TOWN OF CALEDON PLANNING RECEIVED

Sept.17, 2021

Fluvial Geomorphological Assessment and Flow Monitoring

# **Tributary of Etobicoke Creek**

**Snell's Hollow Secondary Plan Town of Caledon, Ontario** 



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Geomorphology Earth Science Observations

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## **1** Introduction and Background

GEO Morphix Ltd. was retained to complete a fluvial geomorphological assessment and flow monitoring in support of the Snell's Hollow Secondary Plan in the Town of Caledon, hereafter referred to as the subject lands. The subject lands are bounded by Highway 410 to the north and east, Kennedy Road to the west, and Mayfield Road to the south. A portion of the Heart Lake Wetland Complex, a provincially significant wetland (PSW), is located in the southern portion of the subject lands. This wetland complex and associated drainage features are located within the Etobicoke Creek watershed and the jurisdiction of Toronto and Region Conservation Authority (TRCA).

The following activities were completed as part of the fluvial geomorphological assessment:

- Conduct rapid geomorphological assessments and collect general observations to document existing channel conditions
- Complete a detailed geomorphological assessment, including a survey of the longitudinal profile and six (6) cross sections (including two monumented cross sections)
- Install erosion pins to quantify the rate and extent of erosion at monumented crosssections
- Complete grain size analysis using a modified Wolman (1954) pebble count or through collection of bed sample to observe changes in bed composition over time, as appropriate
- Determine an erosion threshold for the reach downstream of Mayfield Road
- Collect time stamped monumented photographs to provide a record of existing conditions

The following activities were completed as part of the 2019 flow monitoring program:

- Install stream flow monitoring equipment in four (4) locations within the subject lands to record water level and temperature at 15-minute intervals
- Record local atmospheric temperature and pressure at 15-minute intervals
- Install monumented cross-sections at each monitoring station for the periodic collection of velocity measurements
- Collect time stamped monumented photographs to provide a record of existing conditions

Stream flow monitoring activities will continue in 2020, with all 4 monitoring stations re-installed on March 24, 2020 for the April 1<sup>st</sup> start of the monitoring season. This report will subsequently be updated to include additional data following removal of all monitoring equipment in the late fall of 2020.

## 2 Background Review and Desktop Assessment

#### 2.1 Physiography and Geology

Channel morphodynamics are largely governed by the flow regime and the availability and type of sediments (i.e., surficial geology) within the stream corridor. These factors are explored as they not only offer insight into existing conditions, but also potential changes that could be expected in the future as they relate to a proposed activity.

The subject lands are located within the gently sloping drumlinized till plains of South Slope physiographic region (Chapman and Putnam, 2007). Published mapping indicates that the local surficial geology within and north of the subject lands consists of clay to silt-textured till derived from glaciolacustrine deposits or shale. These fine-grained till deposits are considered to be

relatively resistant to erosion. In areas where wetlands are currently present, surficial geology consists of organic deposits (OGS, 2010).

#### **2.2 Reach Delineation**

Reaches are homogeneous segments of channel used in geomorphological investigations. They are studied semi-independently as each is expected to function in a manner that is at least slightly different from adjoining reaches. This allows for the meaningful characterization of a watercourse as the aggregate of reaches, or an understanding of a particular reach, for example, as it relates to a proposed activity. Reaches in the study area were delineated first through a desktop assessment using the Ministry of Natural Resources and Forestry (MNRF) stream layer and recent digital aerial photography from Google Earth Pro. Reaches were delineated based on changes in the following:

- Channel planform
- Channel gradient
- Physiography
- Land cover (land use or vegetation)
- Flow, due to tributary inputs
- Soil type and surficial geology
- Certain types of anthropogenic channel modifications

This follows scientifically defensible methodology proposed by Montgomery and Buffington (1997), Richards et al. (1997), Brierley and Fryirs (2005), and the Toronto and Region Conservation Authority (2004). A reach map is provided in **Appendix A**. Reaches were numbered from downstream to upstream to provide geographic context and then verified during field reconnaissance.

Five reaches were delineated within the subject lands. Reach **EC-1** extended from Mayfield Road to Heart Lake. Reach **EC-2** consisted of the pond feature north of Mayfield Road. Reach **EC-2a** extended from an agricultural field at the north extent of the subject lands to the pond feature. Reach **EC-3** contained the wetland that extended from Kennedy Road to the pond feature. Reach **EC-3a** extended from the property line of a landowner in the western extent of the subject lands to the subject lands to the wetland so the wetland feature.

R.J. Burnside and Associates Limited (Burnside) completed headwater drainage feature assessments (HDFAs) within the subject lands in 2019. Existing conditions documented herein focus on geomorphologic observations and should be considered in conjunction with HDFA assessment results prepared by Burnside under separate cover.

### **3 Field Assessment**

Field assessments of reaches within the subject lands were completed on May 10, 2019 and included the following activities:

- Observations of riparian conditions
- Estimates of bankfull channel dimensions, as appropriate
- Characterization of bed and bank material composition and structure
- Observations of erosion, scour, or deposition
- Collection of georeferenced photographs

These observations and measurements are summarized below and in **Table 1** in the following section. The descriptions are supplemented and supported with representative photographs, which

are included in **Appendix B**. Reach summary field sheets are provided in **Appendix C**. The Rapid Geomorphological Assessment (RGA; MOE, 2003) and the Rapid Stream Assessment Technique (RSAT; Galli, 1996) were not applicable due to the poorly defined nature of the features.

### 3.1 General Reach Observations

Reach **EC-1** began at the outlet of the pond feature (**EC-2**) and flowed through a steel culvert under Mayfield Road, continuing south through a confined valley towards Heart Lake. The reach had a low gradient and where defined, contained a wide, shallow channel. Riparian vegetation was mainly comprised of mature trees and was greater than 10 channel widths. Bank materials ranged from clay to sand and little to no bank erosion was observed. There were no riffles or pools. Bed materials consisted of organic material, clay, silt, and fine sand. Two trail crossings were present across the channel and valley. Woody debris was present in the channel but was not attributed to channel widening. Reach **EC-1** was chosen as the location for the detailed geomorphological assessment and erosion threshold analysis.

Reach **EC-2** consisted of a pond feature that separated wetland reach **EC-3** upstream to the west and Mayfield Road downstream to the southeast. Reach **EC-2a** extended from the border of an agricultural field to the north. This feature was characterized as poorly defined and had a moderate gradient. Burnside identified the upstream portion of this reach as a headwater drainage feature. The riparian vegetation buffer was continuous and comprised of grasses that extended more than 10 channel widths. The feature was extensively encroached with grasses, and a large, man-made woody debris pile was present in the middle of the reach. Bankfull width and depth at the downstream extent of the reach were 6.0 m and 0.4 m, respectively. Bank materials consisted of clay, silt, and sand. Bank angles ranged from 30 – 60 degrees with little to no erosion. There was no evidence of riffle-pool morphology. Bed materials were comprised of clay, silt, and sand.

Reach **EC-3** consisted of a large wetland feature that began at the southwest extent of the subject lands. The southwest corner of the feature was bound by a retaining wall adjacent to Mayfield Road and the stormwater management (SWM) pond at the corner of Kennedy Road and Mayfield Road. Recorded velocity measurements showed that the wetland slowly drained eastwards into the pond feature (**EC-2**). Vegetation within the wetland consisted of cattails, deciduous trees, shrubs and grasses.

Reach **EC-3a** began at the property line of a landowner in the northwest corner of the subject lands. The reach was unconfined, and consisted of a low gradient channelized feature that was moderately entrenched. Burnside identified the upstream portion of this reach as a headwater drainage feature. The riparian buffer zone was wide and mainly comprised of grasses. Average bankfull width and depth were 1.4 m and 0.3 m, respectively. Bank angles ranged from 60 – 90 degrees and the reach showed minimal signs of erosion. Bank materials consisted of clay, silt, and sand. Riffle-pool morphology was not present. Bed materials were comprised of sand and gravel.

	Average	Average	Substrate				
Reach	Bankfull Width (m)	Bankfull Depth (m)	Bed	Bank	Riparian Vegetation	Notes	
EC-1	17.95	0.32	Organic material, clay, silt, Find Sand	Clay, silt, sand	Mature trees	Wetland-like channel; confined valley; wide, shallow channel; no evidence of channel widening	

#### Table 1. General channel characteristics



	Average	Average Average		Substrate		
Reach	Bankfull Width (m)	Bankfull Depth (m)	Bed	Bank	Riparian Vegetation	Notes
EC-2	N/A-Pond Feature		N/A		Grasses	Outlets south to steel culvert crossing at Mayfield Road
EC-2a	6.0	0.4	Clay, Silt, Sand	Clay, Silt, Sand	Grasses	Extensive vegetation encroached; large man- made woody debris pile mid-reach
EC-3	N/A; Wetland Feature		N/A		Grasses	Unconfined; no defined channel; cattails, trees, shrubs, grasses present
EC-3a	1.4	0.3	Clay, Silt, Sand	Sand, Gravel	Grasses	Channelized feature; moderately entrenched

### 3.2 Detailed Geomorphological Assessment

A detailed geomorphological assessment was completed on May 6, 2019 within Reach **EC-1** as this reach was identified as the most sensitive to erosion. The specific location within the reach was chosen as it had the most defined section of channel. The assessment included a longitudinal survey of the channel bed and water level to determine gradients, and the completion of six detailed cross-section surveys. Two of these cross-sections were monumented and included the installation of erosion pins. At each cross section, bankfull geometry was recorded, as well as riparian conditions, bank material, bank height/angle, the presence of undercutting, and bank root density. Characterization of channel bed material at each cross section was completed using a modified Wolman (1954) pebble count technique or through collection of bed samples, as appropriate. Photographs of each cross section and both channel banks were also collected at the time of the survey. Results from the detailed assessment are summarized in **Table 2**. A complete summary of the detailed assessment is provided in **Appendix D**.

Channel Parameter	EC-1			
Measured				
Average bankfull channel width (m)	17.95			
Average bankfull channel depth (m)	0.32			
Bankfull channel gradient (%)	0.66			
D <sub>50</sub> (mm)	< 2.0			
Manning's n roughness coefficient	0.050			
Computed				
Bankfull discharge (m <sup>3</sup> /s) *	4.30			
Average bankfull velocity (m/s)*	0.76			

#### Table 2. Measured and computed channel parameters

\* Based on Manning's Equation

## 4 Erosion Threshold Assessment

#### 4.1 Methodology

Erosion thresholds are used to determine the magnitude of flow required to potentially entrain and transport bed and/or bank materials. As such, they may be used to inform erosion reduction strategies in channels influenced by conceptual flow management plans. The erosion threshold analysis provides a depth, velocity, or discharge at which sediment of a particular size may potentially be entrained. This is then field-validated through sediment transport observations under a range of flows. Due to the variability between bed and bank composition and structure, erosion thresholds are typically determined for both bed and bank materials. Threshold targets are determined using different methods that are dependent on channel and sediment characteristics. For example, thresholds for non-cohesive sediments are commonly estimated using a shear stress approach, similar to that of Miller et al. (1977), which is based on a modified Shield's curve. A velocity approach could also be applied. For non-cohesive materials, a method such as that described by Komar (1987), or empirically-derived values such as those compiled by Fischenich (2001) or Julien (1994), could be applied.

An erosion threshold is quantified based on the bed and bank materials and local channel geometry, in the form of a critical discharge. Theoretically, above this discharge, entrainment and transport of sediment can occur. The velocity, U is calculated at various depths, until the average velocity in the cross section slightly exceeds the critical velocity of the bed material. The velocity is determined using a Manning's approach, where the Manning's n value is visually estimated through a method described by Arcement and Schneider (1989) or calculated using Limerinos's (1970) approach. The velocity is mathematically represented as

 $U = \frac{1}{n} d^{2/3} S^{1/2}$ 

[Eq. 1]

where, d is depth of water, S is channel slope, and n is the Manning's roughness. The discharge is then calculated using the area of a typical cross section at that depth.

For the bank materials, following Chow (1959) in a simplified cross section, 75% of the bed shear stress acts on the channel banks. In a similar approach, the depth of flow is increased until the shear stress acting on the banks exceeds the resisting shear strength of the bank materials.

### 4.2 Results

Erosion thresholds were determined for the bed and bank materials within **Reach EC-1** of the Tributary of Etobicoke Creek. This reach was deemed to be the most sensitive to erosion of the reaches assessed, although it was still considered to be a low risk environment as it was depositional.

Channel bed and bank materials were considered equivalent, and conservatively estimated to consist of a fairly compact to loose clay. A critical shear stress approach was taken using the criteria of Julien (1994) for this material, which has a critical shear stress of 6.2 N/m<sup>2</sup>. This threshold shear stress was then applied to a representative cross section measured from the detailed assessment to calculate the critical discharge, or the discharge at which it is expected that sediment entrainment will begin to occur. The results of the erosion assessment are provided in **Table 4**. Using the criteria of Chow, the critical discharge to entrain the bed materials within **Reach EC-1**, was determined to be  $1.25 \text{ m}^3/\text{s}$ .

We note that **Reach EC-1**, as well as the others that may receive stormwater flows in the subject lands, are relatively resilient to potential erosion given their low gradient and wide, oversized bankfull channels. Consequently, we do not advocate for using the erosion threshold assigned to **Reach EC-1** to aid in designing the associated SWM pond and outlet structure given the high volume of water the channel has the capacity to tolerate. Doing so could conceivably cause downstream erosion concerns in other reaches that are more sensitive to erosion. Instead, we suggest using the 24 or 48 hour detention of the 25 mm event to prevent erosion both within the study area, and downstream within Etobicoke Creek.

#### Table 3. Erosion thresholds and average channel parameters

Channel Parameter	Reach EC-1			
Average bankfull channel width (m)	17.95			
Maximum bankfull channel depth (m)	0.32			
Average channel gradient (%)	0.66			
Calculated bankfull discharge (m <sup>3</sup> /s)	4.3			
Bankfull shear stress (N/m <sup>2</sup> )	20.53			
Erosion thresholds for bed and bank materials				
Critical shear stress (N/m <sup>2</sup> )	6.2			
Critical discharge (m <sup>3</sup> /s)	1.25			

### 5 Flow Monitoring

During 2019, flow monitoring was conducted at four (4) locations on the subject lands to assess water quantity characteristics. A map of monitoring locations is provided for reference in **Appendix A. Table 4**, below, summarizes monitoring activities at each location.

Table 4.	Flow	monitoring	sites,	sampling	parameters,	and	sampling	duration	in
2019									

Site	Sampling Parameters	Monitoring Duration	# Visits
W Inlet	Continuous water level & temperature Velocity measurements when possible	April 4 – November 30	8
S Inlet	Continuous water level & temperature Velocity measurements when possible	April 4 – November 30	8
Bridge	Continuous water level & temperature Velocity measurements when possible	April 4 – November 30	8
Outlet	Continuous water level & temperature Velocity & discharge measurements when possible	April 4 – October 30*	8

\*Sensor stolen/lost between October 30 visit and sensor removal

Activities at all locations included the following:

• Collect water level and temperature data at 15-minute intervals using a HOBO U20 pressure and temperature logger, with an additional control sensor to measure atmospheric pressure and air temperature on- site

- - Record velocity measurements using Acoustic Doppler Velocimeter (ADV), when possible, to calculate discharge
  - Collect monumented photographs of all sampling activities to verify location and timing

All sampling activities adhere to the Ontario Stream Assessment Protocol outlined by the Ontario Ministry of Natural Resources and Forestry (MNRF, 2017). Daily rainfall data was acquired from a Weather Underground weather station (Climate ID: ICALED1) located approximately 1.5 km west of the subject lands to account for precipitation and climatic conditions.

### 5.1 Water Level Monitoring

Water level loggers recorded continuous pressure throughout the entire monitoring season (April 4 – November 30). Discrete stilling well measurements were taken during each site visit in order to ensure data quality and data verification.

Baseflow is the portion of streamflow derived from natural storage sources and does not include direct runoff from precipitation. There must not be any evidence in the stage discharge hydrograph of any recent storm events to be considered baseflow. Due to the intermittent/ephemeral nature of these watercourses, all four sites were dry following the spring freshet. During spring, the baseflow levels of the **W inlet, S inlet, Bridge,** and **Outlet** sites were approximately 0.02 m, 0.13 m, 0.10 m, and 0.03 m respectively.

Water level responses are dependent on the magnitude of the rainfall event and antecedent conditions. The maximum water levels during 2019 for the **W Inlet** site was observed on May 25 following a 33.53 mm rain event. The maximum water depth at the **W Inlet** site was 0.09 m on this day. Maximum water depths at the **S Inlet**, **Bridge**, and **Outlet** sites were 0.20 m, 0.19 m, and 0.09 m respectively, recorded on April 26, following a 23.37 mm rain event.

Minimum and maximum water levels recorded by monitoring equipment is summarized below in **Table 5**. The full set of continuous water level measurements, as well as discrete measurements, are provided in **Appendix E.** 

Sampling Location	2019 Water Depth (m)					
	Minimum	Maximum				
W Inlet	0.00	0.09				
S Inlet	0.00	0.20				
Bridge	0.00	0.19				
Outlet	0.00	0.09				

#### Table 5. Minimum and maximum water depths at each sampling location

### **5.2 Velocity and Discharge Monitoring**

In addition to continuous water level and temperature monitoring, discrete measurements of velocity (**W Inlet, S Inlet,** and **Bridge** sites) were recorded, when possible. A summary of measured discharge at each sampling location is summarized below in **Table 6**.



Measurement Date (mm-dd-yyyy)	Location	Average Velocity (m/s)	Discharge (m³/s)
	W Inlet	0.0114	0.0002
04 00 2010	S Inlet	0	0
04-09-2019	Bridge	0	0
	Outlet	0.2734	0.0150
	W Inlet	0.0538	0.0009
05 10 2010	S Inlet	0	0
05-10-2019	Bridge	0.0400	0.0023
	Outlet	0.3392	0.0180
	W Inlet	0	0
06 20 2010	S Inlet	N/A*	N/A*
00-20-2019	Bridge	N/A*	N/A*
	Outlet	0.0170	0.0004

Table 6. Average velocity and measured discharge at each sampling location

\*Channel dry or too shallow for measurement

Due to the intermittent/ephemeral nature of these sites, velocity measurements were only possible during the spring freshet. A full record of attempted velocity readings is provided in **Appendix E**. Velocity measurements were not possible during monitoring visits at the **S Inlet** site. This is due to the lack of channel definition and wetland characteristics at the sensor location. Maximum discharges at the **W Inlet, Bridge,** and **Outlet** sites were 0.0009 m<sup>3</sup>/s, 0.0025 m<sup>3</sup>/s, and 0.0180 m<sup>3</sup>/s respectively, which occurred on May 10, 2019 following 21.59 mm of rainfall in 24 hours.

## 6 Summary and Conclusions

GEO Morphix was retained to complete a fluvial geomorphological assessment of the drainage features within the subject lands. This assessment included a background review, reach delineation and rapid field reconnaissance to confirm existing conditions. A detailed geomorphic assessment was completed downstream of the subject lands, along Reach **EC-1**, to determine an appropriate erosion threshold in support of the stormwater management strategy. The critical discharge to entrain the bed materials within **Reach EC-1** was determined to be 1.25 m<sup>3</sup>/s. Notably, reaches within and downstream of the subject lands are relatively resilient to potential erosion due to their generally low gradients and wide, oversized bankfull channels. Consequently, the erosion threshold assigned to **Reach EC-1** could potentially cause downstream erosion concerns in other reaches that are more sensitive to erosion. Rather, the 24 or 48 hour detention of the 25 mm event is recommended to prevent erosion both within the study area, and downstream within Etobicoke Creek.

Water level and temperature data were collected at 15-minute intervals at 4 sites within the subject lands. Monumented cross sections were installed at each site to collect periodic velocity measurements to determine discharge. Monitoring results revealed that these drainage features are ephemeral, as they only contained water during the spring freshet. Due to a lack of channel definition, discharge could not be calculated for the **S Inlet** site. Maximum discharges at the **W** 



**Inlet, Bridge,** and **Outlet** sites were 0.0009 m<sup>3</sup>/s, 0.0025 m<sup>3</sup>/s, and 0.0180 m<sup>3</sup>/s respectively, which occurred on May 10, 2019 following 21.59 mm of rainfall in 24 hours.

We trust this report meets your requirements. Should you have any questions please contact the undersigned.

Respectfully submitted,

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Appendix A Reach Delineation and Monitoring Station Locations



#### Legend

- Reach break
   Watercourse
   Wetland
   Contour (1 m)
   Secondary Plan Area
- Atmospheric sensor
   Pressure logger
   Detailed assessment
   Top of bank\*

   \*Staked by TRCA (Oct. 23, 2018) and Biason Surveying Inc. (Sept. 20, 2011)

## Reach Delineation and Monitoring Station Locations

Tributary of Etobicoke Creek

Snell's Hollow Secondary Plan Area

V

GEO MORPHIX™

0 100 LLLLL Metres

Imagery: Google Earth Pro, 2018. Top of bank: GSAI, 2019. Reach break, Monitoring Jocations, and Detailed assessment: GEO Morphix Ltd., 2019. Watercourse and Wetland: MNRF and GEO Morphix Ltd., 2019. Contour: J.H. Gelbloom Surveying Limited and GEO Morphix Ltd., 2019.

# Appendix B Photographic Record





































# Appendix C Field Observations

Reach Chara	cteristics		Project Coo	de: PKI	1903	š3	GEO	Geomorphole Earth Science Observations	RPH	IX
Date:	2019-05-10	Stream	n/Reach:	Pond	()	: Mn	UP:01	1 Rd.	IEC.	7
Weather:	overcast 8°C	Locatio	on:	Mauf	ipld	ld +	Ferr	redu	PRN	
Field Staff:	LG + KIM	Waters	shed/Subwatershed:	Etah	cor	PC	(F	)		
UTM (Upstream)			Downstream)			Law Car	<ul> <li>Preprint</li> </ul>	••••••••••••••••••••••••••••••••••••••		
Land Use (Table 1)	(Table 2) Channel Type (Table 3) Channel (Table 3)	Zone ble 4)	JA Flow Type (Table 5)	IA □Grou	undwater	E	vidence: _	Nor	R	
<b>Riparian Vegetation</b>			Aquatic/Instream Ve	getation			Water Qu	ality		
Dominant Type:     Cov       (Table 6)     3       Species:     1	Channel widths     Age Class (yrs):     Encroachmer       None     1-4     Immature (<5)     (Table       Fragmented     4-10     Established (5-30)     1       Continuous     > 10     Mature (>30)     1	nt: 7)	Type (Table8)       No         Woody Debris       Present in Cutbank         Present in Cutbank       Present in Channel         Not Present       Not Present	Coverage of Density of Coverage of Density of Coverage Coverage Coverage Coverage Coverage Coverage of Density of Coverage of Density of Coverage of Density of Coverage of Density of Coverage of Coverage of Coverage of Coverage of Coverage of Coverage of Coverage of Coverage of Coverage of Coverage Cover Coverage Coverage Cove Coverage Coverage Coverag	Reach (%) f WD: WDJ/!	50m:		Odour (1 / Turbidity	āble 16) (Table 17)	
Channel Characteristic	CS									
Sinuosity (Type)	Sinuosity (Degree) Gradient Num	ber of Cl	hannels	Clay/Silt	Sand	Gravel	Cobble	Boulder	Parent	Rootlets
(Table 9) NF	(Table 10) $\mathbb{N} \cap$ (Table 11) $\mathbb{N} \cap$ (Table 11)	le 12)	NA Riffle Substra	ite MA						
Entrenchment	Type of Bank Failure Downs's Classification		Pool Substra	nte ND/A						
(Table 13)	(Table 14) NA (Table 15) NA		Bank Material	×						
Bankfull Width (m) Bankfull Depth (m) Riffle/Pool Spacing (m Pool Depth (m) Velocity (m/s)	NA       Wetted Width (m)         NA       Wetted Depth (m)         NA       % Riffles:       NA       % Pools:       N         NA       Riffle Length (m)       NA       Undercuts (m)         NA       Wiffle ball / ADV	NA NA Mea NA / Estimat	nder Amplitude:	JA JA	<b>ak Angle</b> 0 – 30 30 – 60 50 – 90 Jindercut	Bank Er S < 5% 5 - 3 30 - 60 -	osion 0% 60% 100%	Notes:		
				Compl	eted by:	(H	C	hecked by	:	

Reach Characteristics		Project Code:	PN190	33		GEO	M O Geomorphak Eerth Science Observations	R P H	IX		
Date: 7019-05-16	Stream/Reach: Wiglet of Wetlaw						TEC	- 20			
Weather: Christ & Christ & Christ	Location:	Location: Montfield Rd @ Kow						rodu Roerd			
Field Staff: LG + WM	Staff: (G + WM Watershed: Ftabico Ke (r/L.										
UTM (Upstream) 595897.89 nE 4845049.02 m N	UTM (Downstr	ream)	95954.9	11mE	4844	971.78 .	n N				
Land Use (Table 1)     Valley Type (Table 2)     Channel Type (Table 3)     Channel (Table 3)	able 4)	Flow Type (Table 5)	Groun	dwater	E۷	vidence: _					
Riparian Vegetation	Aqua	tic/Instream Veget	ation			Water Qu	ality				
Dominant Type: Coverage: Channel widths Age Class (yrs): Encroachm	ent: Type	(Table8) 1 C	overage of R	each (%)	100		Odour (	able 16)			
(Table 6) 3 □ None □ 1-4 🗹 Immature (<5) (Tab	le 7) Wood	dy Debris	Density of	WD:							
Species: $\Box$ Fragmented $\Box$ 4-10 $\Box$ Established (5-30)		esent in Cutbank esent in Channel	I Low □ Modera	wDJ/50	)m:		Turbidity	(Table 17)			
		ot Present	🗆 High								
Channel Characteristics											
Sinuosity (Type) Sinuosity (Degree) Gradient Nu	mber of Channels		Clay/Silt	Sand	Gravel	Cobble	Boulder	Parent	Rootlets		
(Table 9) 2 (Table 10) 1 (Table 11) 2 (T	able 12) 1	Riffle Substrate	NIA	Γ 🗆							
Entrenchment Type of Bank Failure Downs's Classification		Pool Substrate	X	X							
(Table 13) 1 (Table 14) 1 (Table 15) d		Bank Material	×	Ŋ							
Bankfull Width (m)	Y NI	ANIA	Bank	Angle	Bank Ero	osion	Notes:	arge	Man-n		
Bankfull Depth (m)	DOC NI	AVIA	□ 30	) – 60	□ 5 – 30	0%	1.10		File		
	0101	N WIT	🖾 🖾	) – 90	□ 30 – 6	50%	WVU	na	way		
Riffle/Pool Spacing (m) % Riffles: NIA % Pools:	Meander An	nplitude: MI		ndercut	□ 60 - 3	L00%	dow	in re	ach.		
Pool Depth (m) U/A Riffle Length (m) V/A Undercuts (m	n) N/A-Com	ments: Draw	age	Feat	ure		Norit	this o	V		
Velocity (m/s)	V / Estimated	from	tg-fi	eld			pod	15			
BFW and BFD taken at DS end a	of reach		Comple	ted by: _	6	C	hecked by	:			

Reach Characteristics		Project Co	de: PN19	1033	>	GEO	M O Geomorpholog Earth Science Observations	RPH "	ΙX			
Date: 1019-05-10	Stream	/Reach:	Wet	land	US 1	Jay fie	Geld Rd. / FC-B					
Weather: Outvest 8°C	Locatio	n:	Marfiel	d Kd	0	du Rood						
Field Staff: $(G + h)M$	Waters	Watershed/Subwatershed: Etabicable CCL.						ex.(				
UTM (Upstream) 595371.06 mE 4844455.82 MN	UTM (D	UTM (Downstream) 596062.82mF 484484798m N										
Land Use 3 Valley Type 3 Channel Type (Table 1) (Table 2) (Table 3) (Table 3)	Zone ble 4)	TA Flow Type (Table 5)	IA □Grou	ndwater	E	vidence:						
Riparian Vegetation		Aquatic/Instream Vegetation					Water Quality					
Dominant Type:       Coverage:       Channel widths       Age Class (yrs):       Encroachment:         (Table 6)       3       None       1-4       Immature (<5)       (Table 7)         Species:       Immature       4-10       Established (5-30)       1         Cattails, burd decid uous frees in wetland.       1       Mature (>30)       1       Immature       Immature         Not Present       Immature       1       Not Present       Immature       Imm												
Channel Characteristics												
Sinuosity (Type) Sinuosity (Degree) Gradient Num	nber of Ch	nannels	Clay/Silt	Sand	Gravel	Cobble	Boulder	Parent	Rootlets			
(Table 9) W/A (Table 10) N/A (Table 11) N/A (Tab	ble 12)	11 A Riffle Substra	ate NAX									
Entrenchment Type of Bank Failure Downs's Classification		Pool Substra	ate NB									
(Table 13) NIA (Table 14) NIA (Table 15) NIA		Bank Material	D.									
Bankfull Width (m)       NIA       Wetted Width (m)         Bankfull Depth (m)       NIA       Wetted Depth (m)         Riffle/Pool Spacing (m)       NIA       % Riffles:       NIA       % Pools:       NI         Pool Depth (m)       NIA       Riffle Length (m)       NIA       Undercuts (m)         Velocity (m/s)       NIA       Wiffle ball / ADV	NIA NIA Mean NIA	Inder Amplitude:	Valley Ban Davd DAD Vard DAD Vard West	Slope k Angle ) - 30 :0 - 60 :0 - 90 Indercut taken	No er Bank Er 2 < 5% 5 - 3 30 - 60 - 0 60 -	osion 0% 60% 100% Vorth, Vs.	Notes: U No de throug Docum betwee	let lar fined gh we ented en well	d, channel chand. flow flavel at			
Northinlet, South inlet. West inlet, and or are separate XS survey's and velocity	otlet , da	channels ta	Compl	eted by:	16	C	Sm hecked by	all Po	nd.			
Luring WQM visits.												
Reach Characteristics	Project Co	de: PN19	033	GEC	M O Geomorphole Earth Science Observations	RPH	ΙX					
---	--	---	---	--------------	---	-------------------------	----------					
Date: 2019-05-10	Stream/Reach:	Winle	t of WC	tland	/ EC	-3a						
Weather: Overcast PC	Location:	Malifiel	d hd C	Kenn	edy ho	ical						
Field Staff: 16 + WM	Watershed/Subwatershed:	Etabicoke Crk.										
UTM (Upstream) 5953108.19 mE 484445299 mN	UTM (Downstream)	TM (Downstream) 595431.33 n.E. 4844513.44 m.N										
Land Use     Solution     Valley Type     Channel Type     Channel Type       (Table 1)     (Table 2)     (Table 3)     (Table 3)	Zone Z Flow Type 1 (Table 5) 1	Ground	lwater I	vidence: _								
Riparian Vegetation	Aquatic/Instream Ve	getation		Water Qu	uality/							
Dominant Type:       Coverage:       Channel widths       Age Class (yrs):       Encroachmen         (Table 6)       3       None       1-4       Immature (<5)       (Table         Species:       Fragmented       4-10       Established (5-30)       3         M       Continuous       >10       Mature (>30)	Type (Table8)       7)     Woody Debris       Image: Descent in Cutbank       Image: Descent in Channe       Image: Descent in Cha	Coverage of Re Density of V C C C C C C C C C C C C C C C C C C C	ach (%) <u>100</u> / <b>D:</b> WDJ/50m:		Odour (1	ʿable 16) (Table 17)						
Channel Characteristics												
Sinuosity (Type) Sinuosity (Degree) Gradient Num	nber of Channels	Clay/Silt	Sand Gravel	Cobble	Boulder	Parent	Rootlets					
(Table 9) 1 (Table 10) 1 (Table 11) 1 (Tab	ole 12) 1 Riffle Substra	ate 🗆										
Entrenchment Type of Bank Failure Downs's Classification	Pool Substra	ate 🕱										
(Table 13) 2 (Table 14) 1 (Table 15) d	Bank Material	X										
Bankfull Width (m)	6.5	Bank /	Angle Bank E	rosion 6	Notes:	litch	W					
Bankfull Depth (m)	6.05	□ 30 60	- 60 □ 5 - 1 - 90 □ 30 -	30% - 60%	Savd	1 Fir	e					
Riffle/Pool Spacing (m) NIA % Riffles: NIA % Pools: NI	A Meander Amplitude:	IMA □ Un	dercut 🗌 60 -	100%	sedir	rent	Res-					
Pool Depth (m)	NIA Comments:				depa	usition	0					
Velocity (m/s)	y Estimated				- F							
- XS surveyed in RTK unit at pressure - velocity and wetted reasurements will each WOM with	logger 11 be taken duriv	Complet	ed by: <u>6</u>	C	ı Checked by	:						

## **Reach Characteristics Key**



# Appendix D Detailed Geomorphological Assessment Summary

					GEO MORPHIX terrine photogy Each Scance Deservations		
Gene	eral Site Cha	irac	teristics		Project Code: PH19033		
Date:		M	046.20	9	Stream/Reach:		
Weath	ner:	5	10+15%		Location: HPOGLOYPCA		
Field S	Staff:	0	HOKIN		Watershed/Subwatershed: TE fobicc/PC//		
Featur	'es		11 8-1	•	Site Sketch:		
	Reach break			-	E S X V CO S + O		
××	Cross-section				C C C C C C C C C C C C C C C C C C C		
	Flow direction			K	E K on R N		
	Riffle			9			
attitue	Pool Medial have			~			
++++++++++++	Froded bank			-			
	Undercut bank			-			
XXXXXX	Rip rap/stabilization	n/gabio	on	-			
	Leaning tree	, 5		in			
xx	Fence			9	Q Q X ST A	X53M	
	Culvert/outfall			Cr.			
$\bigcirc$	Swamp/wetland						
VVV	Grasses			-	Fail		
	I ree			188 m. 1997 -	BUILD'W!		
×××	Woody debris				Cover W		
<b>.</b>	Station location			-	C C C C C C C C C C C C C C C C C C C		
(V)	Vegetated island				V V D Ocheano		
Flow T	уре						
H1	Standing water						
H2	Scarcely perceptible	e flow			VX XX4-N	100	
H3	Smooth surface flow	V		i	M ON		
H4	Upwelling			1			
H5	Rippled			-			
H7	Broken standing	wave					
H8	Chute	IVE		-			
H9	Free fall						
Substr	ate						
<b>S1</b>	Silt	<b>S</b> 6	Small boulder		W _ X +		
S2	Sand	<b>S</b> 7	Large boulder				
<b>S</b> 3	Gravel	<b>S</b> 8	Bimodal		on the co		
S4	Small cobble	<b>S9</b>	Bedrock/till		V C C C		
S5	Large cobble			-	MANNA MARKEN MARKEN		
BM	Benchmark	FD	Frosion pin	T	Room		
BS	Backsight	RB	Rebar	-			
DS	Downstream	US	Upstream	-			
WDJ	Woody debris jam	TR	Terrace		A Christian and and the statement of the		
vwc	Valley wall contact	FC	Flood chute		C C Scale:	)	
BOS	Bottom of slope	FP	Flood plain		Additional Notes: Munstream		
TOS	Top of slope	КР	Knick point				
				-	11 Acort AMERICA		
XT	* This area was chosen as it is the most narrow						
(	section	of)	hete	0M	Completed by: Checked by:		

Completed by: \_\_\_\_\_ Checked by: \_\_\_\_\_

## GEO

### **Detailed Geomorphological Assessment Summary**

Reach EC-1

<b>Project Number:</b>	PN19033	Date:	May 10, 2016
Client:	Snell's Hollow Landowner Group	Length Surveyed (m):	105.6
Location:	Heart Lake Conservation Park	# of Cross-Sections:	6

Reach Characteristics						
Drainage Area:	Not measured	Dominant Riparian Vegetation Type:	Trees			
Geology/Soils:	Clay to silt-textured till	Extent of Riparian Cover:	Continuous			
Surrounding Land Use:	Forest	Width of Riparian Cover:	>10 channel widths			
Valley Type:	Confined	Age Class of Riparian Vegetation:	Mature (>30 years)			
Dominant Instream Vegetation Ty	<b>pe:</b> Rooted submergent	Extent of Encroachment into Channel:	Minimal			
Portion of Reach with Vegetation:	20%	Density of Woody Debris:	Moderate			

Hyarology			
Measured Discharge (m <sup>3</sup> /s):	Not measured	Calculated Bankfull Discharge (m <sup>3</sup> /s):	4.30
Modelled 2-year Discharge (m <sup>3</sup> /s):	Not modelled	Calculated Bankfull Velocity (m/s):	0.76
Modelled 2-year Velocity (m/s):	Not modelled		

Profile Characteristics		Planform Characteristics	
Bankfull Gradient (%):	0.66	Sinuosity:	1.13
Channel Bed Gradient (%):	0.26	Meander Belt Width (m):	Not measured
Riffle Gradient (%):	N/A: no riffles	Radius of Curvature (m):	Not measured
Riffle Length (m):	N/A: no riffles	Meander Amplitude (m):	Not measured
Riffle-Pool Spacing (m):	N/A: no riffle and pools	Meander wavelength (m):	Not measured

#### Longitudinal Profile

.. . .



### **Bank Characteristics**

	Minimum	Maximum	Average		Minimum	Maximum	Average
Bank Height (m):	0.2	0.70	0.45				
Bank Angle (deg):	10	45	24	Torvane Value (kg/cm <sup>2</sup> ):		Not measured	
Root Depth (m):	0.05	0.20	0.10	Penetrometer Value (kg/cm <sup>3</sup> ):		Not measured	
Root Density (%):	10	70	42	Bank Material (range):		Clay, silt, sand	
Bank Undercut (m):		No undercuts					

#### **Cross-Sectional Characteristics**

	Minimum	Maximum	Average
Bankfull Width (m):	12.70	27.90	17.95
Average Bankfull Depth (m):	0.18	0.49	0.32
Bankfull Width/Depth (m/m):	29	108	61
Wetted Width (m):	4.90	18.50	11.95
Average Water Depth (m):	0.04	0.25	0.13
Wetted Width/Depth (m/m):	48	175	108
Entrenchment (m):		Not measured	
Entrenchment Ratio (m/m):		Not measured	
Maximum Water Depth (m):	0.09	0.54	0.26
Manning's <i>n</i> :		0.050	



Photograph at cross section 4 (left bank)





#### **Channel Thresholds**

Flow Competency (m/s):	
for D <sub>50</sub> :	0.00
for D <sub>84</sub> :	0.00
Unit Stream Power at Bankfull (W/m <sup>2</sup> ):	15.50

Tractive Force at Bankfull  $(N/m^2)$ : Tractive Force at 2-year flow  $(N/m^2)$ : Critical Shear Stress  $(D_{50}) (N/m^2)$ : 20.53 Not modelled 0.00

#### General Field Observations

#### **Channel Description**

Reach EC-1 consisted of a fairly straight and low gradient channel through a confined valley. The continuous and wide riparian buffer zone consisted of mature trees. The average bankfull width and depth were 17.95 m and 0.32 m. Bank materials ranged from clay to sand. Little to no bank erosion was observed. There were no riffles or pools. Bed materials consisted of organic material, clay, silt, and fine sand. Two trail crossings were present across the channel and valley. Woody debris was present within the channel but not due to the channel widening.

#### **Cross Section 4 - Facing Downstream**



# Appendix E Flow Monitoring Data







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**S Inlet Water Temperature** 







**Bridge Water Temperature** 









**Outlet Water Temperature** 








































## **ADV Discharge Measurement Summary**

Measurement Date (mm-dd- yyyy)	Location	Average Velocity (m/s)	Measured Discharge (m³/s)
04-09-2019	W Inlet	0.0114	0.0002
	S Inlet	0	0
	Bridge	0	0
	Outlet	0.2734	0.0150
05-10-2019	W Inlet	0.0538	0.0009
	S Inlet	0	0
	Bridge	0.0400	0.0023
	Outlet	0.3392	0.0180
06-20-2019	W Inlet	0	0
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	0.0170	0.0004
07-16-2019	W Inlet	N/A	N/A
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	N/A	N/A
08-13-2019	W Inlet	N/A	N/A
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	N/A	N/A
08-30-2019	W Inlet	N/A	N/A
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	N/A	N/A
10-01-2019	W Inlet	N/A	N/A
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	N/A	N/A
10-30-2019	W Inlet	N/A	N/A
	S Inlet	N/A	N/A
	Bridge	N/A	N/A
	Outlet	N/A	N/A

N/A - Channel dry/too shallow, unable to complete measurement