ACO StormBrix(B))
Inside= 603 mm/W x 12.1 mm/H => 0.350 m² x 12.1 ml = 0.44 m³
Outside= 603 mm/W x 12.1 mm/H => 0.369 m² x 12.1 ml = 0.44 m³
21 Chambers/Row x 12.1 m Long = 16.87 m Row Length
16 Rows x 603 mm Wide = 682 m Base Width
612 mm Chamber Height = 0.61 m Field Height
112 Chambers x 0.44 m = 47.28 m³ Chamber Storage
112 Chambers x 0.44 m = 49.76 m³ Displacement
Chamber Storage = 47.28 m³, 0.047 ml
Overall Storage Efficiency = 98.0%
Overall System Size = 16.07 m x 4.42 m x 0.61 m



P:\1599\ShaeffelStorm Water Management
1599-REV2 Prepared by Trifagor Engineering Ltd.
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T/RCRA AES 6-hr 50-Year Rainfall=73 mm
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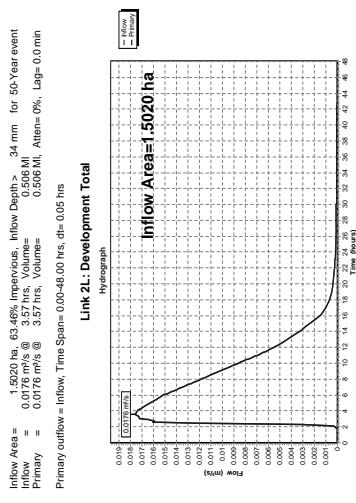
Pond RS: Residential Storage - Chamber Wizard Field B



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T/RCRA AES 6-hr 50-Year Rainfall=73 mm
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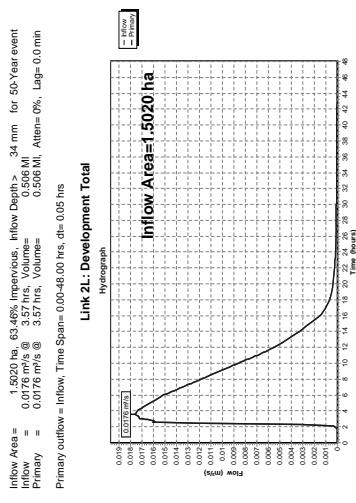
Pond RS: Residential Storage - Chamber Wizard Storage



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T/RCRA AES 6-hr 50-Year Rainfall=73 mm
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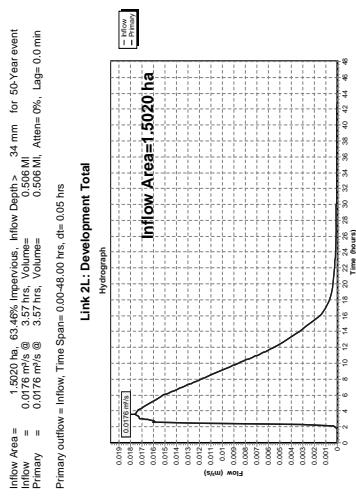
Summary for Link 2L: Development Total



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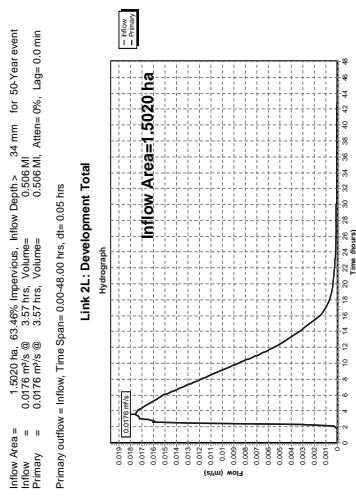
Summary for Subcatchment 1C: Commercial Lands



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T/RCRA AES 6-hr 100-Year Rainfall=80 mm
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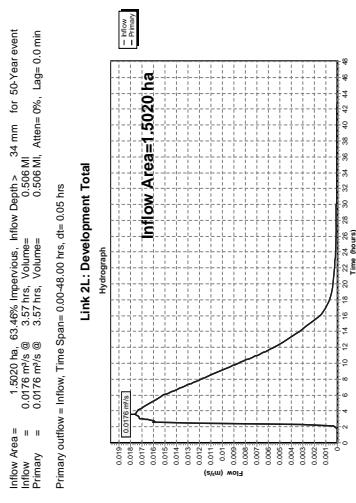
Summary for Subcatchment 1C: Commercial Lands



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T/RCRA AES 6-hr 100-Year Rainfall=80 mm
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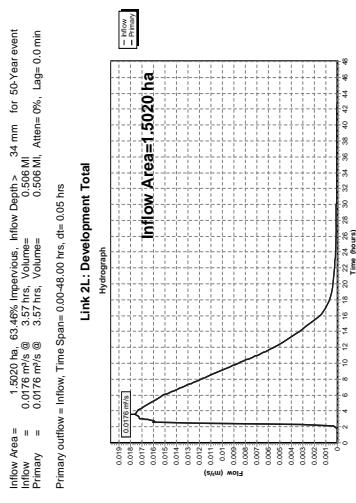
Summary for Link 4L: Residential Total



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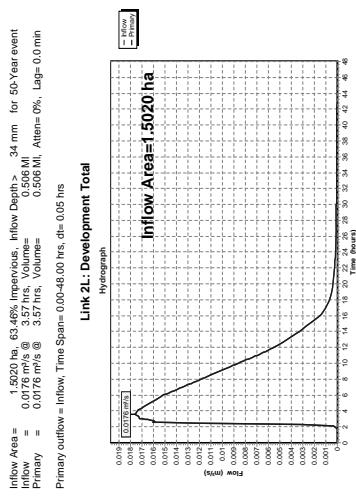
Summary for Link 3L: Commercial Total



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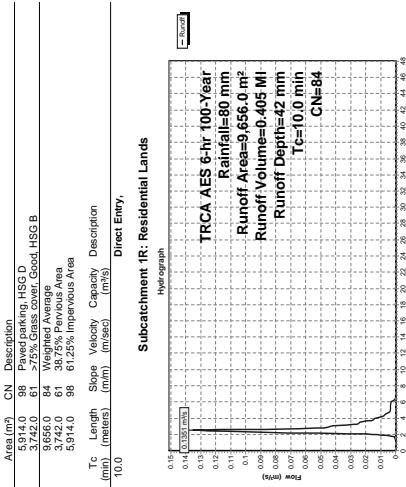
T/RCRA AES 6-hr 100-Year Rainfall=80 mm
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Summary for Link 3L: Commercial Total



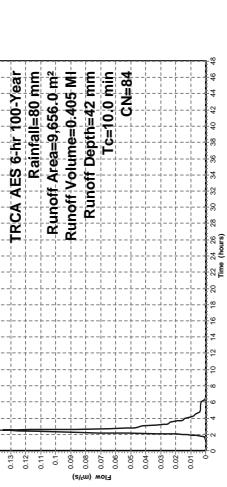
Summary for Subcatchment 1A: Residential Lands

Runoff = 0.1351 m³s @ 2.53 hrs. Volume= 0.405 MI. Depth= 42 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



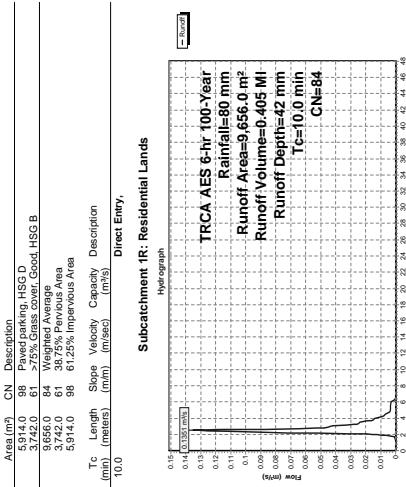
Subcatchment 1B: Residential Lands

Runoff = 0.1347 m³s @ 2.53 hrs. Volume= 0.405 MI. Depth= 42 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



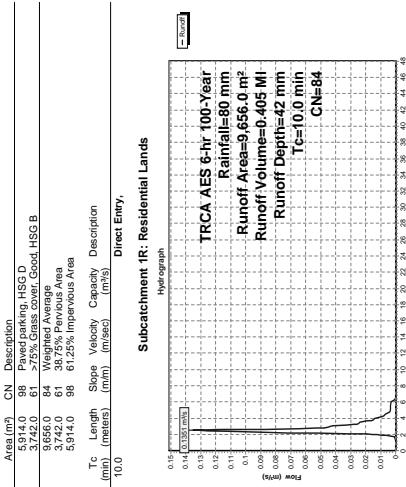
Summary for Subcatchment 3A: Commercial Lands (Uncontrolled)

Runoff = 0.0047 m³s @ 2.53 hrs. Volume= 0.015 MI. Depth= 22 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



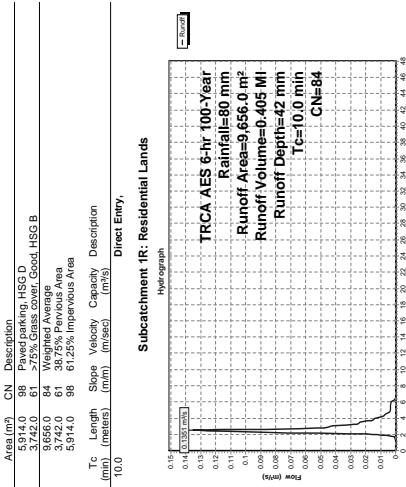
Summary for Subcatchment 3B: Commercial Lands (Uncontrolled)

Runoff = 0.0047 m³s @ 2.53 hrs. Volume= 0.015 MI. Depth= 22 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



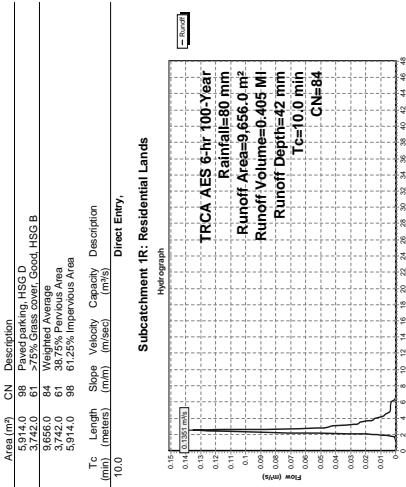
Summary for Subcatchment 4A: Residential Lands (Uncontrolled)

Runoff = 0.0004 m³s @ 2.57 hrs. Volume= 0.001 MI. Depth= 11 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



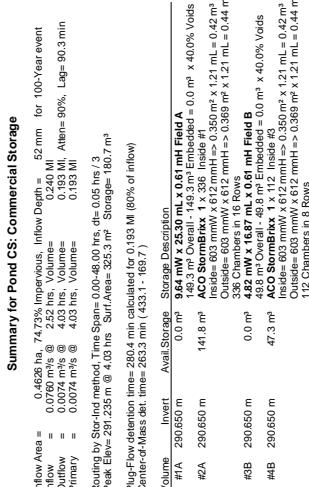
Summary for Subcatchment 4B: Residential Lands (Uncontrolled)

Runoff = 0.0004 m³s @ 2.57 hrs. Volume= 0.001 MI. Depth= 11 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



Summary for Pond CS: Commercial Storage - Chamber Wizard Field A

Inflow Area = 0.4626 m² @ 74.3% Impervious. Inflow Depth = 52.00 mm for 100-Year event
Inflow = 0.0760 m³s @ 2.52 hrs. Volume= 0.124 MI. Attnet = 90%. Lag= 90.3 min
Outflow = 0.0074 m³s @ 4.03 hrs. Volume= 0.0374 MI. Attnet = 90%. Lag= 90.3 min
Routing by Step-Ind method 1 hr. Time= 0.00-48.00 hrs, dt= 0.05 hrs / 3
Peak Elevation= 29.25 m. @ 4.03 hrs. Surf.Area= 232.3 m². Storage= 180.7 m³
Plug-Play detention time= 286.3 min (333.1 - 169.7)
Center of Mass set. time= 263.3 min (333.1 - 169.7)



Storage Description

Volume = 290.650 m³ Overall = 0.0 m³ x 1.61 m Field A

#A 290.650 m 141.8 m³ Overall = 141.3 m³ Embodied = 0.0 m³ x 0.0% Voids
Inside= 603 mmW x 12.0 mmH => 0.150 m³ x 1.21 ml = 0.42 m³
Outside= 603 mmW x 16.2 mmH => 0.369 m³ x 1.21 ml = 0.44 m³

#B 290.650 m 0.0 m³ Overall = 0.0 m³ x 1.61 m Field B

#B 290.650 m 47.3 m³ Overall = 49.8 m³ Embodied = 0.0 m³ x 40.0% Voids
Inside= 603 mmW x 12.0 mmH => 0.150 m³ x 1.21 ml = 0.42 m³
Outside= 603 mmW x 16.2 mmH => 0.369 m³ x 1.21 ml = 0.44 m³

#B 290.650 m 188.1 m³ Total Available Storage

Storage Group A created with Chamber Wizard

Storage Group B created with Chamber Wizard

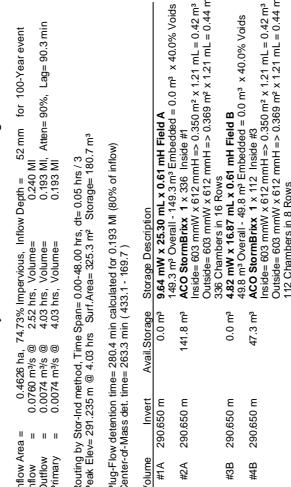
Device Route# Invert Outlet Device# C= 0.600

Primary Outflow Max=0.0074 m³s @ 4.03 hrs H=29.25 m (Free Discharge)

#1 Orifice/Drate Orifice Controls 0.0074 m³s @ 1.68 m/s

Summary for Subcatchment 4S: Residential Lands (Uncontrolled)

Runoff = 0.0004 m³s @ 2.57 hrs. Volume= 0.001 MI. Depth= 11 mm
Runoff by SCS-TRB-20 method UH+SCS, Weighted-CN. Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
TRCA AES 6-hr 100-Year Rainfall=80 mm



Storage Description

Volume = 290.650 m³ Overall = 0.0 m³ x 1.61 m Field A

#A 290.650 m 141.8 m³ Overall = 141.3 m³ Embodied = 0.0 m³ x 0.0% Voids
Inside= 603 mmW x 12.0 mmH => 0.150 m³ x 1.21 ml = 0.42 m³
Outside= 603 mmW x 16.2 mmH => 0.369 m³ x 1.21 ml = 0.44 m³

#B 290.650 m 0.0 m³ Overall = 0.0 m³ x 1.61 m Field B

#B 290.650 m 47.3 m³ Overall = 49.8 m³ Embodied = 0.0 m³ x 40.0% Voids
Inside= 603 mmW x 12.0 mmH => 0.150 m³ x 1.21 ml = 0.42 m³
Outside= 603 mmW x 16.2 mmH => 0.369 m³ x 1.21 ml = 0.44 m³

#B 290.650 m 188.1 m³ Total Available Storage

Storage Group A created with Chamber Wizard

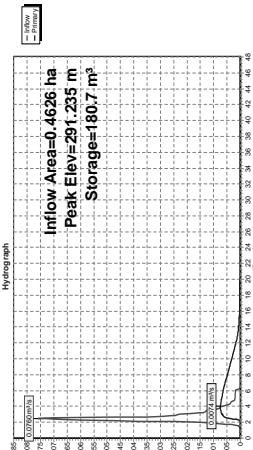
Storage Group B created with Chamber Wizard

Device Route# Invert Outlet Device# C= 0.600

Primary Outflow Max=0.0074 m³s @ 4.03 hrs H=29.25 m (Free Discharge)

#1 Orifice/Drate Orifice Controls 0.0074 m³s @ 1.68 m/s

Pond CS: Commercial Storage



Summary for Pond RS: Residential Storage

	Inflow	Avg. Storage	Storage Description
#IA	291.000 m ³	1048.000 m ³	1048 mW, 9.64 mW x 122 mH Field A
#A1	291.000 m ³	121.6 m ³	ACO StormBoxx 2 x 122 mH Inside = 0.001 m ³ x 40.0% Voids
#A2	291.000 m ³	0.000 m ³	ACO StormBoxx 2 x 122 mH Inside = 0.001 m ³ x 40.0% Voids
#B1	291.000 m ³	0.0 m ³	144 Chambers in 18 Rows
#B2	291.000 m ³	0.0 m ³	144 Chambers in 18 Rows
#AB	291.000 m ³	253.3 m ³	ACO StormBoxx 2 x 120 mH Inside = 0.001 m ³ x 40.0% Voids
			Outsize= 603 mm/W x 1224 mm/H => 0.737 m ² x 121 mL = 0.89 m ³
			360 Chambers in 15 Rows
			374.8 m ³ Total Available Storage

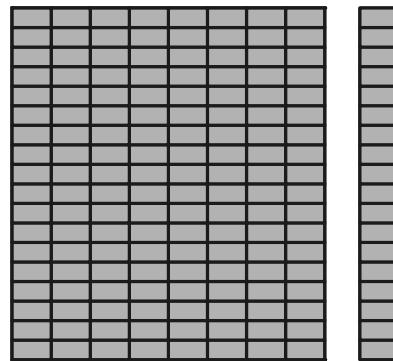
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Routing
Invert: Outflow Devices
#1 Primary
#2 OffFlow Max=0.012 m³/s @ 13 hrs H=21.984 m (Free Discharge)
#3 OffFlow Max=0.012 m³/s @ 2.53 m/s (Free Discharge)

Pond RS: Residential Storage - Chamber Wizard Field A

Chamber Model = ACO StormBoxx 2 (ACO StormBoxx®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 mL = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 mL = 0.89 m³

8 Chamber Row x 121 m Long = 9.64 m Row Length
18 Rows x 603 mm Width = 10.85 m Base Width
1.224 mm Chamber Height = 122 m Field Height
144 Chambers x 0.84 m = 121.57 m³ Chamber Storage
144 Chambers x 0.84 m = 121.57 m³ Displacement

Chamber Storage = 121.57 m³ ml = 0.22 Ml
Overall Storage Efficiency = 36.0%
Overall System Size = 9.64 m x 10.85 m x 1.22 m

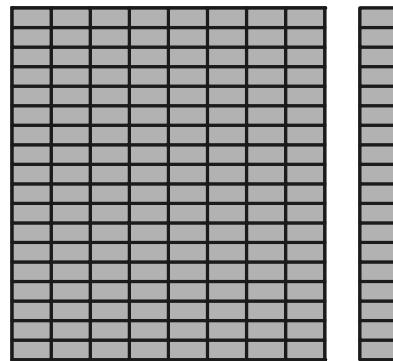


Pond RS: Residential Storage - Chamber Wizard Field B

Chamber Model = ACO StormBoxx 2 (ACO StormBoxx®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 mL = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 mL = 0.89 m³

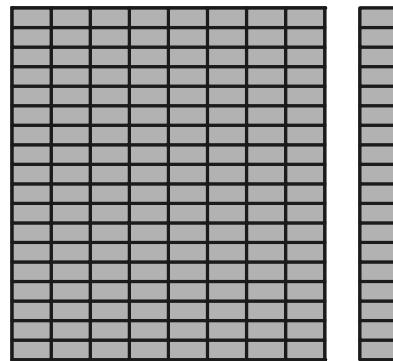
15 Rows x 603 mm Width = 12.21 m Base Width
1.224 mm Chamber Height = 122 m Field Height
300 Chambers x 0.84 m = 252.36 m³ Chamber Storage
300 Chambers x 0.84 m = 265.69 m³ Displacement

Chamber Storage = 252.26 m³ = 0.533 Ml
Overall Storage Efficiency = 95.0%
Overall System Size = 24.10 m x 9.04 m x 1.22 m



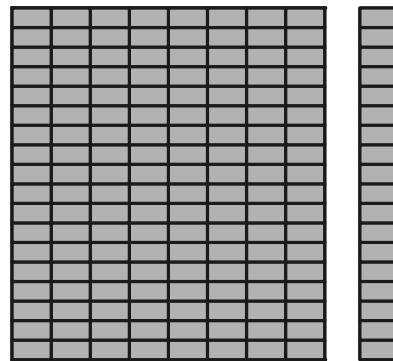
Summary for Link 2L: Development Total

Link 2L: Development Total
Inflow Area = 1.5020 ha
Inflow = 0.0196 m³/s @ 3.56 hrs, Volume= 0.537 Ml, Attent= 0%
Primary = 0.0195 m³/s @ 3.56 hrs, Volume= 0.537 Ml, Attent= 0%
Primary outflow = Inflow, Time Span=0.48-0.00 hrs, dis= 0.05 ms



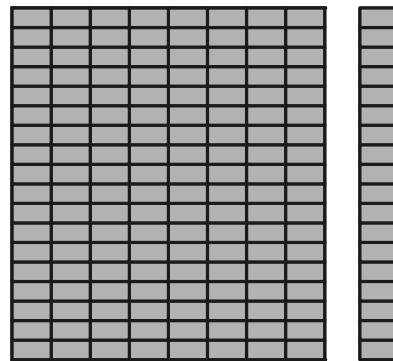
Link 2L: Development Total

Link 2L: Development Total
Inflow Area = 1.5020 ha
Inflow = 0.0196 m³/s @ 3.56 hrs, Volume= 0.537 Ml, Attent= 0%
Primary = 0.0195 m³/s @ 3.56 hrs, Volume= 0.537 Ml, Attent= 0%
Primary outflow = Inflow, Time Span=0.48-0.00 hrs, dis= 0.05 ms



Pond CS: Residential Storage

Pond CS: Residential Storage
Hydrograph

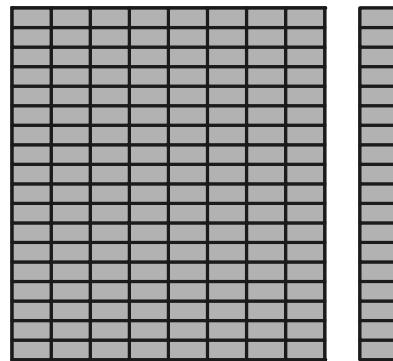


Pond RS: Residential Storage - Chamber Wizard Field B

Chamber Model = ACO StormBoxx 2 (ACO StormBoxx®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 mL = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 mL = 0.89 m³

15 Rows x 603 mm Width = 12.21 m Long = 24.0 m Row Length
1.224 mm Chamber Height = 122 m Field Height
300 Chambers x 0.84 m = 252.36 m³ Chamber Storage
300 Chambers x 0.84 m = 265.69 m³ Displacement

Chamber Storage = 252.26 m³ = 0.533 Ml
Overall Storage Efficiency = 95.0%
Overall System Size = 24.10 m x 9.04 m x 1.22 m

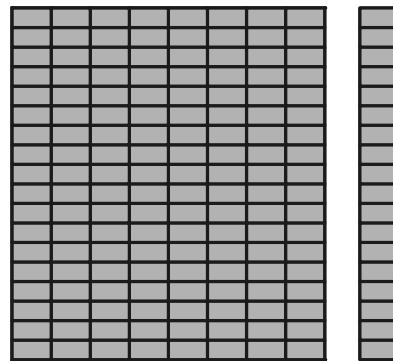


Pond RS: Residential Storage - Chamber Wizard Field A

Chamber Model = ACO StormBoxx 2 (ACO StormBoxx®)
Inside= 603 mm/W x 1224 mm/H => 0.701 m² x 121 mL = 0.84 m³
Outside= 603 mm/W x 1224 mm/H => 0.737 m² x 121 mL = 0.89 m³

144 Chambers x 0.84 m = 121.57 m³ Chamber Storage
144 Chambers x 0.84 m = 121.57 m³ Displacement

Chamber Storage = 121.57 m³ ml = 0.22 Ml
Overall Storage Efficiency = 36.0%
Overall System Size = 9.64 m x 10.85 m x 1.22 m



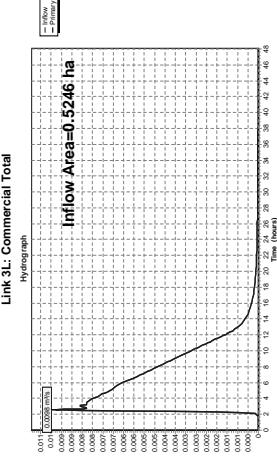
P1159 ShocaStorm Water Management
159-RV2 Trafliger Engineering Ltd.
 Prepared by Trafliger Engineering Ltd.
 Printed 29/02/2019
 Page 90

P1159 ShocaStorm Water Management
159-RV2 TRCA AES 6-hr 100-Year Rainfall=80 mm
 Prepared by Trafliger Engineering Ltd.
 Printed 29/02/2019
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HydroCAD 11.0.0.24.5in 10575 © 2018 HydroCAD Software Solutions LLC
 Printed 29/02/2019
 Page 91

Summary for Link 3L: Commercial Total

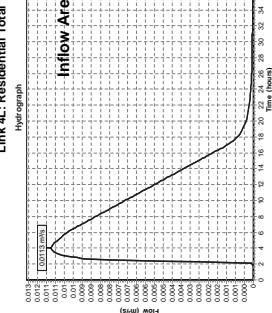
Inflow Area = 0.5546 ha, 68.87% Impervious, Inflow Depth > 39 mm for 100-Year event
 Inflow = 0.0388 m³/s @ 2.55 hrs, Volume= 0.277 ML, Atten= 0%, Ldg= 0.0 min
 Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dte: 0.05 hrs



Summary for Link 4L: Residential Total

Inflow Area = 0.9774 ha, 60.51% Impervious, Inflow Depth= 40 mm for 100-Year event
 Inflow = 0.0331 m³/s @ 4.077 hrs, Volume= 0.386 ML, Atten= 0%, Ldg= 0.0 min
 Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dte: 0.05 hrs

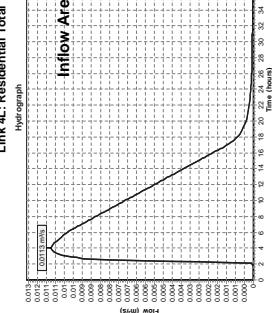
Link 4L: Residential Total



Summary for Link 4L: Residential Total

Inflow Area = 0.9774 ha, 60.51% Impervious, Inflow Depth= 40 mm for 100-Year event
 Inflow = 0.0331 m³/s @ 4.077 hrs, Volume= 0.386 ML, Atten= 0%, Ldg= 0.0 min
 Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dte: 0.05 hrs

Link 4L: Residential Total



Brief Stormceptor Sizing Report - Commercial Site

Project Information & Location			
Project Name	16114 Airport Road	Project Number	1599
City	Caledon	State/ Province	Ontario
Country	Canada	Date	2/8/2018
Designer Information		EOR Information (optional)	
Name	James Nelson	Name	
Company	Trafalgar Engineering Ltd	Company	
Phone #	905-338-3366	Phone #	
Email	jnelson@trafalgareng.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	Commercial Site
Target TSS Removal (%)	80
TSS Removal (%) Provided	
Recommended Stormceptor Model	STC 750

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 300	70
STC 750	80
STC 1000	81
STC 1500	82
STC 2000	84
STC 3000	86
STC 4000	89
STC 5000	90
STC 6000	91
STC 9000	94
STC 10000	94
STC 14000	95
StormceptorMAX	Custom

Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (ha)	0.46	TSS Removal (%)	80.0
Imperviousness %	85.00	Runoff Volume Capture (%)	
Rainfall			
Station Name	TORONTO CENTRAL	Oil Spill Capture Volume (L)	
State/Province	Ontario	Peak Conveyed Flow Rate (L/s)	
Station ID #	0100	Water Quality Flow Rate (L/s)	
Up Stream Storage			
Years of Records	18	Storage (ha-m)	Discharge (cms)
Latitude	43°37'N	0.000	0.000
Up Stream Flow Diversion			
Longitude	79°23'W	Max. Flow to Stormceptor (cms)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
City of Toronto PSD		
Particle Diameter (microns)	Distribution %	Specific Gravity
10.0	20.0	2.65
30.0	10.0	2.65
50.0	10.0	2.65
95.0	20.0	2.65
265.0	20.0	2.65
1000.0	20.0	2.65

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<http://www.imbriumsystems.com/technical-specifications>

Brief Stormceptor Sizing Report - Residential Component

Project Information & Location			
Project Name	16114 Airport Road	Project Number	1599
City	Caledon	State/ Province	Ontario
Country	Canada	Date	2/8/2018
Designer Information		EOR Information (optional)	
Name	James Nelson	Name	
Company	Trafalgar Engineering Ltd	Company	
Phone #	905-338-3366	Phone #	
Email	jnelson@trafalgareng.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	Residential Component
Target TSS Removal (%)	80
TSS Removal (%) Provided	81
Recommended Stormceptor Model	STC 2000

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary		
Stormceptor Model	% TSS Removal Provided	% Runoff Volume Captured Provided
STC 300	65	80
STC 750	75	90
STC 1000	77	90
STC 1500	77	90
STC 2000	81	94
STC 3000	82	94
STC 4000	86	97
STC 5000	86	97
STC 6000	88	98
STC 9000	91	99
STC 10000	91	99
STC 14000	94	100
StormceptorMAX	Custom	Custom

Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (ha)	0.97	TSS Removal (%)	80.0
Imperviousness %	70.00	Runoff Volume Capture (%)	90.00
Rainfall			
Station Name	TORONTO CENTRAL	Oil Spill Capture Volume (L)	
State/Province	Ontario	Peak Conveyed Flow Rate (L/s)	
Station ID #	0100	Water Quality Flow Rate (L/s)	
Up Stream Storage			
Years of Records	18	Storage (ha-m)	Discharge (cms)
Latitude	43°37'N	0.000	0.000
Up Stream Flow Diversion			
Longitude	79°23'W	Max. Flow to Stormceptor (cms)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
Fine Distribution		
Particle Diameter (microns)	Distribution %	Specific Gravity
20.0	20.0	1.30
60.0	20.0	1.80
150.0	20.0	2.20
400.0	20.0	2.65
2000.0	20.0	2.65

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<http://www.imbriumsystems.com/technical-specifications>

Appendix 'E'

Caledon East Wetland Complex Catchment Basin

Caledon East Wetland Complex Roll #21240500010570000000

Pre-Development Model Subcatchment Areas (Figure 3)

Post-Development Model Subcatchment Areas (Figure 4)

Model Parameters

Interpreted Groundwater Flow (R.J. Burnside)

EPA SWMM Model Input File

Caledon East Wetland Complex Catchment Basin



SOURCE OF INFORMATION

Information provided by the Ministry of Natural Resources & Forestry district office in Aurora.
Ministry of Natural Resources & Forestry - Aurora District
50 Bloomington Road West, Aurora, ON L4G 3G8

Base information derived from the Ontario Base Map, 1983 at a scale of 1:10,000 and the Natural Resources Values Information System (NRVIS).

PLEASE NOTE

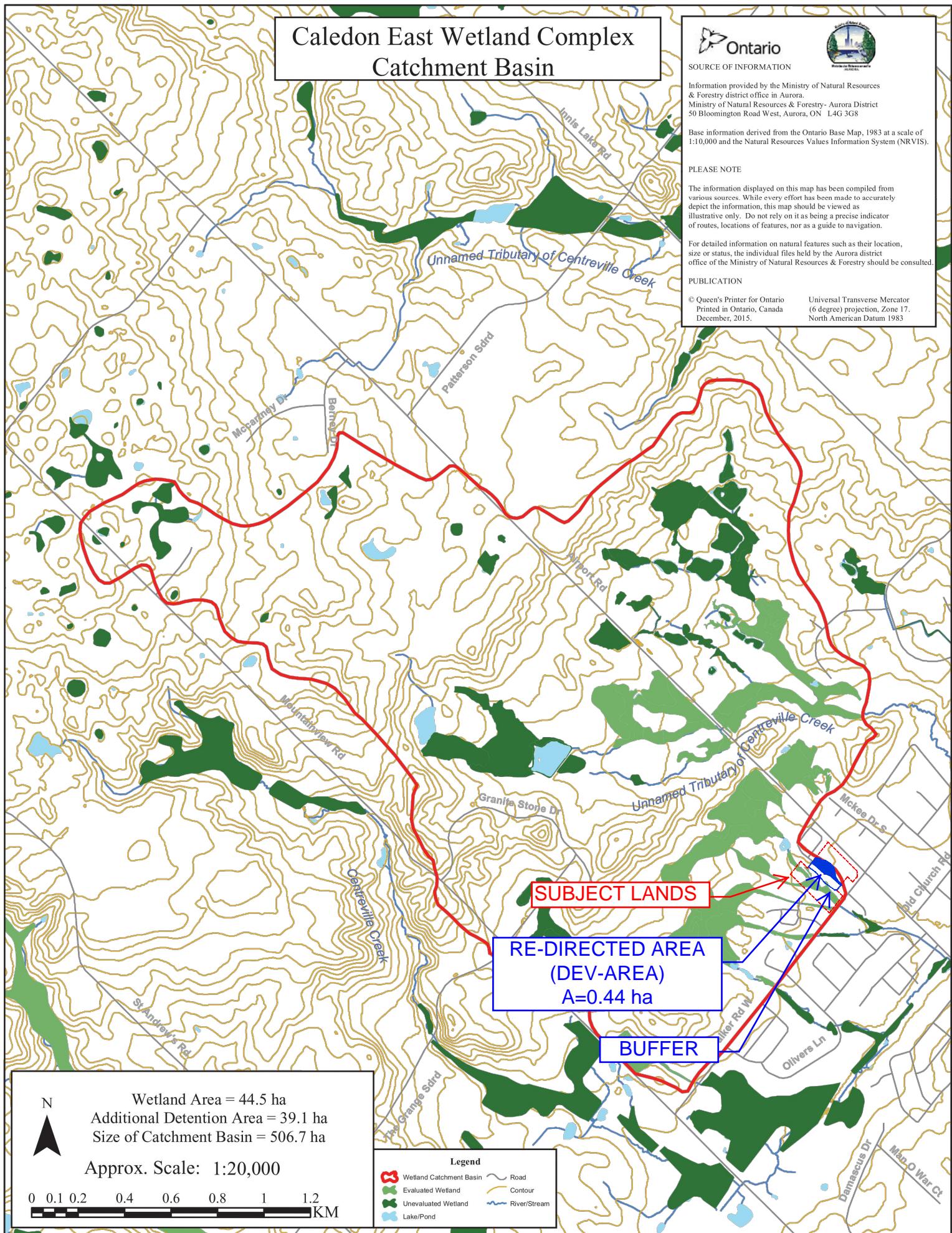
The information displayed on this map has been compiled from various sources. While every effort has been made to accurately depict the information, this map should be viewed as illustrative only. Do not rely on it as being a precise indicator of routes, locations of features, nor as a guide to navigation.

For detailed information on natural features such as their location, size or status, the individual files held by the Aurora district office of the Ministry of Natural Resources & Forestry should be consulted.

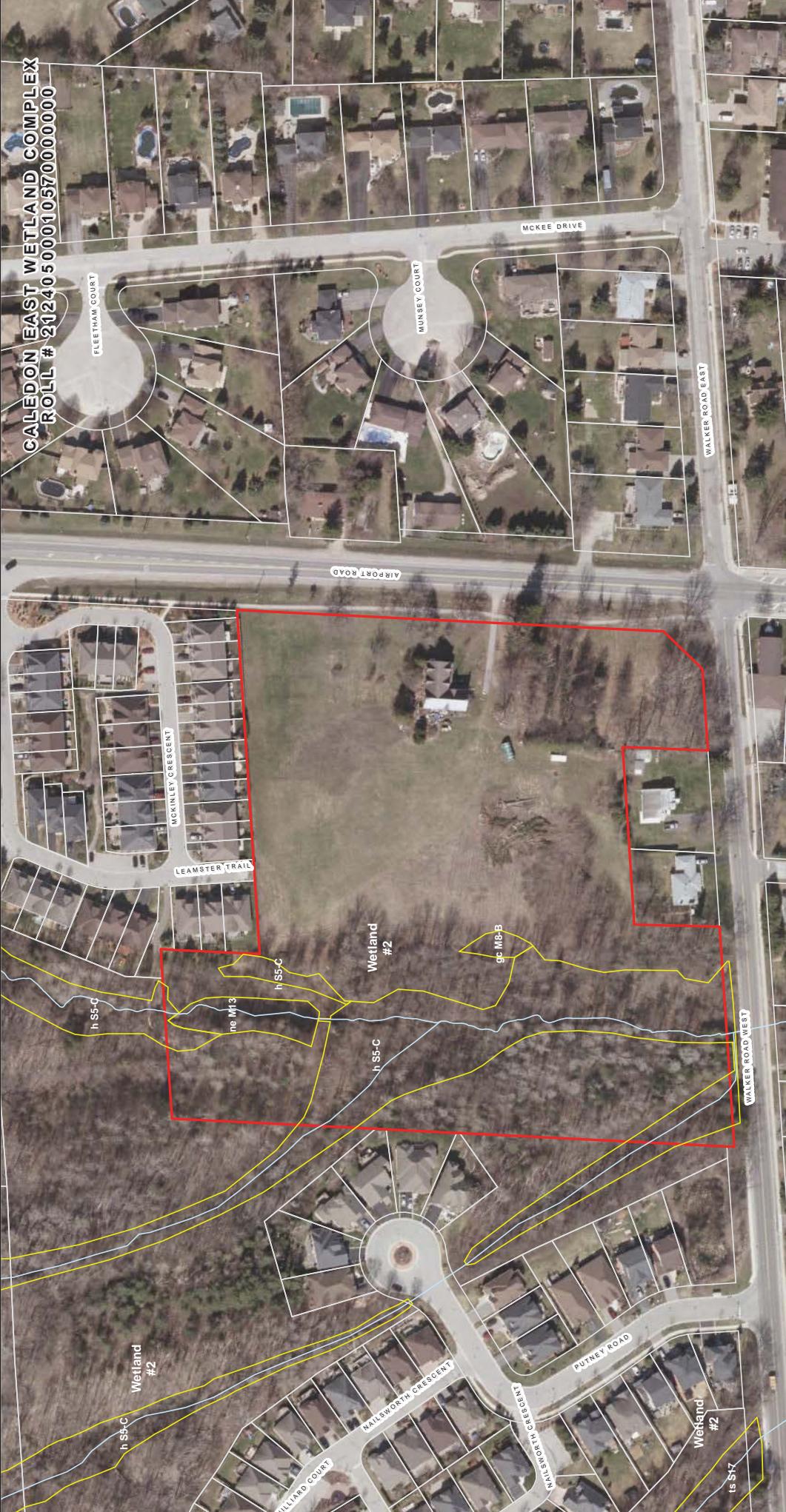
PUBLICATION

© Queen's Printer for Ontario
Printed in Ontario, Canada
December, 2015.

Universal Transverse Mercator
(6 degree) projection, Zone 17.
North American Datum 1983



CALEDON EAST WETLAND COMPLEX
ROLL # 21240500010570000000



Scale 1:1,500 (approx.)

0 25 50 100 150
M



Ontario

SOURCE OF INFORMATION

Information provided by the Ministry of Natural Resources & Forestry district office in Aurora, Ministry of Natural Resources & Forestry-Aurora District Soil Management Unit West, Aurora, ON L4G 3G8. Base information derived from the Ontario Base Map, 1983 at a scale of 1:10,000 and the Natural Resources Values Information System (NRVS).

NOTE

The information displayed on this map has been compiled from various sources. While every effort has been made to accurately depict the information, it is not intended to be a precise indicator of actual features, locations or boundaries. For detailed information on natural features such as their location, size or status, the individual files held by the Aurora Street Office of the Ministry of Natural Resources & Forestry should be consulted. Imagery capture date Spring 2013 copyright, J.D. Barnes and Land Information Ontario

PUBLICATION
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Printed in Ontario, Canada
November 2015.

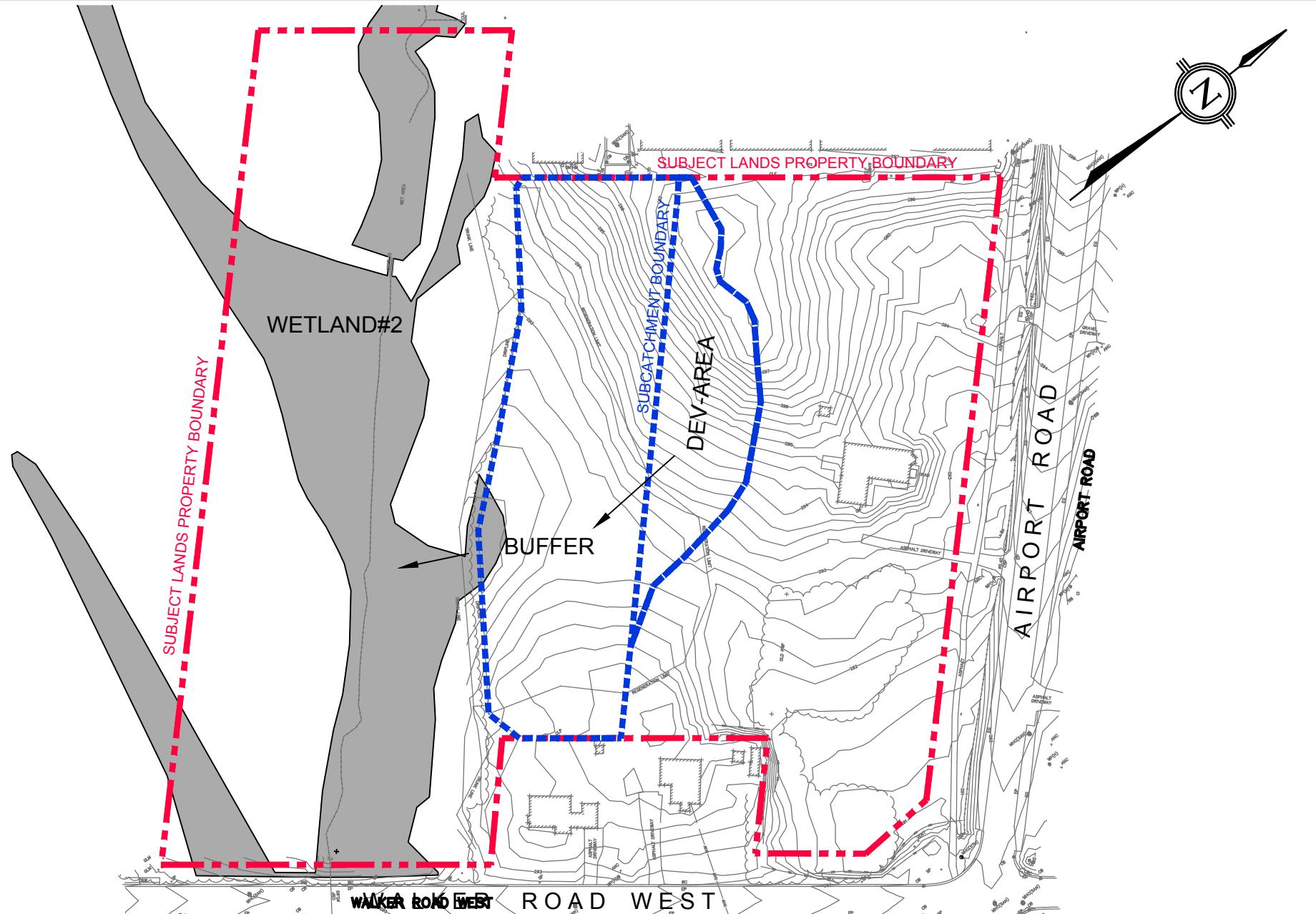
Cartography by Aurora District
Geomatics.

Universal Transverse Mercator
(6 degree) projection Zone 17.
North American Datum 1983

Legend
 Watercourse
 MNRF Evaluated Wetland
 Subject Lands
 Parcel Fabric
 Wetland Vegetation
 Community
c S17

Imagery capture date Spring 2013 copyright, J.D. Barnes and Land Information Ontario

FILE ID 2015-763



PROJECT TITLE

16114 AIRPORT ROAD TOWN OF CALEDON

DRAWING TITLE

PRE-DEVELOPMENT SUBCATCHMENT AREA PLAN

TRAFalgar Engineering Ltd.

TEL: (905) 338-3366 FAX: (905) 338-7734
tel@trafalgareng.com

DESIGN BY

JN

SCALE

1:1500

FIGURE No.

FIGURE 3

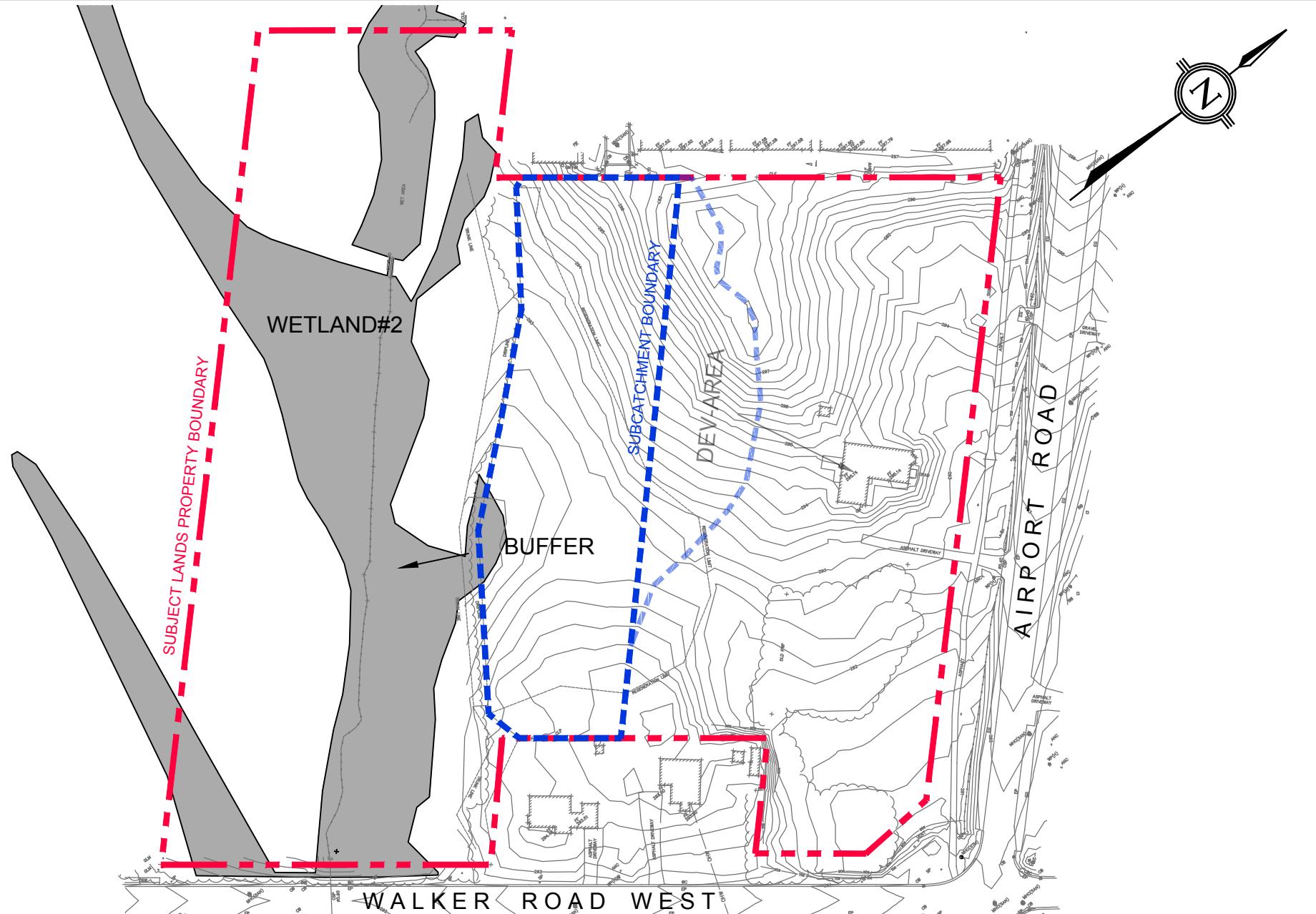
DRAWN BY

SP

CAD FILE 1599-FIG2-3.DWG

DATE

08/02/2018



PROJECT TITLE

16114 AIRPORT ROAD
TOWN OF CALEDON

DRAWING TITLE

POST-DEVELOPMENT SUBCATCHMENT AREA PLAN

TRAFALGAR ENGINEERING LTD.
TEL: (905) 338-3366 FAX: (905) 338-7734
tel@trafalgareng.com

DESIGN BY

JN

SCALE

1:1500

FIGURE No.

FIGURE 3

DRAWN BY

SP

CAD FILE 1599-FIG2-3.DWG

DATE

08/02/2018

Model Parameters

E.1 Groundwater

Groundwater modelling is not a part of the simulation.

E.2 Evaporation

Evaporation is calculated by the software using the Hargreaves method. Reference may be made to the EPA SWMM Reference Manual, Volume I: Hydrology section 4.2 for further detail. The evaporation is determined using the temperature data provided by TRCA.

E.3 Infiltration

Parameters used with the Green-Ampt infiltration method are obtained from Tables 4-7 and 4-8 from the EPA SWMM Reference Manual, attached. Parameter values are interpolated in Table 4-7 based on the hydraulic conductivity of the soil. Typical values of maximum moisture deficit (Initial Deficit) are provided in Table 4-8 from the EPA SWMM Reference Manual based on soil type. The Initial Deficit used in the simulation is selected based on approximate soil type (between Sandy Loam and Loamy Sand, by hydraulic conductivity).

E.4 Rainfall and Climate Data

All rainfall and climate data for the simulation period is obtained from TRCA.

E.5 Subcatchment Geometry and Properties

Subcatchment Properties

Subcatchment	Area (ha)	Width (m)	Slope (%)	% Imp.	n, Imp.	n, Per.	Dep. Storage, Imp. (mm)	Dep. Storage, Per. (mm)
DEV-AREA	0.439	105	9	0	0.015	0.035	2	5
BUFFER	0.441	155	4	0	0.015	0.035	2	5

Infiltration Parameters

Subcatchment	Suction Head (mm)	Hydraulic Conductivity (mm/hr)	Initial Deficit (Fraction)
DEV-AREA	98.4	15.5	0.33
BUFFER	98.4	15.5	0.33

Table 4-7 Green-Ampt parameters for different soil classes (Rawls et al., 1983)(Numbers in parentheses are \pm one standard deviation from the parameter value shown.)

Soil Class	Porosity, ϕ	Effective Porosity, ϕ_e^*	Wetting Front Suction Head, ψ_s (in)	Saturated Hydraulic Conductivity, K_s (in/hr)
Sand	0.437 (0.374–0.500)	0.417 (0.354–0.480)	1.95 (0.38–9.98)	4.74
Loamy sand	0.437 (0.363–0.506)	0.401 (0.329–0.473)	2.41 (0.53–11.00)	1.18
Sandy loam	0.453 (0.351–0.555)	0.412 (0.283–0.541)	4.33 (1.05–17.90)	0.43
Loam	0.463 (0.375–0.551)	0.434 (0.334–0.534)	3.50 (0.52–23.38)	0.13
Silt loam	0.501 (0.420–0.582)	0.486 (0.394–0.578)	6.57 (1.15–37.56)	0.26
Sandy clay loam	0.398 (0.332–0.464)	0.330 (0.235–0.425)	8.60 (1.74–42.52)	0.06
Clay loam	0.464 (0.409–0.519)	0.309 (0.279–0.501)	8.22 (1.89–35.87)	0.04
Silty clay loam	0.471 (0.418–0.524)	0.432 (0.347–0.517)	10.75 (2.23–51.77)	0.04
Sandy clay	0.430 (0.370–0.490)	0.321 (0.207–0.435)	9.41 (1.61–55.20)	0.02
Silty clay	0.479 (0.425–0.533)	0.423 (0.334–0.512)	11.50 (2.41–54.88)	0.02
Clay	0.475 (0.427–0.523)	0.385 (0.269–0.501)	12.45 (2.52–61.61)	0.01

*Effective porosity is the difference between the porosity ϕ and the residual moisture content ϕ_r that remains after a saturated soil is allowed to drain thoroughly.

OBTAINED FROM EPA SWMM REFERENCE MANUAL, VOLUME I: HYDROLOGY (REVISED)

EPA/600/R-15/162A JANUARY 2016

Rossman, L. AND W. Huber. Storm Water Management Model Reference Manual Volume I, Hydrology. US EPA Office of Research and Development, Washington, DC, EPA/600/R-15/162A, 2015.

Maximum Moisture Deficit (θ_{dmax})

The maximum moisture deficit, θ_{dmax} is defined as the difference between the moisture content at saturation and at the start of the simulation. Because this parameter is the most sensitive of the three parameters for estimates of runoff from pervious areas (Brakensiek and Onstad, 1977), some care should be taken in determining the best θ_{dmax} value to use. The saturated moisture content is approximately equal to the soil's porosity ϕ (i.e., the fraction of voids), assuming one ignores the 5 - 10% of trapped air that typically exists at saturation. After a saturated soil is allowed to drain thoroughly, the residual moisture content that remains is ϕ_r . The effective porosity ϕ_e is defined as $\phi - \phi_r$ and can be used to represent θ_{dmax} for dry antecedent conditions. Typical values of ϕ_e are included in the Rawls et al. (1983) data set listed in Table 4-7.

Sandy soils tend to have lower porosities than clay soils, but drain to lower moisture contents between storms because the water is not held so strongly in the soil pores. Consequently, values of θ_{dmax} for dry antecedent conditions tend to be higher for sandy soils than for clay soils. Table 4-8, derived from Clapp and Hornberger (1973), is another source of θ_{dmax} values for various soil types.

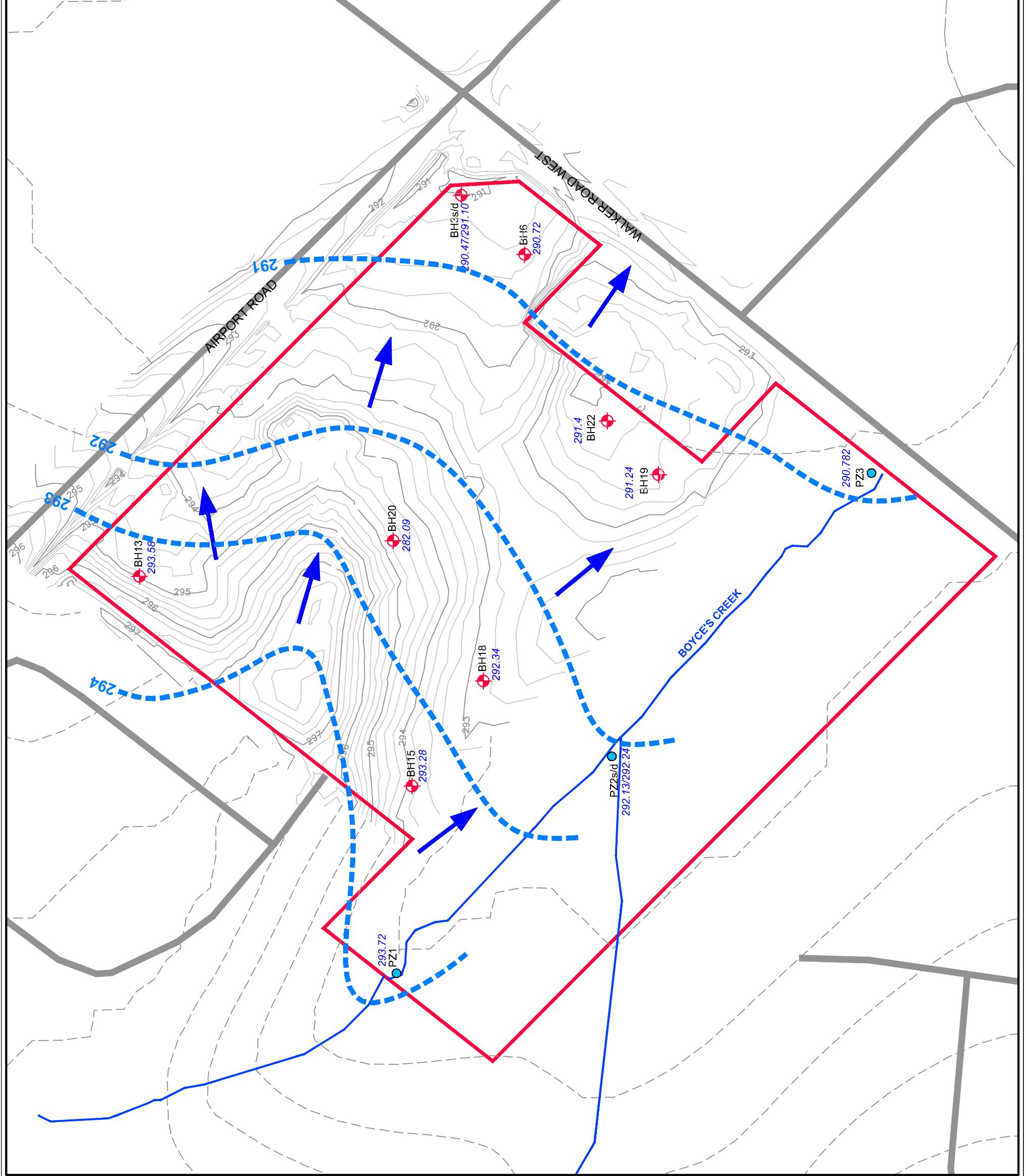
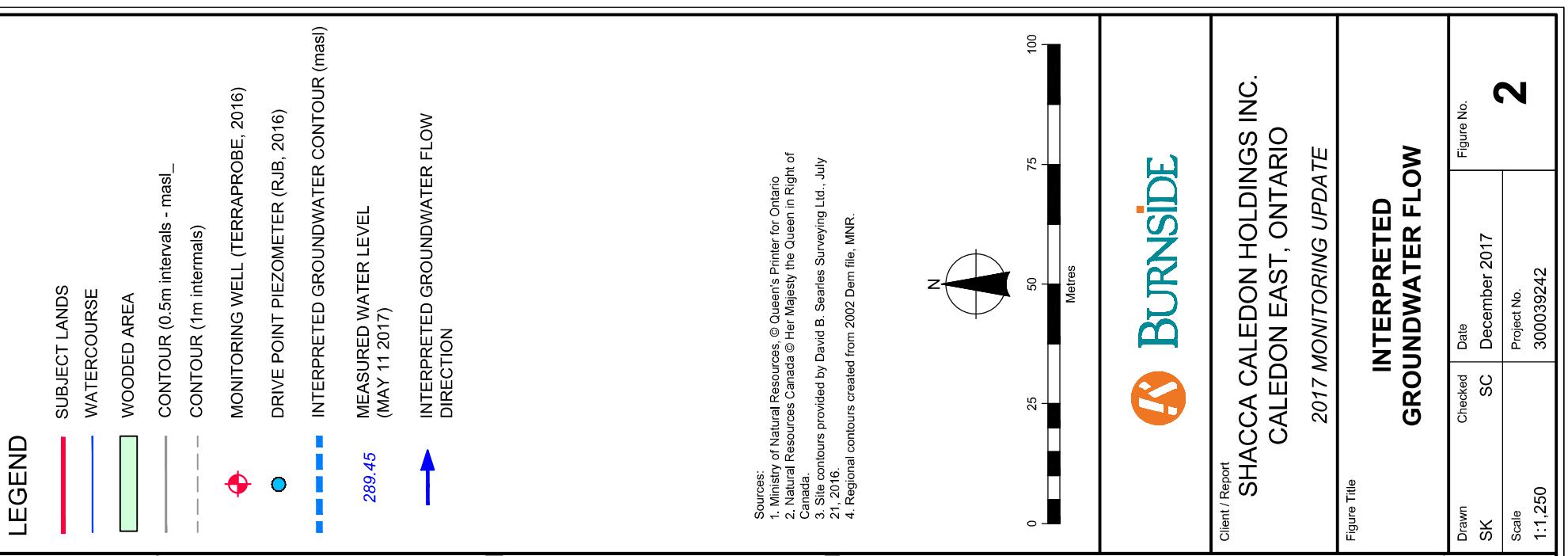
Table 4-8 Typical values of θ_{dmax} for various soil types.

Soil Texture	Typical θ_{dmax} at Soil Wilting Point
Sand	0.34
Sandy Loam	0.33
Silt Loam	0.32
Loam	0.31
Sandy Clay Loam	0.26
Clay Loam	0.24
Clay	0.21

OBTAINED FROM EPA SWMM REFERENCE MANUAL, VOLUME I: HYDROLOGY (REVISED)

EPA/600/R-15/162A JANUARY 2016

Rossman, L. AND W. Huber. Storm Water Management Model Reference Manual Volume I, Hydrology. US EPA Office of Research and Development, Washington, DC, EPA/600/R-15/162A, 2015.



[TITLE]

;; Project Title/Notes

[OPTIONS]

;; Option	Value
FLOW_UNITS	CMS
INFILTRATION	GREEN_AMPT
FLOW_ROUTING	DYNWAVE
LINK_OFFSETS	DEPTH
MIN_SLOPE	0
ALLOW_PONDING	YES
SKIP_STEADY_STATE	NO

START_DATE	05/23/1986
START_TIME	00: 00: 00
REPORT_START_DATE	05/23/1986
REPORT_START_TIME	00: 00: 00
END_DATE	11/01/2007
END_TIME	23: 59: 00
SWEEP_START	01/01
SWEEP_END	12/31
DRY_DAYS	0
REPORT_STEP	01: 00: 00
WET_STEP	00: 05: 00
DRY_STEP	00: 05: 00
ROUTING_STEP	0: 01: 00
RULE_STEP	00: 00: 00

INERTIAL_DAMPING	PARTIAL
NORMAL_FLOW_LIMITED	BOTH
FORCE_MAIN_EQUATION	H-W
VARIABLE_STEP	0.75
LENGTHENING_STEP	0
MIN_SURFAREA	1.14
MAX_TRIANGLES	8
HEAD_TOLERANCE	0.0015
SYS_FLOW_TOL	5
LAT_FLOW_TOL	5
MINIMUM_STEP	0.5
THREADS	1

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DRY_ONLY	NO

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ADC PERVIOUS	1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0

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CurbLen	SnowPack					
;;-----	-----	-----	-----	-----	-----	-----

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1599-FOR REPORT. i np

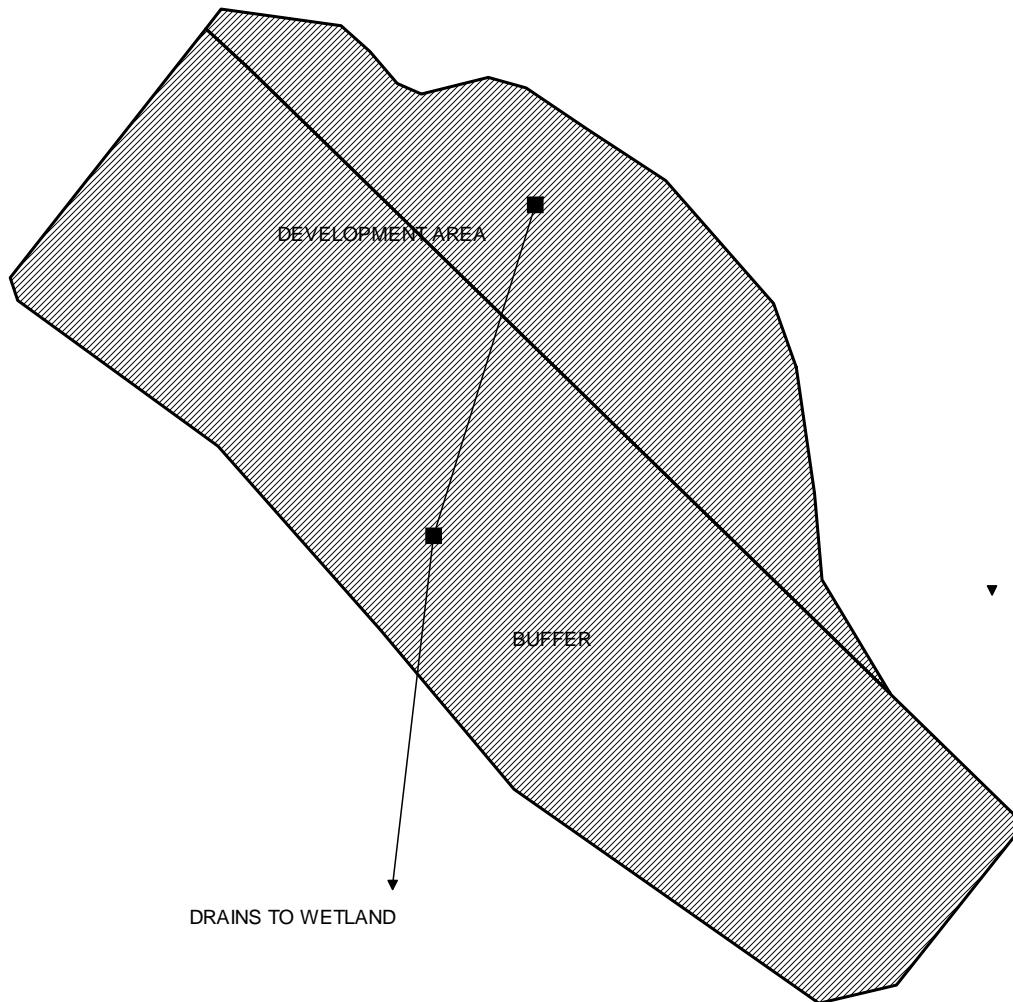
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DEV-AREA BUFFER	98.4 98.4	15.5 15.5	0.33 0.33				
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CONTROLS	YES						
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NODES	ALL						
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[MAP]							
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Units	Meters						
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[VERTICES]							
;; Link	X-Coord	Y-Coord					
[Polygons]							
;; Subcatchment	X-Coord	Y-Coord					
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DEV-AREA	590694.360	4858287.092					
DEV-AREA	590679.505	4858303.962					
DEV-AREA	590668.709	4858310.997					
DEV-AREA	590660.524	4858316.619					
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DEV-AREA	590616.446	4858324.728					
DEV-AREA	590622.158	4858319.501					
DEV-AREA	590710.380	4858233.614					
DEV-AREA	590701.028	4858249.127					
DEV-AREA	590700.042	4858261.163					
BUFFER	590622.158	4858319.501					
BUFFER	590616.446	4858324.728					
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4858215. 772

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05/23/1986 01:00:00



Untitled

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

Element Count

Number of rain gages	1
Number of subcatchments	2
Number of nodes	2
Number of links	0
Number of pollutants	0
Number of land uses	0

Rainage Summary

Name	Data Source	Data Type	Recording Interval
RG-1	P:\1599_Shacca\Storm Water Management\Water Balance\615HMAK_Hourly_Precip.dat		

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
DEV-AREA	0.22	105.00	0.00	9.0000	RG-1	BUFFER
BUFFER	0.65	155.00	0.00	4.0000	RG-1	WETLAND

Node Summary

Name	Type	Invert El ev.	Max. Depth	Ponded Area	External Inflow
WETLAND	OUTFALL	290.39	0.00	0.0	
DIVERTED	OUTFALL	0.00	0.00	0.0	

Rainfall File Summary

Station ID	First Date	Last Date	Recording Frequency	Periods w/Precip	Periods Missing	Periods Mal Func.
STA01	05/23/1986	11/01/2007	60 min	6337	0	0

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units CMS
Process Models:

Untitled

Rainfall /Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Surcharge Method EXTRAN
 Starting Date 05/23/1986 00:00:00
 Ending Date 11/01/2007 23:59:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00

Control Actions Taken

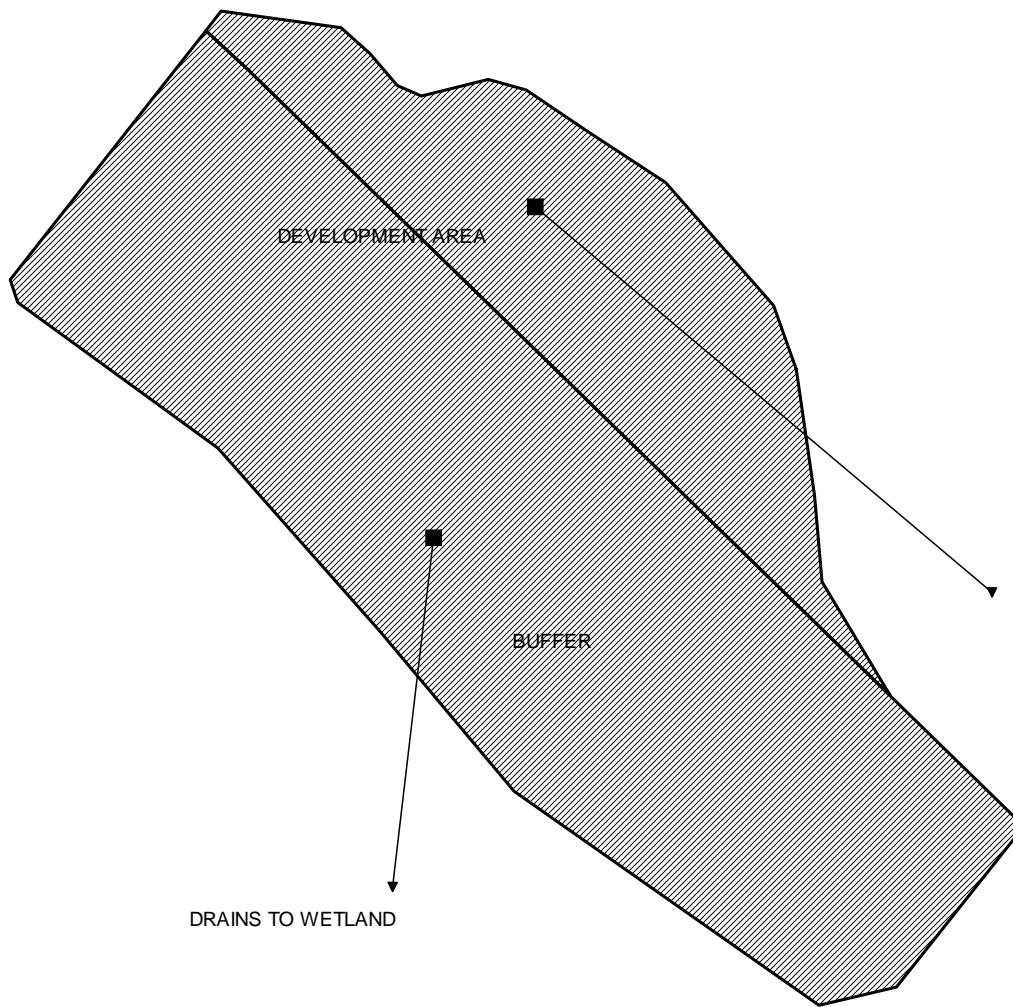
Runoff Quantity Continuity

	Volume hectare-m	Depth mm
Total Precipitation	8.259	9471.100
Evaporation Loss	0.001	0.983
Infiltration Loss	8.256	9468.008
Surface Runoff	0.002	2.862
Final Storage	0.000	0.000
Continuity Error (%)	-0.008	

Flow Routing Continuity

	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.002	0.025
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.002	0.025
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Analysis begun on: Thu Oct 10 11:44:14 2019
 Analysis ended on: Thu Oct 10 11:44:20 2019
 Total elapsed time: 00:00:06



Untitled

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

Element Count

Number of rain gages	1
Number of subcatchments	2
Number of nodes	2
Number of links	0
Number of pollutants	0
Number of land uses	0

Rainage Summary

Name	Data Source	Data Type	Recording Interval
RG-1	P:\1599_Shacca\Storm Water Management\Water Balance\615HMAK_Hourly_Precip.dat		

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
DEV-AREA	0.22	105.00	0.00	9.0000	RG-1	
DIVERTED BUFFER	0.65	155.00	0.00	4.0000	RG-1	WETLAND

Node Summary

Name	Type	Invert El ev.	Max. Depth	Ponded Area	External Inflow
WETLAND	OUTFALL	290.39	0.00	0.0	
DIVERTED	OUTFALL	0.00	0.00	0.0	

Rainfall File Summary

Station ID	First Date	Last Date	Recording Frequency	Periods w/Precip	Periods Missing	Periods Mal func.
STA01	05/23/1986	11/01/2007	60 min	6337	0	0

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units CMS
Process Models:

Untitled

Rainfall /Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Surcharge Method EXTRAN
 Starting Date 05/23/1986 00:00:00
 Ending Date 11/01/2007 23:59:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00

Control Actions Taken

Runoff Quantity Continuity

	Volume hectare-m	Depth mm
Total Precipitation	8.259	9471.100
Evaporation Loss	0.001	0.972
Infiltration Loss	8.256	9467.434
Surface Runoff	0.003	3.385
Final Storage	0.000	0.000
Continuity Error (%)	-0.007	

Flow Routing Continuity

	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.003	0.030
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.003	0.030
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Analysis begun on: Thu Oct 10 11:42:37 2019
 Analysis ended on: Thu Oct 10 11:42:43 2019
 Total elapsed time: 00:00:06