July 29, 2020

REPORT ON

HYDROGEOLOGICAL INVESTIGATION

Proposed Sub-Division Development

17791 Mount Hope Road

Part of Lot 28, Concession 8

Town of Caledon, Region of Peel

Ontario

Prepared for:

Mr. Joe Triumbari Palgrave Development Inc 3 Manswood Crescent Brampton, ON L6P 0A3

Prepared By:

SIRATI & PARTNERS CONSULTANTS LIMITED



ieotechnical Hydrogeological & Environmental Solutions

12700 Keele Street, King City Ontario, L7B 1H5 Tel: 905-833-1582 Fax: 905-833-5360

Project: SP18-334-00 November 22, 2019

TABLE OF CONTENTS

SEC	SECTION			
1.0	INTRODUCTION AND BACKGROUND	1		
2.0	SCOPE OF WORK			
	LAND USE			
3.0	LAND USE	Z		
4.0	ENVIRONMENTAL FEATURES			
5.0	PHYSICAL SETTING	4		
5.1	TOPOGRAPHY AND DRAINAGE	4		
5.2	Physiography	4		
5.3	OVERBURDEN	5		
5.4	BEDROCK	5		
6.0	HYDROGEOLOGY	5		
6.1	PRIVATE WATER WELLS	5		
6.2	PALGRAVE MUNICIPAL WATER SUPPLY WELLS	6		
7.0	FIELD WORK METHODOLOGY	6		
7.1	BOREHOLE DRILLING AND MONITORING WELL INSTALLATION	6		
7.2	GROUNDWATER MONITORING AND ELEVATION SURVEY	8		
7.3	HYDRAULIC CONDUCTIVITY TEST (SINGLE WELL RESPONSE TEST/SLUG TEST	ST)8		
8.0	SUMMARIZED SITE CONDITIONS			
8.1	SOIL STRATIGRAPHY	8		
8.2	GROUNDWATER CONDITIONS	9		
8.2.1	Groundwater Levels and Elevations	9		
8.2.2	Groundwater Flow Direction and Hydraulic Gradients			
8.3	ESTIMATED HYDRAULIC CONDUCTIVITY			
9.0	CONSTRUCTION DEWATERING REQUIREMENTS	11		
10.0	ASSESSMENT OF IMPACTS DUE TO SITE DEVELOPMENT			
10.1	PRIVATE AND PUBLIC WATER WELLS	12		
10.2	EPHEMERAL CREEK OR OPEN WATER BODY			

10.3	WELL HEAD PROTECTION AREA (WHPA)	
10.4 Aquife	SIGNIFICANT GROUNDWATER RECHARGE AREA (SGRA) AND HIG r (HVA)	
11.0	WATER BALANCE	
11.1	SITE CONDITION	
11.2	SITE LEVEL WATER BALANCE	14
11.3	Сыматіс Дата	
11.4	INFILTRATION AND RUNOFF	
11.5	SUMMARY OF WATER BALANCE CALCULATION	
12.0	CONCLUSIONS AND RECOMMENDATIONS	
13.0	SELECTED BIBLIOGRAPHY	21
14.0	LIMITATIONS AND USE OF THE REPORT	

FIGURES
Figure 1-1 – Site Location Plan
Figure 3-1 – Innisfil Subwatershed Location Map
Figure 3-2 – Land Use Designation Map-Oak Ridges Moraine Conservation Plan
Figure 4-1 – Natural Features Area Map
Figure 4-2 – NVCA Regulated Areas Map
Figure 4-3 – SGRA and HVA Areas Map
Figure 4-4 – Wellhead Protection Area Map
Figure 5-1 – Topographic Map
Figure 5-2 – Physiography Map
Figure 5-3 – Surficial Geology Map
Figure 5-4 – Bedrock Geology Map
Figure 6-1 – MECP Water Well Location Plan
Figure 7-1 – Borehole/Monitoring Well Location Plan
Figure 8-1 – Cross Section Profile A-A'
Figure 8-2 – Groundwater Elevation and Inferred Groundwater Flow Direction Map
Figure 11-1 - Pre-Development Plan – Site Statistics
Figure 11-2 – Post-Development Plan – Site Statistics

- Figure 11-3 Mean Annual temperature at the Site
- Figure 11-4 Mean Annual precipitation at the Site
- Figure 11-5 Monthly Average Temperature and Precipitation at the Site

APPENDICES

- Appendix A Site Development and Grading Plan
- Appendix B Borehole Logs
- Appendix C Slug Tests Results
- Appendix D Water Balance Calculations

1.0 INTRODUCTION AND BACKGROUND

Sirati & Partners (SIRATI) was retained by Palgrave Development Inc. (the Client) to conduct a Hydrogeological Investigation of a property located at 17791 Mount Hope Road, described as Part of Lot 28, Concession 8, in the Town of Caledon, Ontario (the Site). The approximate site location is presented in Figure 1-1.

The Site is located on the northeast side of Mount Hope Road, in the Town of Caledon and consists of a parcel of land with an area of approximately 41 hectares (102 acres). The Site is currently a cultivated land with no structures developed on the property. It is understood that a residential subdivision development consisting of twenty-nine (29) single homes has been proposed to be built on part of the property with a total development area of about 29.8 hectares. A recent site development grading plan dated October 2019 provided by the Client is presented in Appendix A.

The purposes of the investigation were to characterize the soil and groundwater conditions at the Site, assess the pre- and post-construction water balances, identify the potential impacts of land development on local groundwater and surface water resources and/or natural environments, and provide mitigative options, when required.

2.0 SCOPE OF WORK

This hydrogeological investigation was carried out consisting of the following scope of work:

- **Review of available background information**: a review of available geological and hydrogeological information for the Site and surrounding areas was conducted to understand the regional geological and hydrogeological settings.
- **Review of available investigation reports**: a review of available subsurface investigation reports completed for the Site to understand the soil and groundwater conditions of the Site.
- Site inspection: an inspection of the Site to review existing site conditions including identification of any hydrogeological features such as significant areas of potential groundwater recharge or areas of groundwater discharge.
- **Completion of Boreholes/Monitoring Wells:** boreholes and monitoring wells were completed at the Site to obtain the information of soil and groundwater at the Site.

- **Groundwater monitoring:** Groundwater levels were measured in the monitoring wells installed at the Site to obtain the groundwater level conditions in the site area for interpretation of groundwater flow directions.
- In-situ hydraulic conductivity tests: In-situ hydraulic conductivity tests (or single well response tests) in the selected existing monitoring wells to estimate the hydraulic conductivity of the underlying soils.
- Water balance (Preliminary): a water balance assessment was completed using the Thornthwaite water balance method for the proposed development as part of the hydrogeological study. The water balance study was based on available climatic information associated with pre-development and proposed post-development conditions at the subject lands.
- Data process and report preparation: the data obtained from this hydrogeological investigation was reviewed and processed, and a report was prepared summarizing the results and findings of the investigation.

It should be noted that this hydrogeological investigation was carried out concurrently with a geotechnical investigation conducted by SIRATI, which consisted of advancing ten (10) boreholes and installation of five (5) monitoring wells at the Site. The results of geotechnical investigation were summarized in a separate report. However, the data obtained was utilized in this hydrogeological report. In addition, the monitoring wells were used for groundwater monitoring in this hydrogeological investigation.

3.0 LAND USE

The Site is geographically located in Nottawasaga Valley Watershed under the jurisdiction of Nottawasaga Valley Conservation Authority (NVCA). Figure 3-1 presents the approximate location of the Site.

The Site consists of an undeveloped parcel of land and is bounded by Mount Hope Road further with residential houses to the southwest, by a vegetated wood lot and a residential property to the northwest, and by woodlots further with residential houses to the southeast and northeast.

The Site is located in Town of Caledon, within the Municipal Region of Peel or Peel Region. Based on Town of Caledon Official Plan, the Site is located in a zoning area identified as rural area on Oak Ridges Moraine (A2-ORM). As shown in Figure 3-2, the Site is located, in an area designated as Palgrave Estate Residential Community land use area within Oak Ridges Moraine Conservation Plan area (a component of Countryside Area), where residential development is permitted.

4.0 ENVIRONMENTAL FEATURES

To assess the natural environmental features, the databases maintained by the Ministry of Natural Resources and Forestry (MNRF), the Ministry of Environment, Conservation and Parks (MECP), Nottawasaga Valley Conservation Authority (NVCA) and Toronto and Region Conservation Authority (TRCA) were reviewed.

As shown on Figure 4-1, no wetland, provincial parks, reserve area or area of natural heritage and scientific interest (ANSI) was found on or near the Property. However, the following features have been identified.

- The Property is located in Oak Ridges Moraines (ORM) area. The ORM is an irregularly shaped geologic feature, formed from glacial sand and gravel deposition and is classified as Significant Recharge Area due to the presence of coarse-grained sand and gravel deposits with layers of silty clay to clayey silt till, protecting the water supply aquifers. The Site, however, as shown in Figure 3-2, is located in Palgrave Estate Residential Community land use area, where residential development is permitted.
- 2) The majority of the Site is located in Nottawasaga Valley Watershed, in Innisfil Creek subwatershed. A very small portion near the southwest property boundary is located in Humber River Watershed under the jurisdiction of Toronto and Region Conservation Authority (TRCA). As shown on Figure 4-1, unnamed creeks or tributaries are located crossing the middle of the property. As a result, part of the Site is located in NVCA regulated areas, which is shown on Figure 4-2.
- 3) As shown on Figure 4-3, the Site is located in an area identified as Significant Groundwater Recharge Areas (SGRAs). SGRAs are geographic areas within which it is desirable to regulate or monitor drinking water threats that may affect the recharge to an aquifer. SGRAs are the areas where the recharge is 15% greater than the average recharge across the study area.
- 4) As shown on Figure 4-4, the whole property of the Site is located in the wellhead protection area (WHPA) -D, (25 year time of travel) and part of the property is located in WHPA-C (5 year time of travel) for the Palgrave municipal system, with a vulnerability score of 2 ~6 out of 10. A wellhead protection area is the area around the wellhead where land use activities have the potential to affect the quality of water that flows into the well.
- 5) A small portion of the Property (shown on Figure 4-3) along east side of Mount Hope Road is located in an area identified as a highly vulnerable aquifer (HVA). HVA is one that is particularly

susceptible to contamination because of either its location near the ground's surface or because of the type of materials found in the ground around it.

5.0 PHYSICAL SETTING

5.1 **Topography and Drainage**

As shown in Figure 5-1, the topography of the Site ranges in elevation generally between 290 mASL and 310 mASL, mostly around 300 mASL in the southwest of the Property. An unnamed creek crossing from the northwest to the southeast divides the property in two (2) parts.

The Site is almost entirely located within the Nottawasaga Valley Watershed which has nine (9) subwatersheds. The Innisfil Creek subwatershed is one of the nine (9) subwatersheds, and consists of four main creek systems, namely Innisfil Creek, Bailey Creek, Beeton Creek and Penville Creek that drain the southeast portion of the Nottawasaga River watershed. The Subject Property falls within the Beeton Creek system.

Beeton Creek arises on the Oak Ridges Moraine south of Tottenham. Flowing north, the creek enters a reservoir at the Tottenham Conservation Area and then continues downstream. An east branch, originating east of Tottenham, flows westward through agricultural lands and enters Beeton Creek north of Tottenham. Beeton Creek continues to flow northward through an agricultural landscape, skirting the west side of Beeton before joining Bailey Creek and then entering Innisfil Creek.

As shown in Figures 3-1 and 5-1, the unnamed tributary flows southeasterly and joins into a main tributary of Beeton Creek, which flows northwards and joins into Innisfil Creek.

5.2 **Physiography**

The Nottawasaga Valley Watershed is located within five (5) main regional-scale physiographic regions as defined by Chapman and Putnam (1984). These regions include the Horseshoe Moraines, Oak Ridges Moraine, Peterborough Drumlin Field, Schomberg Clay Plains and Simcoe Lowlands (Figure 5-2).

The Subject Property lies within the physiographic region termed as Oak Ridges Moraine. The Oak Ridges Moraine is comprised of rolling sandy hills, hummocky topography and closed depressions that form the source of the headwaters to major stream that drain off the moraine. The moraine within the subwatershed consists primarily of surficial sand and gravel deposits.

5.3 Overburden

As shown in Figure 5-3, the Subject Property is located in an area covered with ice-contact stratified deposits, which generally consist of gravel and sand deposits, with minor till, and also contain esker, kame, end moraine, ice-marginal delta and subaqueous fan deposits.

The Paleozoic bedrock topography appears to strongly influence the overlying Quaternary sediment thickness and distribution. The thicker Quaternary sediments occur in bedrock topographical lows (i.e. within bedrock valleys and beneath the ORM). The overburden thickness within the Oak Ridges Moraine (ORM) ranges from approximately 56 m to 240 m.

5.4 Bedrock

The bedrock consists of shale, interbedded dolomitic siltstone, and minor limestone, which were deposited in shallow seas about 450 million years ago. These beds, named the Georgian Bay Formation are approximately 250 m thick and dip to the southeast at about 5 m/km. Following long periods of additional sedimentation and erosion, the ancient Laurentian River and its tributaries cut several deep, poorly-defined bedrock valleys trending northwest-southeast across the area. As depicted in Figure 5-4, the study area is underlain by the Georgian Bay Formation, which has an important influence on drift thickness and groundwater distribution in the study area.

6.0 HYDROGEOLOGY

The regional hydrogeological conditions were assessed using the data obtained from the MECP water well database including domestic water wells and municipal water wells.

6.1 **Private Water Wells**

As shown in Figure 6-1, a total of thirty (30) water wells have been found within a 500 m radius around the property. One (1) record for an abandoned well is found on Site.

Based on the well records, bedrock was encountered at three (3) wells (#4906507, #4906507 and #4906507) at the depth of $74.1 \sim 74.4$ mbgs, indicating presence of thick overburden sediment at and/or near the Site. In addition, layers of sand and/gravel were recorded, where the well screens were installed. The recorded water levels ranged from approximately 14 mbgs to 26 mbgs. Therefore, several aquifers may be present in the overburden deposits.

6.2 Palgrave Municipal Water Supply Wells

The Palgrave community is being serviced by Palgrave municipal supply wells #2, #3 and #4. The Subject Property is situated within a wellhead protection zone, which is associated with the municipal supply well Palgrave Well#3.

Palgrave Well#3 is located about 220 m southeast of the Site, on the northeast side of Mount Hope Road, in Caledon, Ontario. Geologically, Palgrave Well#3 is located near the edge of a local bedrock valley and is overlain by about 80 m thick overburden of Oak Ridges Moraine Aquifer Complex (ORMAC) stratified sediments. This sequence consists (from surface downward) of about four metres of sand and gravel, about 35 m of silt and clay with minor sand, about 25 m of gravelly sand and silty sand, a silt and clay unit about five metres thick, and finally, about 10 m of gravelly sand where the well is screened (Earthfx , May 2007).

Based on MECP water well record, Palgrave Well#3 was found to have a well ID of #4906859, and was drilled to the depth of approximately 82.3 mbgs into (sandy) clay, and screened between 71.3 mbgs and 80.5 mbgs in (gravelly) sand. The static water level was recorded to be 15.1 mbgs.

Based on the above findings, it can be inferred that layers of a significant thickness of unconsolidated overburden is present in the site area, and multiple sandy or gravelly soils can serve as the aquifer for water supply. However, it is more common that the deeper aquifer is selected for the water supply aquifer, probably because of having a thicker and better protection from the potential contamination.

7.0 FIELD WORK METHODOLOGY

7.1 Borehole Drilling and Monitoring Well Installation

As part of geotechnical investigation, a total of ten (10) boreholes (BH1 to BH10) were drilled on October 18 and 19, 2018, to depths ranging from 9.2 mbgs to 9.7 mbgs. Five (5) monitoring wells identified as BH/MW1, MW/BH3, MW/BH6, MW/BH9 & MW/BH10B) were installed at five (5) locations for subsequent groundwater monitoring and testing.

Boreholes were drilled with solid stem continuous flight auger equipment by a drilling sub-contractor under the direction and supervision of SIRATI personnel. Samples were retrieved at regular intervals with a 50 mm O.D. split-barrel sampler driven with a hammer weighing 624 N and dropping 760 mm in accordance with the Standard Penetration Test (SPT) method. The samples were logged in the field and returned to the SIRATI laboratory for detailed examination by the project engineer and for laboratory testing. Due to the completed dry wells, three (3) additional monitoring wells (BH/MW19-1 to BH/MW19-3) were installed on August 23, 2019 to depths ranging from 9.1 mbgs to 18.3 mbgs. Again, two (2) monitoring wells BH/MW19-1 and BH/MW19-3 were found dry. Therefore, two additional monitoring wells (BH/MW19-1A and BH/MW19-3A) were completed at these two (2) locations, during October 10 and 11, 2019.

The elevations at the borehole locations were surveyed by the SIRATI personnel using a differential GPS system. The approximate borehole/monitoring well locations are shown on Figure 7-1. The borehole logs are provided in Appendix B.

The construction details for the monitoring wells are presented in Table 7-1 below.

Monitoring Well	Borehole Depth (mbgs)	Screen Depth (mbgs)	Screened Soil Type
BH/MW1	9.2	4.0 ~ 7.2	Sand; Gravelly Sand
BH/MW3	9.3	3~6.0	Sand
BH/MW6	9.7	3~6.0	Sand
BH/MW9	9.7	4.6 ~ 7.6	Gravelly Sand
BH/MW10B	9.7	3~6.0	Sand
BH/MW19-1	18.3	10.7 ~ 13.7	Sand; Sand and Gravel
BH/MW19-1A	16.8	13.8 ~ 16.8	Sand
BH/MW19-2	9.1	6.1 ~ 9.1	Sand
BH/MW19-3	14	11 ~ 14	Sand
BH/MW19-3A	16.8	13.4 ~ 16.5	Likely Sand

Table 7-1:Well Construction Details

7.2 Groundwater Monitoring and Elevation Survey

After the well installation, groundwater levels were measured in the existing and new monitoring wells. In addition, the location and elevation survey were conducted using a GPS unit on the monitoring wells installed at the Site.

7.3 Hydraulic Conductivity Test (Single Well Response Test/Slug Test)

In-situ hydraulic conductivity tests, also called as single well response test or slug test, were conducted on October 8 and 21, 2019 in three (3) monitoring wells (BH/MW19-1A, BH/MW19-2 and BH/MW19-3A).

Prior to the test, initial water levels were measured. Then, a datalogger was placed in each of the test wells to record the initial water pressure. To start the test, a certain amount of water was pumped out from a test well using a Waterra pump (Waterra tubing with a foot valve) for a rising head test or introduced into the test well for a falling head test, and the recovery of the water level was recorded by the datalogger. The recorded data was then used for estimating the hydraulic conductivity of the screened soil.

8.0 SUMMARIZED SITE CONDITIONS

8.1 Soil Stratigraphy

The soil stratigraphy of the Site as revealed in the boreholes generally consisted of topsoil and fill materials, underlain by native soils predominantly comprised of cohesionless soils, locally with cohesive soils. No bedrock was encountered at the maximum explored depth of 18.3 mbgs. A summary of the encountered soils is made as follows.

Topsoil: A 300 mm to 450 mm thick surficial layer of topsoil was found at all borehole locations except BH4, BH19-1/1A, BH19-2 and BH19-3/3A. The thickness of the topsoil in each borehole is shown in the respective borehole logs. It should be noted that the thickness of the topsoil observed at the borehole locations may not be representative for the entire site and should not be relied on to calculate the amount of topsoil that need to be stripped from the Site.

Fill Material/Possible Fill: Below the topsoil, where present, a zone of earth fill/disturbed native material was encountered in all boreholes. The fill layer was found extending to depths varying between 0.2 m (BH19-1/1A) and 3 m (BH6) below the existing ground surface. The fill material mainly consisted of silty sand, sandy silt and sand, with trace to some topsoil inclusion at some borehole locations. The composition of the weathered/disturbed sand was generally similar to that of the underlying undisturbed native soils.

<u>Cohesionless Layers</u>: Cohesionless layers of silt, sandy silt, silty sand, sand, gravelly sand, and sand and gravel were encountered in all borehole locations. The native cohesionless layers were encountered at different depths ranging from 0.2 m to 3.0 m below the existing ground surface and extended to the termination depth at all borehole locations.

<u>Cohesive Layer:</u> A thin cohesive layer of clayey silt was observed locally in BH5, extending from 1.5 m to 2.3 m depth.

Grain size analysis and Atterberg limits tests of a representative sample from this stratum (B5/SS3) was conducted and the results are presented in Figures 12 and 14, with the following fractions:

 Clay:
 27%

 Silt:
 54%

 Sand:
 17%

 Gravel:
 2%

The details of the soil stratigraphy are presented in Borehole Logs in Appendix B. In addition, one (1) cross section (as shown on Figure 8-1) was constructed to illustrate the horizontal and vertical extents of the soil and groundwater conditions.

8.2 Groundwater Conditions

Groundwater conditions were observed during the borehole drilling. Wet soils were observed at BH7, BH19-1, BH19-1A and BH19-2 at the depths from 3 mbgs to 15.3 mbgs.

8.2.1 Groundwater Levels and Elevations

Groundwater monitoring was conducted in the monitoring wells installed at the Site on November 1 and 27, 2018, April 10, 2019, August 30, 2019, September 5, October 8, October 18, and October 21, 2019. Dry wells were noted during the groundwater monitoring in all the monitoring events at BH/MW1, BH/MW3, BH/MW10B. The groundwater levels measured in October 2019 are presented in Table 8-1 below.

Monitoring Well	Ground Elevation (mASL)	Screen Depth (mbgs)	Depth to Ground water (mbgs)	Ground water Elevation (mASL)	Depth to Ground water (mbgs)	Ground water Elevation (mASL)	Depth to Ground water (mbgs)	Ground water Elevation (mASL)
			Octobe	er 8, 2017	October	· 18, 2019	October	21, 2019
BH/MW1	298.92	$4.0 \sim 7.2$	dry	dry	dry	dry	dry	dry
BH/MW3	297.75	3~6.0	dry	dry	dry	dry	dry	dry
BH/MW6	299.93	3~6.0	dry	dry	dry	dry	dry	dry
BH/MW9	298.19	$4.6 \sim 7.6$	dry	dry	dry	dry	dry	dry
BH/MW10B	295.67	3 ~ 6.0	dry	dry	dry	dry	dry	dry
BH/MW19-1	299.05	10.7 ~ 13.7	12.98	286.07	-	-	-	-
BH/MW19-1A	299.05	13.8 ~ 16.8	-	-	14.84	284.21	14.85	284.20
BH/MW19-2	298.17	6.1 ~ 9.1	6.33	291.84	6.53	291.64	6.52	291.65
BH/MW19-3	298.24	$11 \sim 14$	Dry	dry	-	-	-	-
BH/MW19-3A	298.24	13.4 ~ 16.5	-	-	14.45	283.79	14.44	283.80

 Table 8-1:
 Historical Recorded/Measured Groundwater Levels

As presented above, the measured groundwater levels ranged from 6.33 mbgs at BH/MW19-2 to 14.85 mbgs at BH/MW19-1A, and elevations ranged from 283.79 mASL at BH/MW19-3A to 291.65 mASL at BH/MW19-3A.

8.2.2 Groundwater Flow Direction and Hydraulic Gradients

Based on the groundwater elevation data obtained on October 21, 2019, a site-specific groundwater elevation contour map was prepared to present the inferred the groundwater flow direction. As shown in Figure 8-2, the groundwater flow direction was inferred to be generally northerly.

The hydraulic gradient is a gradient or slope between two or more hydraulic head measurements over the length of the flow path. The hydraulic gradients help determine the groundwater flux or discharge. Groundwater will flow down the hydraulic gradient. The horizontal hydraulic gradients across the development area were calculated to assess the general slope of the groundwater flow path. Accordingly, the average gradient across the Site was calculated at 0.012 m/m.

Monitoring Well	Groundwater Elevation (mASL)		Distance Between Monitors (m)	Gradient (m/m)
BH19-1A to BH19-2	284.20	291.65	343	0.021
BH19-2 to BH19-3A	291.65	283.80	505	0.015
BH19-1A to BH19-3A	284.20	283.80	477	0.001
AVERAGE				0.012

Table 8-2: Horizontal Hydraulic Gradients – 17791 Mt. Hope Road, Caledon, Ontario

8.3 Estimated Hydraulic Conductivity

The hydraulic conductivity (K-value) of the soils was estimated based on the results obtained from the single well response tests (slug tests).

The data was processed utilizing Aqtesolv pumping test software with the Hvorslev method. Records of the slug tests and the data processing are provided in Appendix C. The results of the estimated hydraulic conductivity are summarized in Table 8-3.

Table 8-3: Results of Estimated Hydraulic Conductivity as per Slug Tests

Borehole/Monitoring Well	Hydraulic Conductivity (m/s)	Screened Soil Type
BH19-1A	3.83 x 10 ⁻⁵	Sand, some silt, trace clay
BH19-2	3.29 x 10 ⁻⁷	Sand, some clay
BH19-3A	1.23 x 10 ⁻⁴	Sand
Average	1.15 x 10 ⁻⁵	

As presented above, the estimated hydraulic conductivity ranged from 3.29×10^{-7} m/s to 1.23×10^{-4} m/s, with a geometric average value of 1.15×10^{-5} m/s.

9.0 CONSTRUCTION DEWATERING REQUIREMENTS

Dewatering during the construction is intended to lower the groundwater levels in the excavation area in order to ensure a dry working condition. If groundwater levels are shallow, groundwater control such as using weeping tiles and foundation drainage system will also be considered to avoid hydrostatic pressure of shallow groundwater on the footing walls as well as to achieve a dry condition for the underground levels.

The requirements for construction dewatering generally depend on the site's soil and groundwater conditions including soil type, soil permeability or hydraulic conductivity, local groundwater levels, and the design of the proposed development, such as the foundation and/or basement elevation, as well as the size of proposed structure, etc.

Based on the latest grading plan in Appendix A, the lowest point of the finished road way is 296.7 mASL. Assuming that the finished ground floor elevation of the buildings will match the roadway elevation and the proposed residential houses will have one-level basement about 3 m below the ground floor, the lowest elevation of the basement will be at 293.7 mASL.

According to the groundwater monitoring, the measured groundwater elevations ranged from 283.79 mASL to 291.65 mASL, which is below the basement level. Therefore, there will be no requirements for construction dewatering either temporary dewatering or long-term drainage/dewatering.

10.0 ASSESSMENT OF IMPACTS DUE TO SITE DEVELOPMENT

10.1 Private and Public Water Wells

As discussed, no dewatering will be required during and after the construction. Therefore, there will be no impact on water wells located near the Site, since the existing water wells were screened or constructed much below the excavation level of the proposed development.

10.2 Ephemeral Creek or Open Water Body

Based on review of the data and observation, no open water body was found to be present on the Property. The water course noted in the central portion of the Site was found to be ephemeral without a well-defined water channel or water flow.

Based on the site development grading plan, a setback from the existing channel has been proposed and the existing grading will remain undisturbed or any grading disturbance will be limited to proposed road areas and around the proposed housed. Therefore, the impact, if any, on the ephemeral creek will be very minor.

10.3 Well Head Protection Area (WHPA)

As no dewatering will be anticipated, there should be no quantitative impact on the use of the municipal well – Palgrave Well#3.

However, it is known that the Site lies within the Palgrave municipal supply well No. 3 wellhead protection area (WHPA). As shown on Figures 4-4, most part of the Site is located within the WHPA-D (5 to 25-year time of travel zone), an area with a vulnerability score of 2 to 4 (low to medium score), while some part lies within WHPA-C (2 to 5-year time of travel zone), with a vulnerability score of 4 to 6. WHPA-C is DNPL/ Contaminant Protection Zone.

From the qualitative aspect, if the best construction and management practices are to be implemented, there would be no contamination during and after the development on the Property, and therefore, the impacts will be considered to be minimal to minor.

10.4 Significant Groundwater Recharge Area (SGRA) and Highly Vulnerable Aquifer (HVA)

Based on the NVCA database, the Property is located within a Significant Groundwater Recharge Area (SGRA), and part of Property is situated within the area with Highly Vulnerable Aquifer (Figure 4-3).

It is anticipated that the excavation for construction of the basement will extend to the depth about three (3) metres below the ground surface, which is very shallow as compared to the phreatic water table elevation and depth of the potential local aquifers.

However, the development may increase the impervious surface, which will result in the decrease in the filtration or the recharge of the groundwater. In addition, application of de-icing salt on the roadways in winter seasons for traffic safety purpose or other activities which involve potential contaminants may be an environmental concern related to the quality of recharge water.

11.0 WATER BALANCE

A preliminary water balance for the Site was calculated for both pre-development and post-development conditions in order to assess the change in overall rate of infiltration.

11.1 Site Condition

The Site is currently undeveloped with no structures or paved areas. It is understood that the Site has a total area of approximately 41 hectares.

Based on the design information, the site development area can be generally categorized into two (2) major types as Impervious Area (including roof and paved area) and Pervious Area (landscape and/or vegetated area). The details of the areas are shown on Figures 11-1 and 11-2, and summarized in Table 11-1.

Type of I	Land Coverage	Pre-Development Area (m²)	Post- Development Area (m²)
Impervious Area	Roofs	-	5000
imper vious r neu	Roadway/Paving/Parking	-	15000
Pervious Area Landscape/Vegetated Area		410,000	390000
	Total	410,000	410,000

Table 11-1 Pre-and Post-development Site Statistics

11.2 Site Level Water Balance

Based on the Thornthwaite and Mather methodology (1957), the water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from ground or evapotranspiration by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage (Δ S).

The annual water budget can be expressed as:

$$\mathbf{P} = \mathbf{E}\mathbf{T} + \mathbf{R} + \mathbf{I} + \Delta\mathbf{S}$$

Where:

P = Precipitation (mm/year)

ET = Evapotranspiration (mm/year)

R = Run-off (mm/year)

I = Infiltration (mm/year)

 ΔS = Change in groundwater storage (taken as zero) (mm/year)

11.3 Climatic Data

The climatic data including monthly average temperature and precipitation were obtained from Environment Canada, for Orangeville WPCP station (climate identifier: 6155790) as the nearest station located at about 8 km distance from the Site.

The data was available for the years from 1962 to 2006. Temporal variations of annual temperature and precipitation are shown in Figures 11-3 and 11-4.

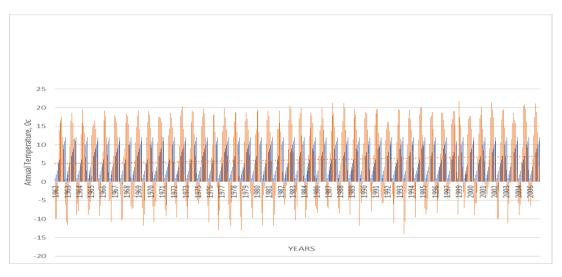


Figure 11-3 Mean Annual Temperature at the Site

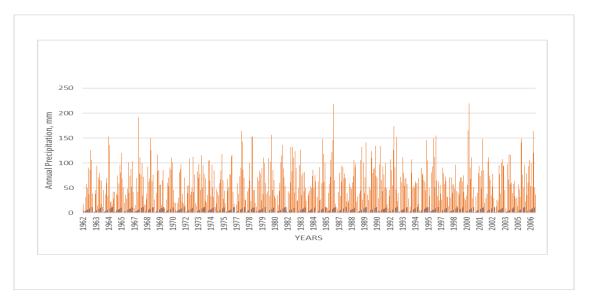


Figure 11-4 Mean Annual Precipitation at the Site

Average monthly variations of both temperature and precipitation were calculated for the period from 1962 to 2006 and is presented below in Figure 11-5. The highest temperature was recorded in the month of July, while the highest rainfall was in the month of August.

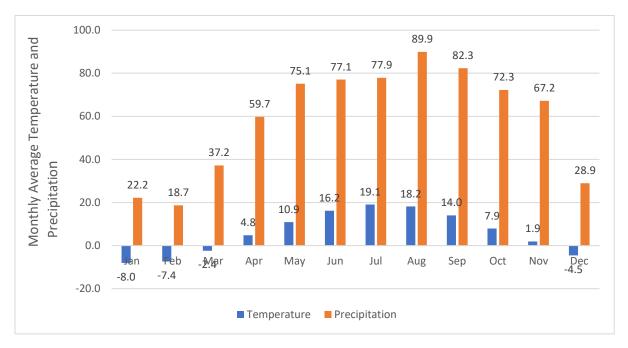


Figure 11-5 Average Monthly Temperature at the Site

Based on the data for the precipitation and temperature, actual evapotranspiration was estimated to be about 482 mm/annum using the USGS Thornthwaite Monthly Water Balance software (Appendix D), and the average annual precipitation was recorded to be 901 mm/annum.

11.4 Infiltration and Runoff

As mentioned above, the actual evapotranspiration was estimated to be 482 mm/annum. Given the average annual precipitation of 901 mm/annum, there is a water surplus of 419 (=901-482) mm/annum occurring at the Site, which can either infiltrate into subsurface or go as run-off.

The rate of infiltration at a site is expected to vary, based on a few factors to be considered in any infiltration model. To partition the available water surpluses into infiltration and surface run-off, the Ministry of Environment, Conservation and Parks (MECP) infiltration factor was used. The MECP Storm Water Management Planning and Design Manual (2003) methodology for calculating total infiltration based on

topography, soil type and land cover was used, and a corresponding run-off component was calculated for the soil moisture storage conditions.

The calculation of infiltration and runoff in the stages of pre-development and post-development is provided in Appendix D, and are presented in Tables 11-2 to 11-5, below.

Land	Use	Area (m²)	Precipitation (m ³)	Evapotranspiration (m ³)	Infiltration (m ³)	Run-off (m ³)
Impervious	Paved Area	0	0	0	0	0
	Roof Area	0	0	0	0	0
	Landscape					
Pervious	Area	410000	369410	197620	103074	68716
Total		410000	369410	197620	103074	68716
Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.						

Table 11-2: Annual Pre-Development Water Balance

 Table 11-3: Annual Post-Development Water Balance

Land U	Jse	Area (m²)	Precipitation (m ³)	Evapotranspiration (m ³)	Infiltration (m ³)	Run-off (m ³)	
	Paved						
	Area	15000	13515	1352	0	12164	
Impervious	Roof Area	5000	4505	451	0	4055	
	Landscape						
Pervious	Area	390000	351390	187980	98046	65364	
Total		410000	369410	189782	98046	81582	
Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.							

Table 11-4: Comparison of Pre- and Post Development Water Balance Components.

	Precipitation (m ³)	Evapotranspiration (m ³)	Infiltration (m ³)	Run-off (m ³)
Pre-Development	369410	197620	103074	68716
Post-Development	369410	189782	98046	81582
Change in Volume			5028	12866
Change in Percentage (%)			-5	19

Table 12-5: Requirement for Infiltration of Roof Run-off.

Volume of Pre-Development Infiltration (m ³)	103074
Volume of Post-Development Infiltration (m ³)	98046
Deficit from Pre to Post Development Infiltration (m ³)	5028
Percentage of Roof Runoff required to match the pre-development infiltration (%)	124

11.5 Summary of Water Balance Calculation

Based on the above calculations, a summary of water balance could be provided as below:

- Without implementation of mitigation measures, there is a net increase in run-off at the Site of about 12,866 m³/annum (or 19% increase), from 68,716 m³/annum to 81,582 m³/annum. This increase is a result of the development of the Site with more impervious areas such as roof and paved areas, and reduction in pervious areas.
- Without implementation of mitigation measures, there is a net deficit of about 5,028 m³/annum (or 5 % decrease) in the post-development infiltration from 103,074 m³ to 98,046 m³ on a yearly basis.
- 3) There is a volume of 4,055 m³/annum collected from the roof area, which can be used for the enhanced infiltration for the purpose of implementing the Low Impact Development (LID) measures. However, it is not enough to compensate for the deficit in infiltration.

12.0 CONCLUSIONS AND RECOMMENDATIONS

This report was prepared by SIRATI in support of a proposed residential development at the Site located at 17791 Mount Hope Road, Town of Caledon, Ontario. Based on the hydrogeological investigation conducted on the Site, the following conclusions are presented:

- The Site geographically falls within the Innisfil Creek subwatershed under the jurisdiction of Nottawasaga Valley Conservation Authority (NVCA).
- As per the Oak Ridges Moraine Conservation Plan, 2017, the Subject Property falls within the Palgrave Estate Residential Community (a component of Countryside Area) land use designation area.
- The Site lies within the physiographic region termed as Oak Ridges Moraine, and is located in an area characterized by coarse-grained ice-contact glaciolacustrine sediments consisting of sand, gravelly sand and gravel.

- The Site is located within the Palgrave municipal supply well No. 3 wellhead protection area, within a Significant Groundwater Recharge Area (SGRA) and partly in a Highly Vulnerable Aquifer (HVA) area.
- The soil stratigraphy of the Site as revealed in the boreholes generally consisted of topsoil and fill materials, underlain by native soils predominantly comprised of cohesionless soils, locally with cohesive soils. No bedrock was encountered at the maximum explored depth of 18.3 mbgs.
- The groundwater levels measured in the monitoring wells at the Site ranged from 6.33 mbgs to 14.85 mbgs and elevations ranged from 283.79 mASL to 291.65 mASL. The groundwater flow was inferred to be generally in a northerly direction.
- The hydraulic conductivity estimated for the screened soils ranged from 1.23 x 10⁻⁴ m/s to 3.28 x 10⁻⁷ m/s, with a geometric average value of 1.15 x 10⁻⁵ m/s, which is in the typical range of hydraulic conductivity for sand and gravelly sand, as observed in the boreholes.
- Given the relatively deep groundwater levels found at the Site, construction dewatering (short-term or long-term) will not be required for the proposed development.
- A preliminary water balance assessment indicated that an infiltration deficit of approximately 5,028 m³/annum (about 13 m³ /day) will occur due to the proposed development.

Based on the findings of this hydrogeological investigation, the following measures would be considered and recommended to protect and preserve the SGRAs and HVAs,

- Incompatible land uses such as storage of chemicals and/or liquids should be avoided and directed away from the SGRAs.
- Since the proposed development is a major development (> 500 m²) within the SGRAs, an Infiltration Management Plan that demonstrates pre-development recharge rates will be maintained, may be a requirement.
- Low-impact development (LID) measures would be designed and implemented to maintain or improve the post-development infiltration and/or groundwater recharge conditions. The measures may include, but are not limited to the following:
 - Use of infiltration trenches or bio-swales at selected areas to maximize the infiltration.
 - Provision of pervious road or parking areas to enhance infiltration.

- Provision of an extra thickness of topsoil at the Site (approximately 0.3 m) on open areas to promote water storage in surficial soil and infiltration.
- Provision of gradual slopes to open areas and back-yards in order to allow extra time for surficial run-off to infiltrate into the topsoil.
- It would be necessary to promote awareness of the importance of SGRAs and HVAs by means of sign boards explaining the linkage between surface activities and their impact on groundwater quality and quantity.
- A salt management plan may be considered to be developed and implemented.
- As the Subject Lands fall within the areas of high aquifer vulnerability, the following uses are prohibited with respect to land in Areas of High Aquifer Vulnerability.
 - Generation and storage of hazardous waste or liquid industrial waste;
 - Waste disposal sites and facilities, organic soil conditioning sites, and snow storage and disposal facilities;
 - Underground and above-ground storage tanks that are not equipped with an approved secondary containment device; and,
 - Storage of a contaminant listed in Schedule 3 (Severely Toxic Contaminants) to Regulation 347 of the Revised Regulations of Ontario, 1990.

13.0 SELECTED BIBLIOGRAPHY

Construction Dewatering and Groundwater Control: New Methods and Applications, Third Edition. J. P. Powers, A. B. Corwin, Paul C. Schmall and W. E. Kaeck Copy Right © 2007 John Wiley & Sons, Inc. ISBN: 978-0-471-47943-7

Freeze, R. A. and Cherry, J. A., 1979, Groundwater, Prentice-Hall, Englewood Cliffs, New Jersey

Groundwater Lowering In Construction: A Practical Guide To Dewatering. 2nd Edition. Boca Raton: CRC Press, 2013. Print.

Ministry of Natural Resources, Ontario Geologic Survey, 1984, The Physiography of Southern Ontario, Third Edition, L. J. Chapman and D. F. Putnam

Ontario Geological Survey, Industrial Minerals Report 38, by D.F. Hewitt, S.E. Yundt, 1971

Ontario Geologic Survey, Preliminary Map P 2204, Quaternary Geology of Toronto and Surrounding

Area, Sharp D.R., 1980.

Ontario Ministry of Municipal Affairs and Housing (OMMAH). 1997. Supplementary Guidelines to the Ontario Building Code 1997, SG-6 Percolation Time and Soil Descriptions, Toronto, Ontario.

Sirati & Partners Consultants Ltd., Report on Preliminary Geotechnical Investigation, Proposed Residential Subdivision, Mount Hope Road, Caledon, Ontario, December 06, 2018 (SP18-334-10).

14.0 LIMITATIONS AND USE OF THE REPORT

This report was produced for the sole use of Palgrave Development Inc. c/o Mr. Joe Triumbari (the Client) for the property located at 17791 Mount Hope Road, Town of Caledon, Ontario and may not be relied upon by any other person or entity without the written authorization of Sirati & Partners Consultants Limited (SIRATI). The conclusions presented in this report are professional opinions based on the historical and current records search, visual observations and limited information provided by persons knowledgeable about past and current activities on this site. As such, SIRATI cannot be held responsible for environmental conditions at the Property that was not apparent from the available information. No investigation method can completely eliminate the possibility of obtaining partially imprecise or incomplete information; it can only reduce the possibility to an acceptable level.

Professional judgement was exercised in gathering and analyzing data and formulation of recommendations using current industry guidelines and standards. Similar to all professional persons rendering advice, SIRATI cannot act as absolute insurer of the conclusion we have reached. No additional warranty or representation, expressed or implied, is included or intended in this report other than stated herein the report.

The assessment should not be considered a comprehensive audit that eliminates all risks of encountering environmental problems. The information presented herein this report is primarily based on information collected during the hydrogeological study based on the condition of the Property at the time of site inspection/drilling followed by a review of historical data, as appended to this report.

In assessing the environmental setting of the Property, SIRATI has solely relied upon information supplied by others in good faith and has therefore assumed that the information supplied is factual and accurate. We accept no responsibility for any inaccurate information, misrepresentation or for any deficiency of the information supplied by any third party.

The scope of services performed in the execution of this investigation may not be appropriate to satisfy third parties. SIRATI accepts no responsibility for damages if any, suffered by any third party as a result of decisions made or action taken based on this report. Any use, copying or distribution of the report in whole or in part is not permitted without the express written permission of SIRATI and use of findings, conclusions and recommendations represented in this report, is at the sole risk of third parties.

In the event that during future work new information regarding the environmental condition of the Property is encountered, or in the event that the outstanding responses from the regulatory agencies indicate outstanding issues on file with respect to the Property, SIRATI should be notified in order that we may reevaluate the findings of this assessment and provide amendments, as required.

Should you have any questions regarding the information presented or limitation set in this report, please do not hesitate to contact our office.

Yours truly,

Sirati and Partners Consultants Limited

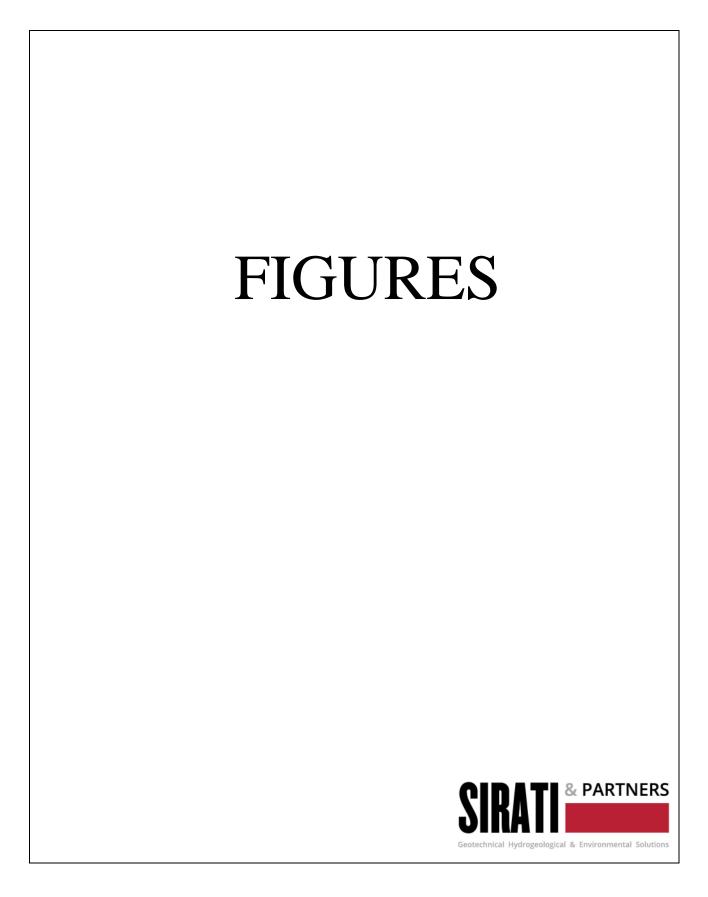


Sudhakar Kurli, M.Sc., P. Geo.

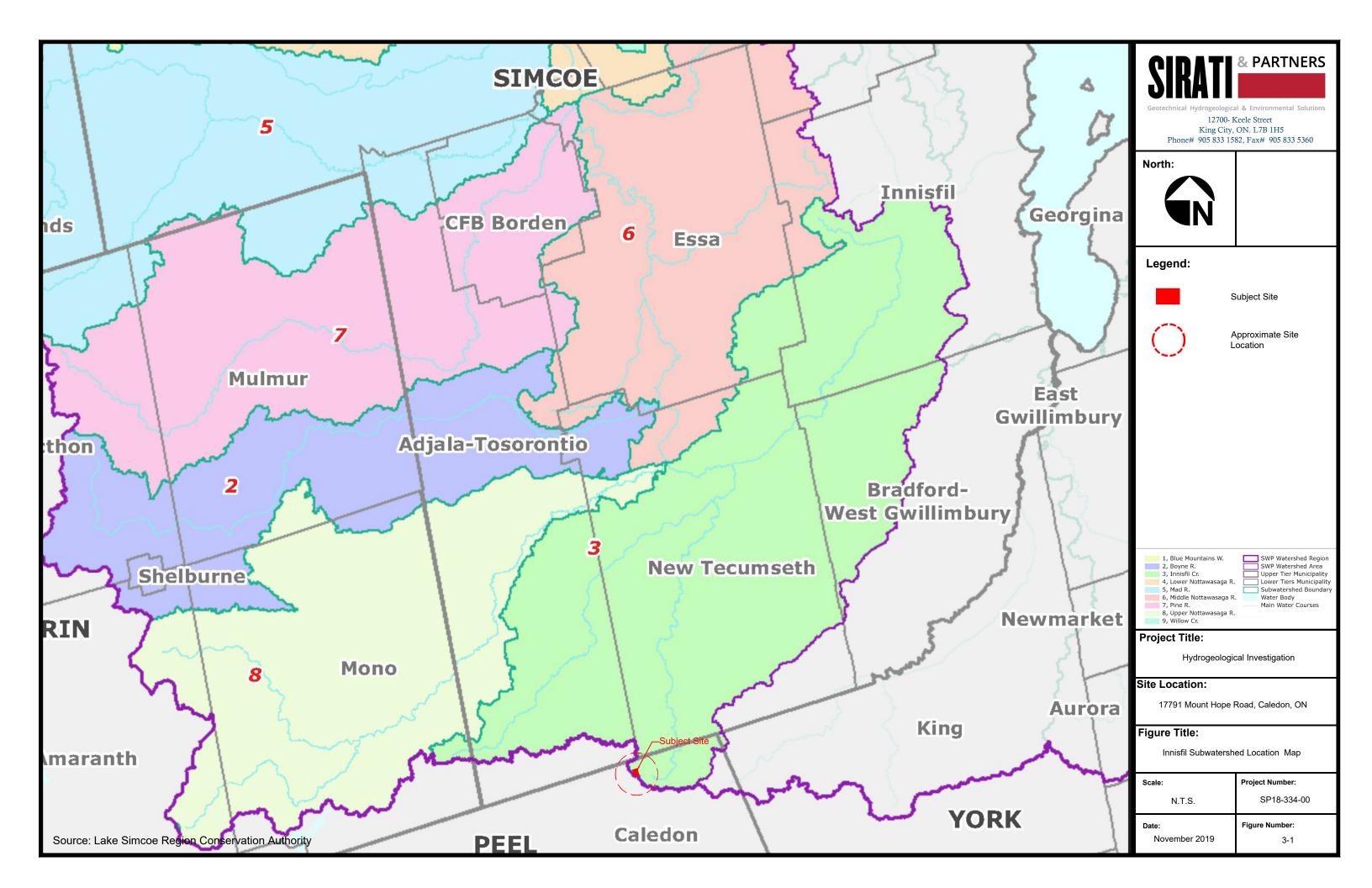
Hydrogeologist/Project Manager

Bujing Guan, M. A. Sc., P. Geo.

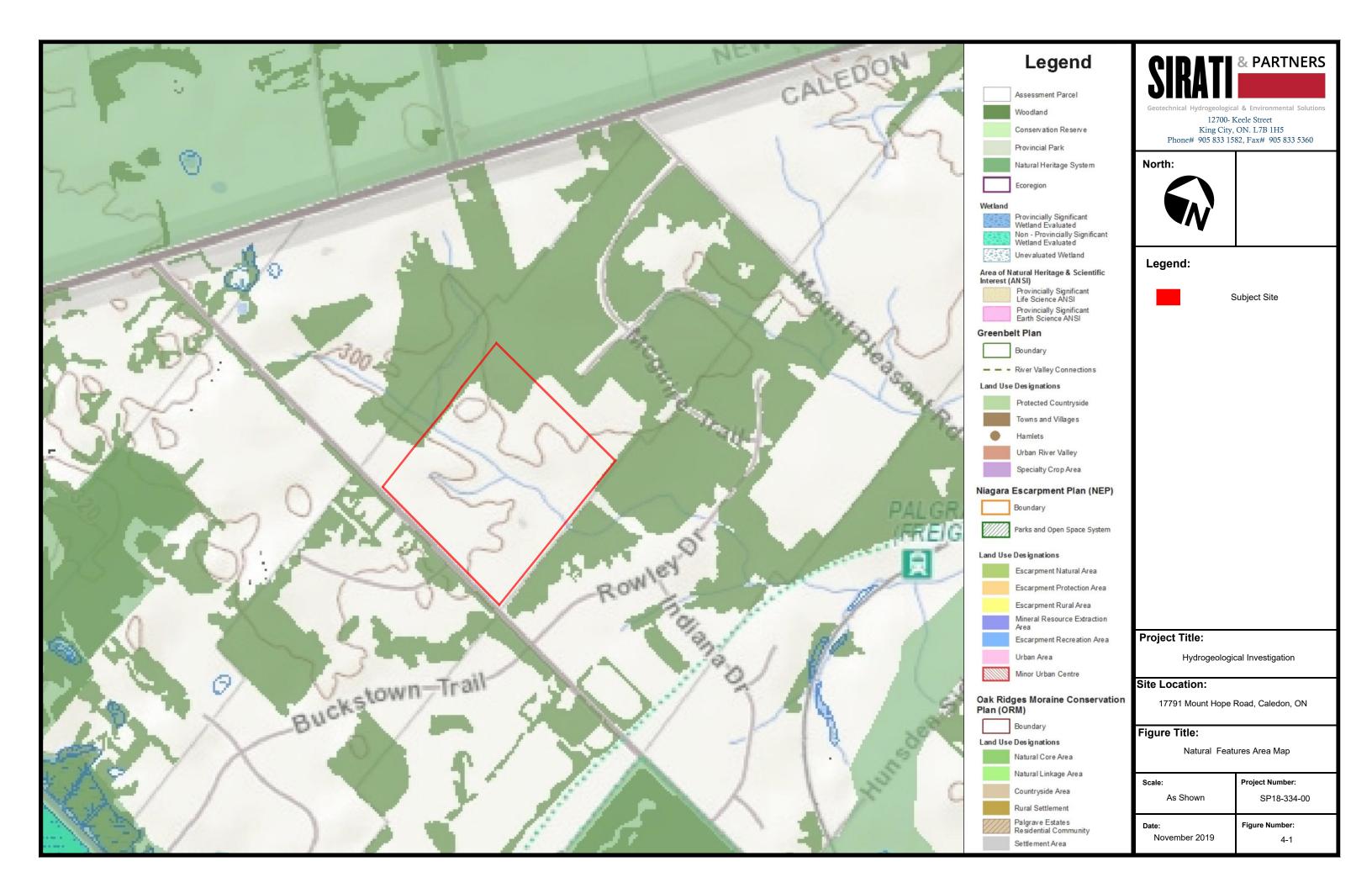
Senior Hydrogeologist/Environmental Specialist

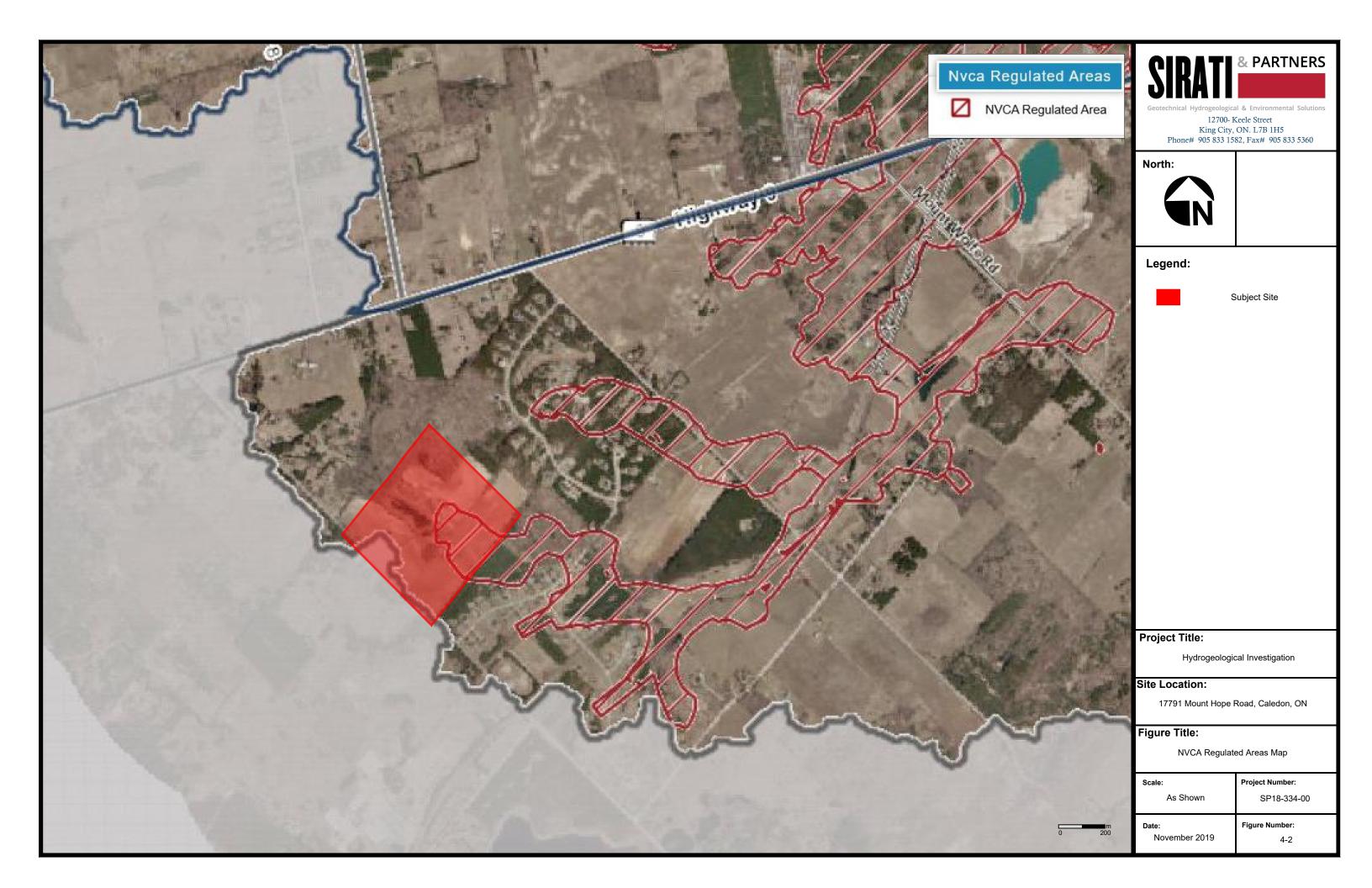


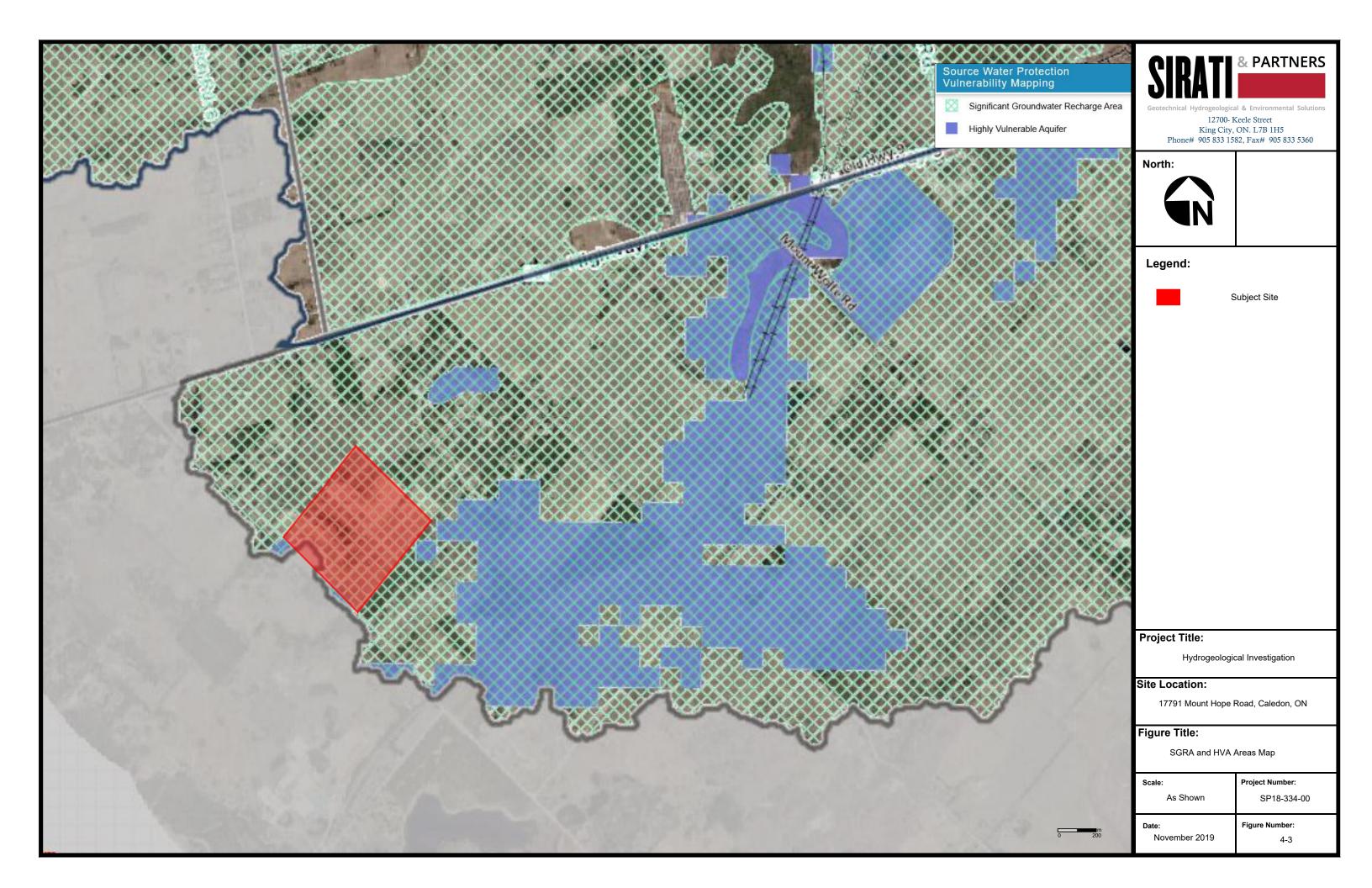


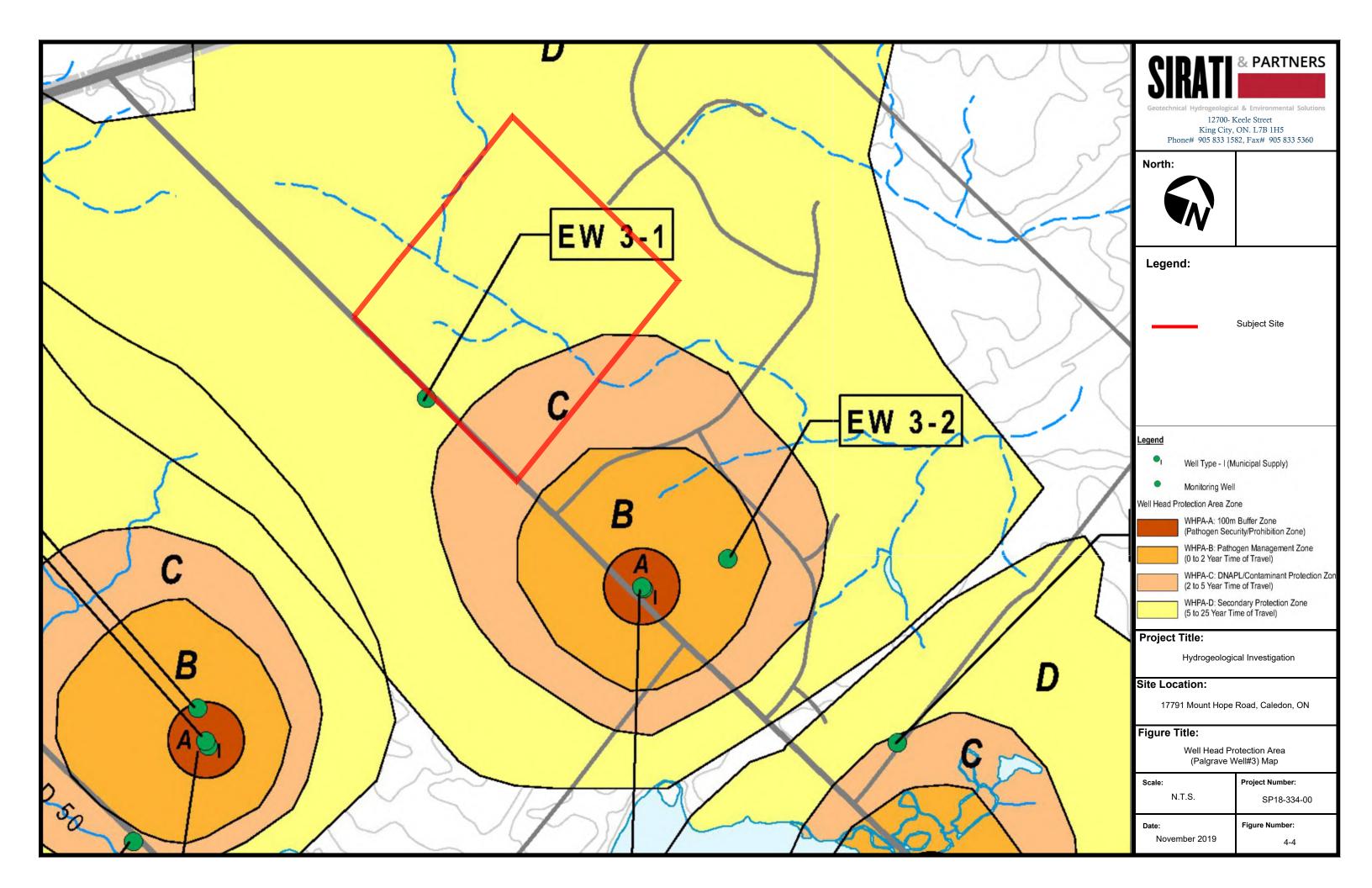


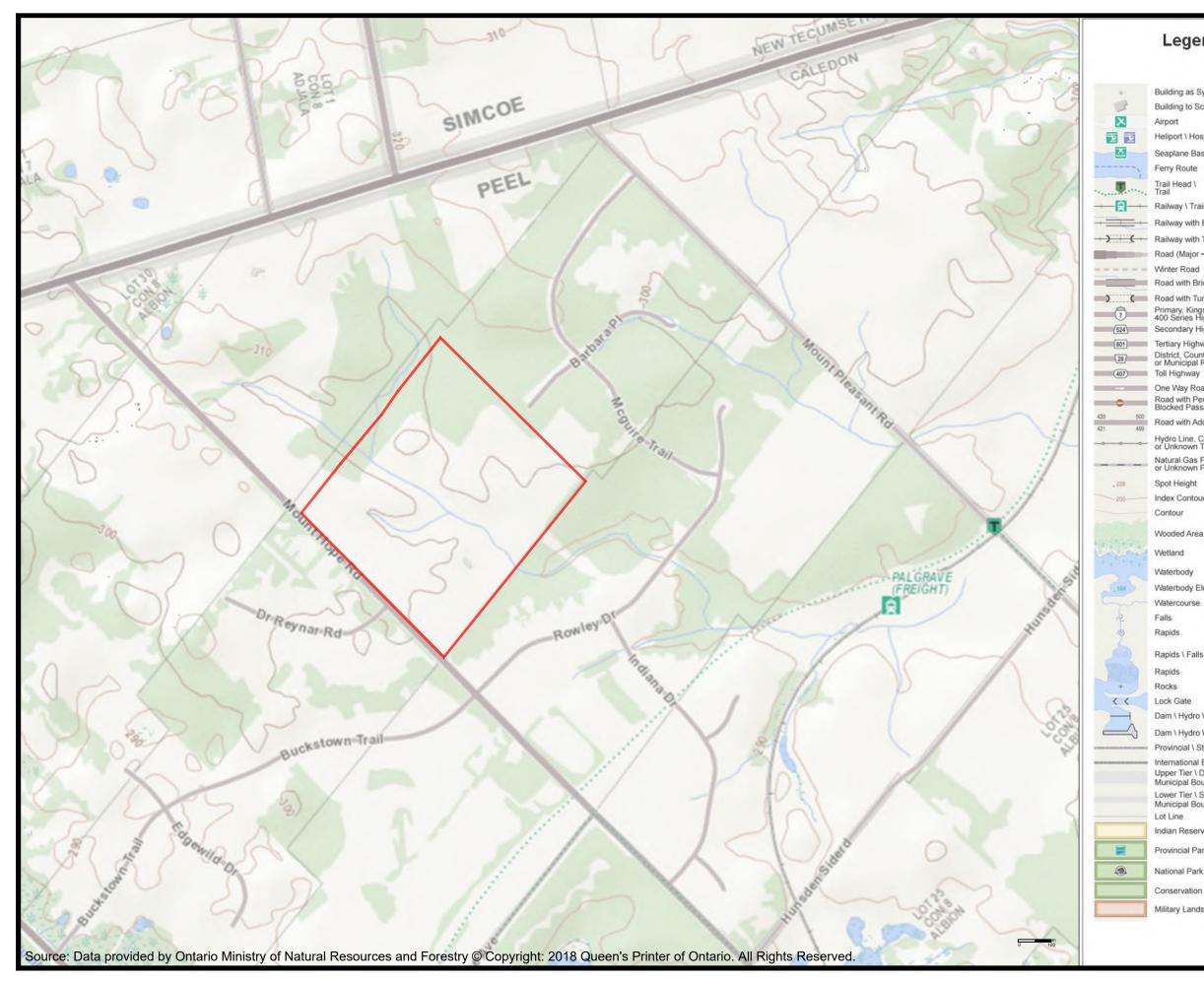












Legend

Building as Symbol Building to Scale Airport Heliport \ Hospital Heliport Seaplane Base Ferry Route Trail Head \ Trail Railway \ Train Station Railway with Bridge Railway with Tunnel Road (Major - Minor) Road with Bridge Road with Tunnel Primary, Kings or 400 Series Highway 524 Secondary Highway 801 Tertiary Highway District, County, Regional or Municipal Road One Way Road Road with Permanent Blocked Passage Road with Address Ranges Hydro Line, Communication Line or Unknown Transmission Line Natural Gas Pipeline, Water Pipeline or Unknown Pipeline Spot Height Index Contour Contour Wooded Area Wetland Waterbody Waterbody Elevation Watercourse Falls Rapids Rapids \ Falls Rapids Rocks Lock Gate Dam \ Hydro Wall Dam \ Hydro Wall Provincial \ State Boundary International Boundary Upper Tier \ District Municipal Boundary Lower Tier \ Single Tier Municipal Boundary Lot Line Indian Reserve Provincial Park National Park Conservation Reserve Military Lands

ARTNERS nical Hydrogeological & Environmental Solution

12700- Keele Street King City, ON. L7B 1H5 Phone# 905 833 1582, Fax# 905 833 5360



Legend:

Property Boundary

Project Title:

Hydrogeological Investigation

Site Location:

17791 Mount Hope Road, Caledon, ON

Figure Title:

Topographic Map

Scale:

Date:

As Shown

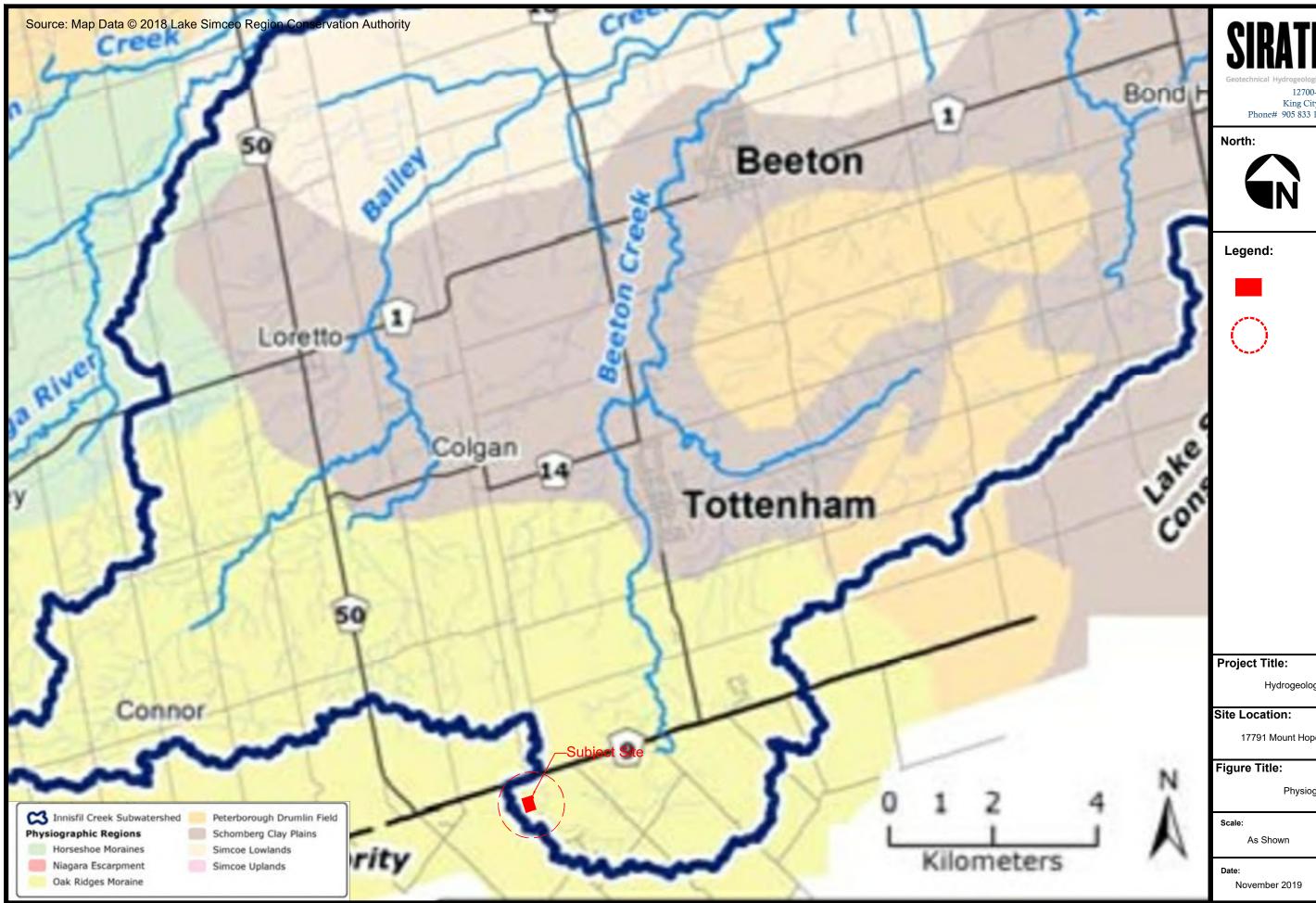
November 2019

Figure Number:

Project Number:

5-1

SP18-334-00



SIRATI & PARTNERS logical & Environmental Solutions

12700- Keele Street King City, ON. L7B 1H5 Phone# 905 833 1582, Fax# 905 833 5360

Subject Site

Approximate Site Location

Hydrogeological Investigation

17791 Mount Hope Road, Caledon, ON

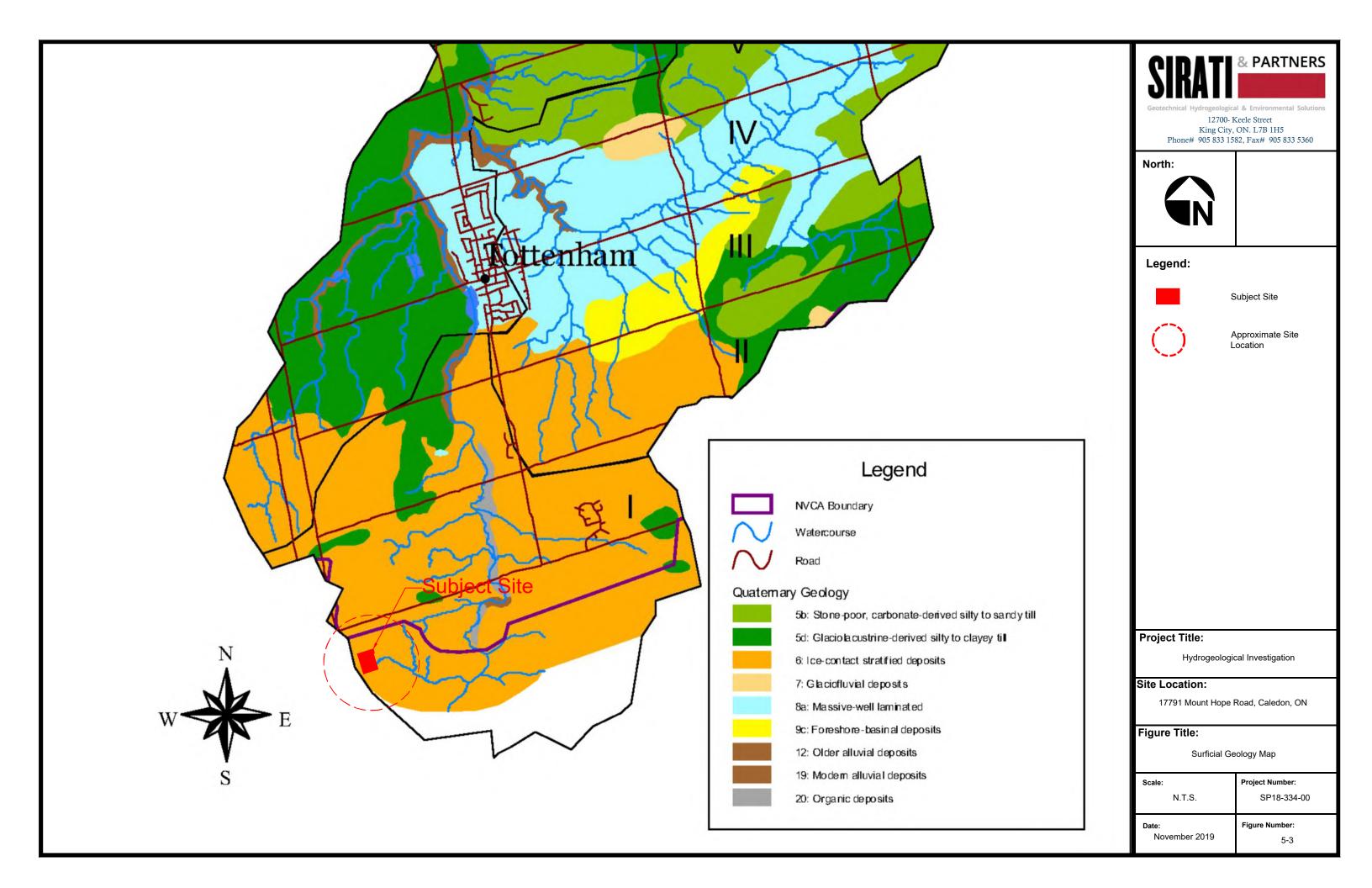
Physiography Map

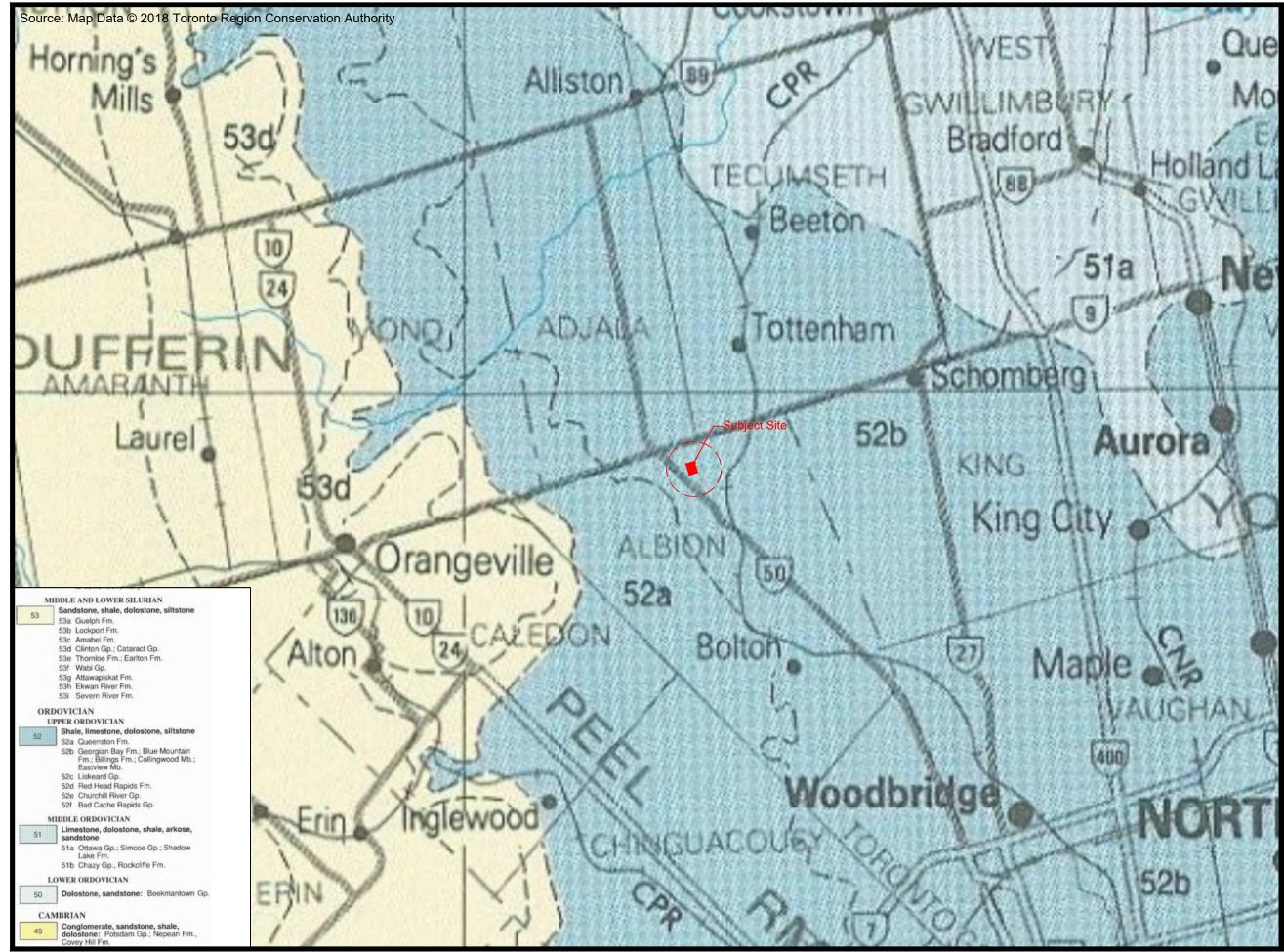
Scale:	
	As Shown

SP18-334-00 Figure Number:

Project Number:

5-2





SIRATI & PARTNERS

Geotechnical Hydrogeological & Environmental Solutions

12700- Keele Street King City, ON. L7B 1H5 Phone# 905 833 1582, Fax# 905 833 5360



Legend:



Subject Site

Approximate Site Location

Project Title:

Hydrogeological Investigation

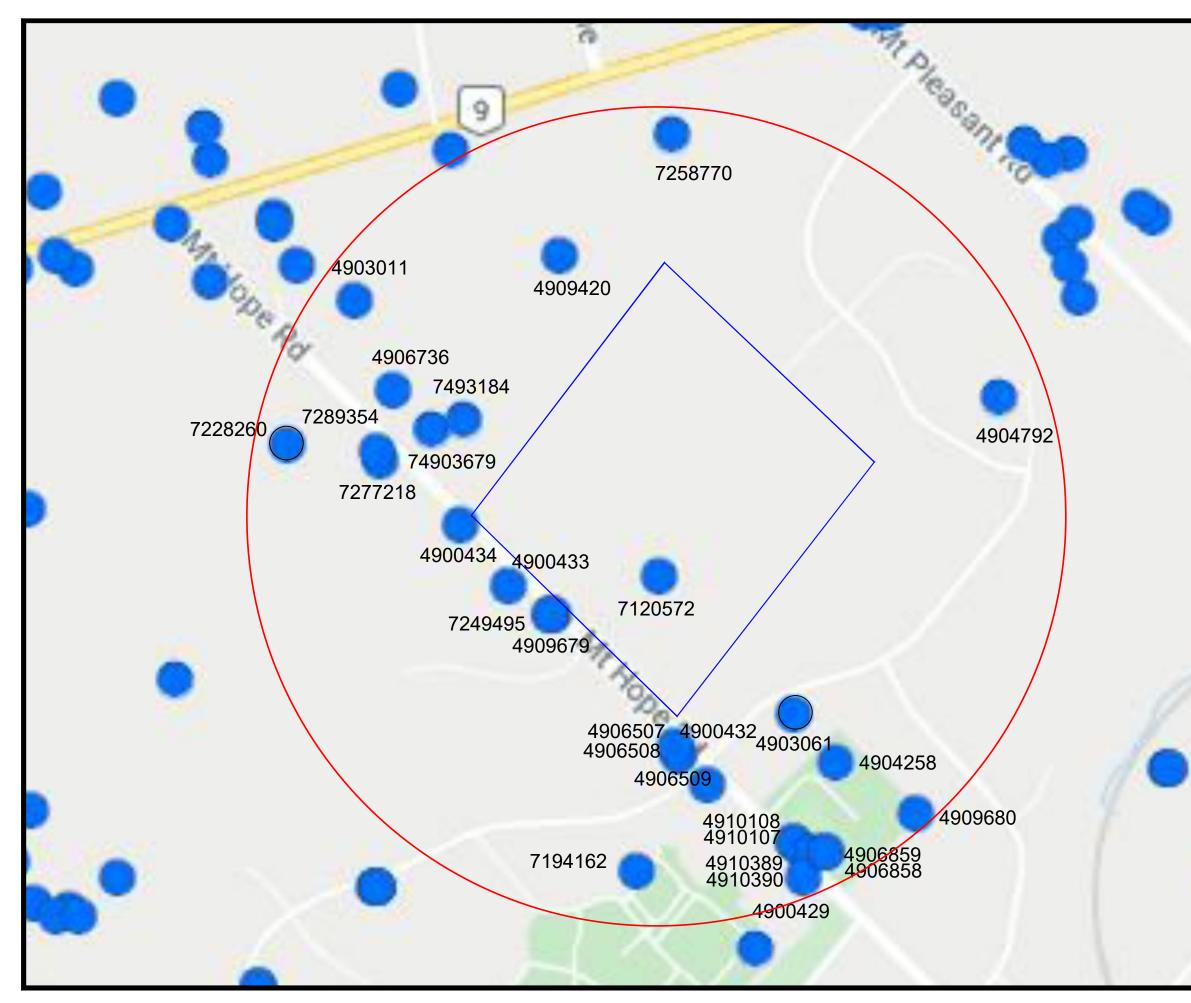
Site Location:

17791 Mount Hope Road, Caledon, ON

Figure Title:

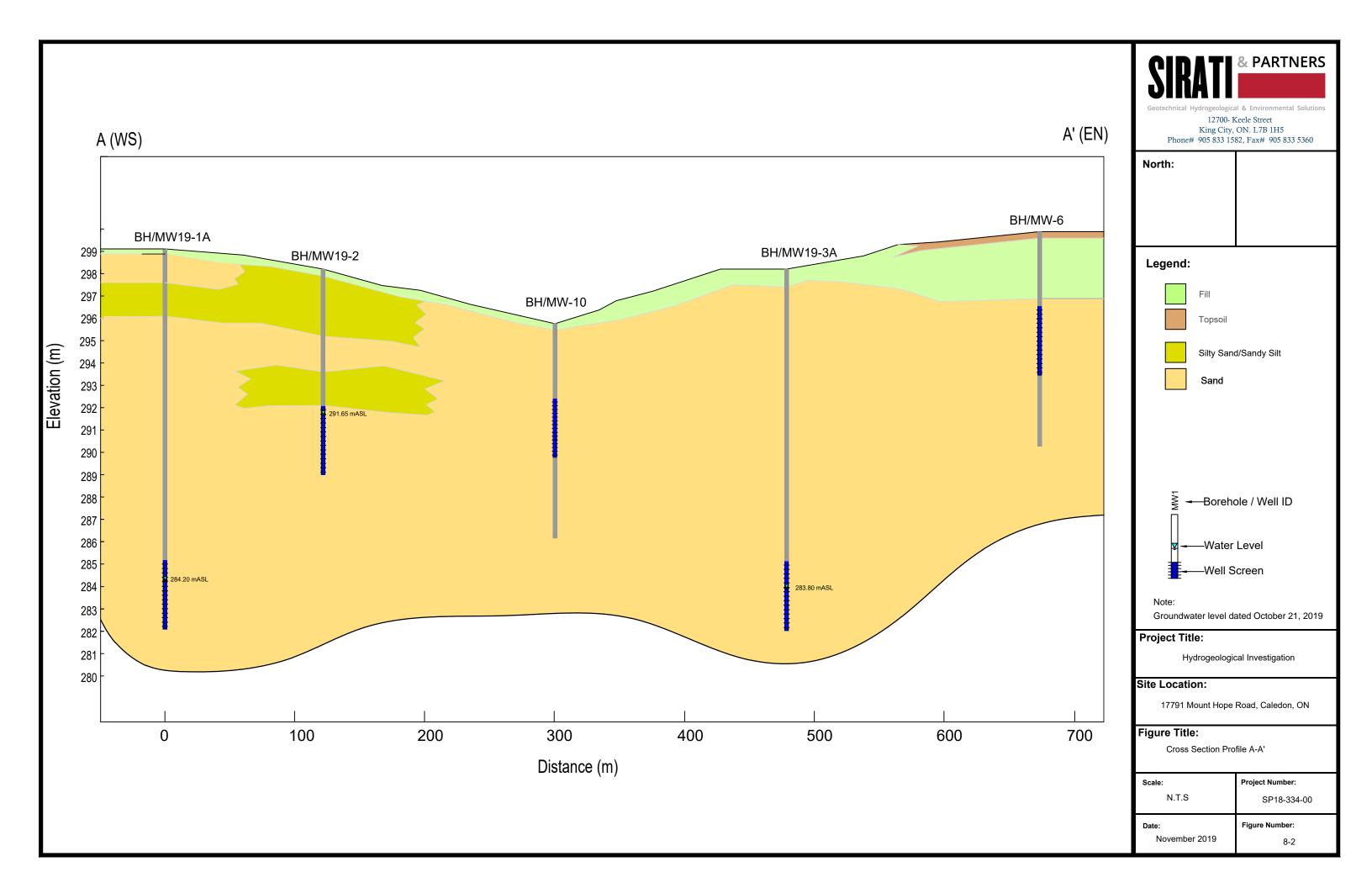
Bedrock Geology Map

Scale:	Project Number:
N.T.S.	SP18-334-00
Date:	Figure Number:
November 2019	5-4



		& PARTNERS
	SIKATI	
	12700- k King City,	Il & Environmental Solutions Ceele Street ON. L7B 1H5
	Phone# 905 833 153	82, Fax# 905 833 5360
	Legend:	
	\$	Subject Site
he		500m Study Area
MI Pleas		MECP Well
.98		
8		
1		
1		
-		
	Project Title:	
-	Hydrogeologic	cal Investigation
	Site Location:	
	17791 Mount Hope	Road, Caledon, ON
	Figure Title:	
	MECP Water W	ell Location Map
1	Scale: As Shown	Project Number: SP18-334-00
0 100	Date: November 2019	Figure Number: 6-1

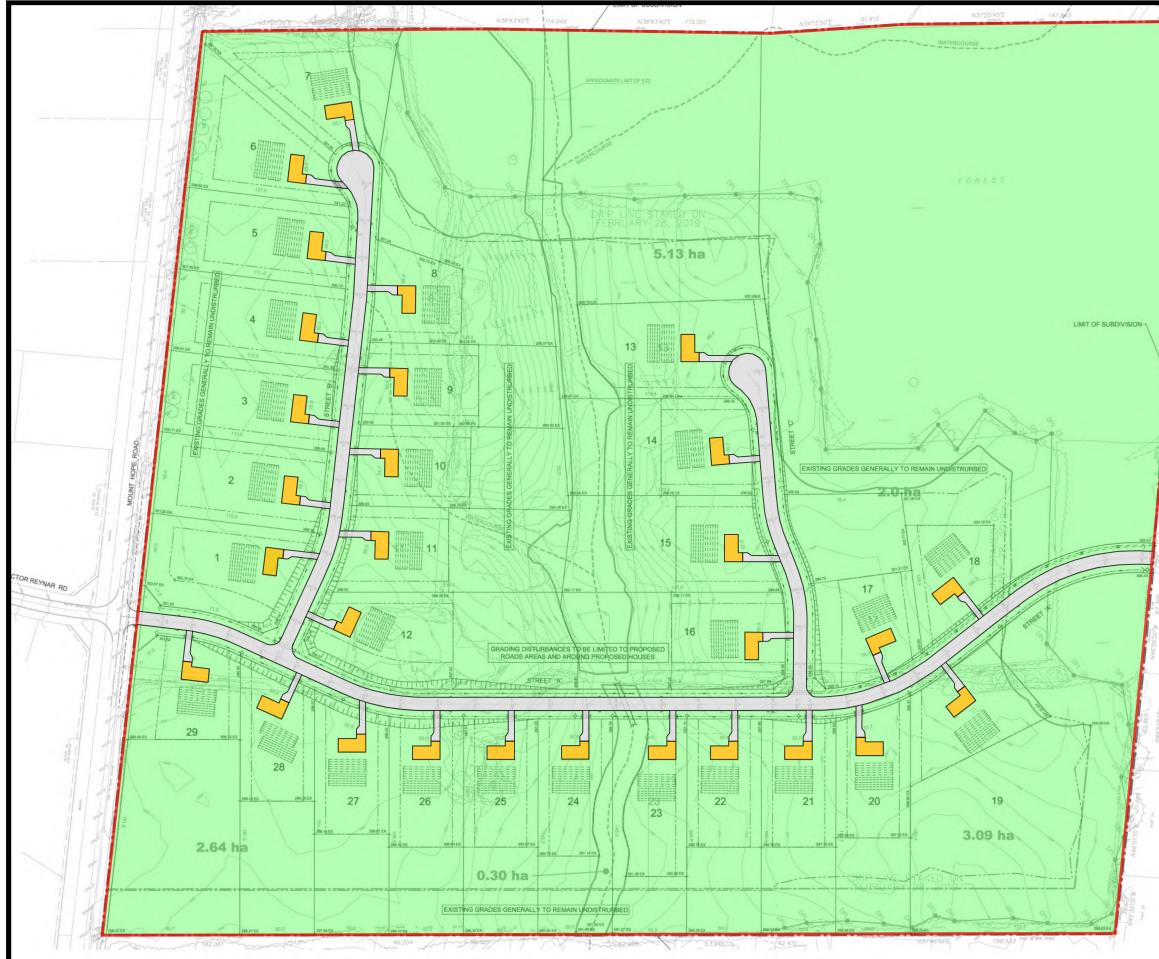




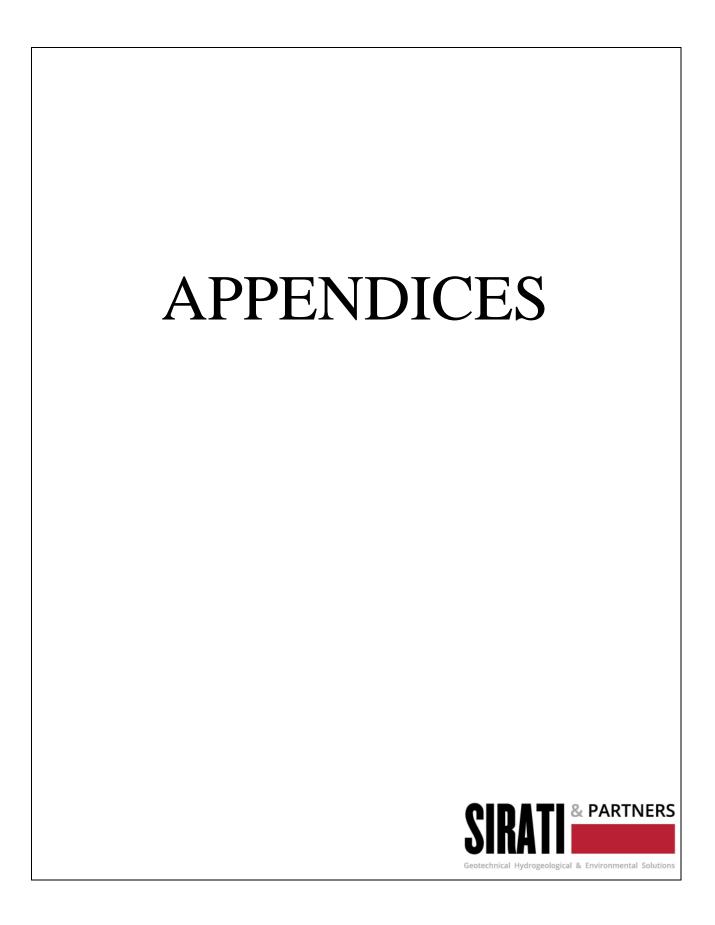


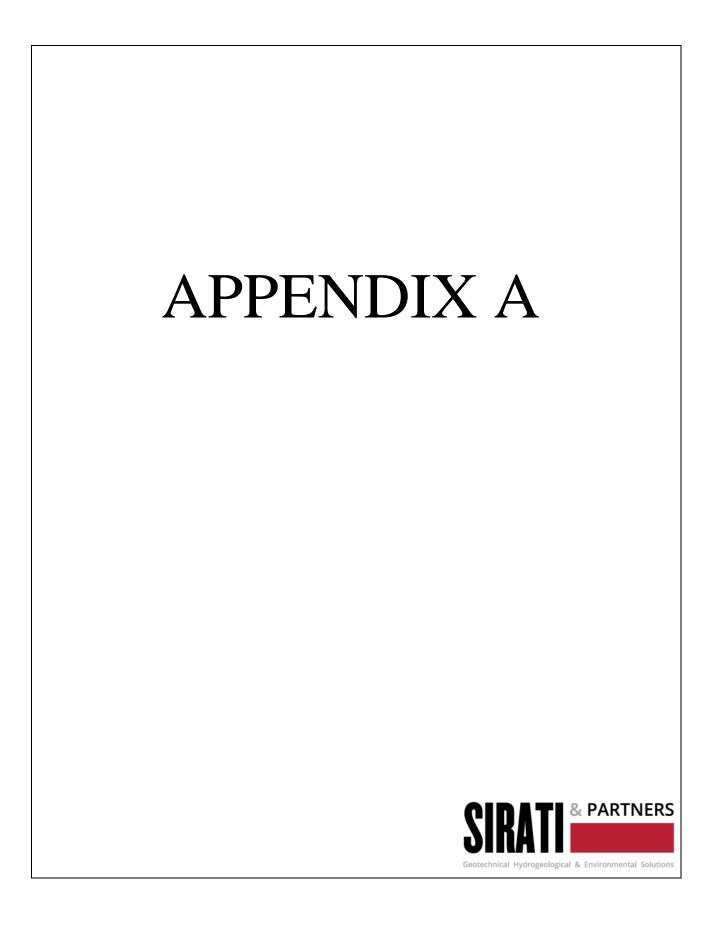
Scale:	Project Number:
As Shown	SP18-334-00
Date:	Figure Number:
November 2019	8-2
	As Shown





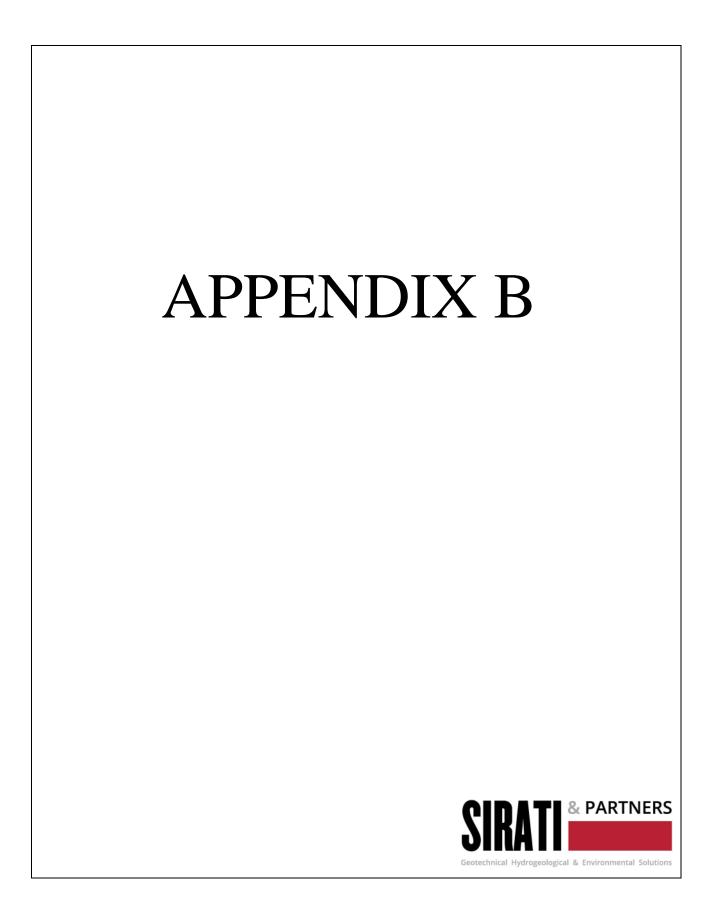
52.544 # ⁹		
25.400 000 Wet 2510 Wet 2510	21KA I I	& PARTNERS
63.169	12700- F King City,	Ceele Street ON. L7B 1H5 82, Fax# 905 833 5360
usaln: sw	North:	
9.377 Sources	Legend:	
2	——————————————————————————————————————	perty Boundary
wotcarw	Pa	/e Area
2	Ro	of
10000 March 10000	Lar	ndscape
FUT-STREET	Site Statistics:	
Anne 19	Pave Area: Roof: Landscape: Total	1.5 ha 0.5 ha <u>39 ha</u> 41 ha
. There	Project Title:	
		cal Investigation
	Site Location:	
سور		Road, Caledon, ON
J.J.	Figure Title: Post-Development I	Plan - Site Statistics
- Source	Scale: As Shown	Project Number: SP18-334-00
jur	Date: November 2019	Figure Number: 11-2







L.	MOUNT HOPE RD
	BUCASTOWN
	BUCKSTOWN PALERAVE AND DRIVE
	HHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHHH
	LEGEND :
	PROPOSED DRIVEWAY WITH CULVERT
	 HYDRANT PROPOSED WATER SERV. CONNECTION
	 ☑ PROPOSED UTILITY BOX ▲ PROPOSED LIGHT POLE ☑ PROPOSED VALVE & BOX
	WATERCOURSE
	EDGE OF VEGETATION
	SEPTIC TANK SYSTEM (TYP)
	PROPOSED HOUSE (TYP) $300.15 \times$ EXISTING ELEVATION
	300.75 × EXISTING ELEVATION 300.20 × PROPOSED ELEVATION
IRE TRAIL	
WCGUIF	
	REVISIONS
	NO. BT DATE REVISION CHECKED APPR'D 1 1
	APPROVED FOR CONSTRUCTION
	THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CERTIFY DIMENSIONAL ACCURACY. THIS APPROVAL IS SUBJECT TO THE FURTHER CERTIFICATION OF THE "AS CONSTRUCTED" WORKS BY A REGISTERED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO.
	DATE: APPROVED BY: H. MUNTZ, P.ENG.
	ELEVATION NOTES ELEVATION SHOWN HEREON ARE GEODETIC AND DERIVED FROM THE TOWN
	OF CALEDON BENCHMARKS. <u>LOCAL BENCHMARK</u> NO. 008197558066 ELEVATION = 277.870 METRES AND NO. 00819778416
	ELEVATION = 318.849 METRES DESIGNED BY APPROVED BY
	W. C. IP
	TOLINCE OF ONTRE
	PROJECT:
	PALGRAVE ESTATES SUBDIVISION
	CONSULTANT: TRANSPORTED CONSULTANT: TRANSPOR
	www.maeng.ca
	Region of Peel Working for you
	TOWN OF CALEDON
	ΠΤLΕ:
	GRADING PLAN
	SCALE: 1:1000 PROJECT No. DESIGNED BY: M.N. DRAWN BY: MAEL CAD
	CHECKED BY: A.I. DATE: OCTOBER 2019



					L	OG (OF B	OR	EHOLE	E BH	1									1 OF 1
PROJ	ECT: Proposed Residential Developme	ent						D	RILLING	DATA										
CLIEN	IT: Palgrave Estate							М	ethod: So	olid Ste	em Aug	gers								
PROJ	ECT LOCATION: Mount Hope Rd., Ca	ledon	, Ont	ario				Di	ameter:	150 mr	n					R	EF. NO).: S	P18-	334-10
	IM: Geodetic							D	ate: Oct/	19/201	8					EI	NCL N	0.: 2	2	
BHLC	DCATION: See Drawing 1							D	rilling Cor	ntracto	r:									
	SOIL PROFILE			SAMPL	ES				Head	Space	e Com	busti	ole		NAT					REMARKS
		Ι.				GROUND WATER			V	/apor	Readi	ing		PLAST LIMIT	IC NAT MOIS CON		LIQUID LIMIT	z.	NATURAL UNIT WT (kN/m ³)	AND
(m) ELEV		101			SE	-AV				(p	pm)			W _P		w	WL	POCKET PEN. (Cu) (kPa)	AL UN	GRAIN SIZE
DEPTH	DESCRIPTION	TAI	BER		BLOWS 0.3 m					\geq							T (0()	DO DO DO DO	E LIR	DISTRIBUTION (%)
2000.0		STRATA PLOT	NUMBER	ТҮРЕ	z	DNO.	ELEVATION		2	4	6	8	10		TER CO		1 (%) 30		ž	GR SA SI CL
298.9	TOPSOIL:350 mm	<u>11/2</u>	-		-			╈	-Ī-	1	Ť	Ť	1		1	1	+			GR SA SI CL
298.6		- Kon	1	SS	5		-	F							0					
0.4	SAND (REWORKED): trace gravel, brown, moist, loose	\mathbb{X}						E												
E	possibly reworked, becoming light	\otimes	}—				~	, t												
-	brown	\otimes	2	SS	5		29	18						0				1		
E 207 4		\mathbb{X}						F												
- 297.4 - 1.5	SAND: light brown, moist, compact	<u> </u>	1					E												
E			3	SS	7		20	,_F						0						
-							29	"[
E	trace gravel		-					F												
-			4	SS	11			E						0						
E							29													
-	becoming brown						28	,0 -												
E			5	SS	15			Ē						0						2 93 1 4
-			_					Ē												
E I							29	5												
-						日日	. 28	, -												
E						日日		Ē												
-						上日	• •	E												
E	some gravel		6	SS	23	日日	29	JE												
-			ľ		20	に目	. ²⁸	⁷⁴						ľ						
E						1 目		F												
-						日日		F												
			·			E	29 29	<u></u>												
⁶ 292.8	GRAVELLY SAND: trace cobbles,	-	-				2	,3 -												
6.1	brown, moist, compact	م. ب م. ()	5 7	SS	19		÷	Ē						0						
-		0	1			日日		ŧ						–						
F.		0				1日	29	2 L												
-		e [8				2	~												
-291.3		Ő						Ē						1						
-291.3	SILT: greyish brown, trace gravel,	- 19-19-					. ·	F						1						
- 7.0 	very moist to wet, very dense		8	SS	74		29	₁Ē						1	•			1		
-			ľ				20	''E												
-								Ē												
								F												
- - - - - - - - - - - - - - - - - - -							29	лĒ												
289.7				- 55	50/		23	Ť							0					
9.2	END OF BOREHOLE:				75			Т												
	Notes:				\mm	/								1						
	1. Borehole was open and dry upon completion of drilling.													1						
	2. Monitoring well was installed in																			
	the borehole upon completion of drilling.																			
	Monitoring well was dry on													1						
	November 1, 2018 and November 28, 2018.													1						
	140Verriber 20, 2010.													1						
														1						
														1						
			1	1	1	L				-			1	1	1	1		1	-	I
<u>GROUN</u>	DWATER ELEVATIONS					GRAPI NOTES	<u><u></u> + <u>s</u> +</u>	3,×	3 . Number to Sens	ers refer sitivity	C	S ^{∎=3%}	Strain	at Failu	re					



					LC	DG O	F BO	REF	IOLE	BH	2									1 OF 1
CLIE PRO DATU	JECT: Proposed Residential Developme NT: Palgrave Estate JECT LOCATION: Mount Hope Rd., Cal JM: Geodetic		, Onta	ario				Meth Diam Date	LING I od: Sol eter: 1 Oct/1	id Ste 50 mn 9/201	י 1 8	ers					EF. NC			334-10
BHL	OCATION: See Drawing 1				F 0	1			ng Cont					1					<u> </u>	
(m) <u>ELEV</u> DEPTH	SOIL PROFILE DESCRIPTION	STRATA PLOT	NUMBER	SAMPL IVBE	"N" BLOWS	GROUND WATER CONDITIONS	ELEVATION		lead S Va	apor I	Readi om)	ng	ole 10	W _P I WA	TER CO	W O ONTEN	LIQUID LIMIT w _L T (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)
299.1 - 0.0 - 298.8	TOPSOIL: 300 mm	<u>x¹ 1₇</u>	2	-	-	00	299				Ĭ	ľ					1			GR SA SI CL IBL in ppm
- <u>298.8</u> - 0.3	FILL: topsoil mixed with silty sand to sand, brown, moist	\bigotimes	1	SS	4		200	-							o					
- - <u>1</u> -	SAND (REWORKED): trace silt, light brown, moist		2	ss	4		298	-						0						
- - - - - -			3	SS	8			-						o						
<u>- 296.8</u> - 2.3	SAND: light brown, moist, compact to dense	×	4	SS	15	-	297	-						0						
- <u>3</u> - - -			5	SS	21		296	-						0				_		
- - - - - - -							295	-										_		
- - - - - - - -	some gravel, trace cobbles, becoming brown		6	SS	21	-	294	-						0				-		78535
- - - - - - - - -	trace gravel					-	293	-										-		
- - - - - - - - -			7	SS	25	-	292	-						0						
2819239471 12012 12018 12019 12018	trace cobbles		8	SS	21	-		-						0						
-10.6PJ SPC							291	-												
	some gravel		9	SS	31	_	290	-						0				-		
1289.4 9.7 9.7 102-W44 71-6 000 M 907	END OF BOREHOLE: Notes: 1. Borehole was open and dry upon completion of drilling.																			
SPCL SC						GRAPH			Number					et Feilu						

					L	ЭG	OF BC	DRE	HOLE	BH	3									1 OF 1
PROJ	ECT: Proposed Residential Developme	ent						DR		DATA										
CLIEN	IT: Palgrave Estate							Me	thod: So	lid Ste	m Au	gers								
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon	, Ont	ario				Dia	meter: 1	50 mn	n					RI	EF. NO).: S	P18-	334-10
DATU	IM: Geodetic							Da	te: Oct/1	8/201	8					Eľ	NCL N	0.: 4		
BH LC	DCATION: See Drawing 1							Dri	lling Con	tracto	r:							_		
	SOIL PROFILE		5	SAMPL	ES				Head S	Space	Corr	nbustil	ole		NAT	URAI			L	REMARKS
(m)						GROUND WATER			V	apor l	Read	ing		PLAST LIMIT	C NAT MOIS CON	TURE	Liquid Limit	- UNIC	NATURAL UNIT WT (kN/m ³)	AND
ELEV		STRATA PLOT			BLOWS 0.3 m	AW 0	ONS ON			(PI _	om)			W _P		N 0	WL	POCKET PEN. (Cu) (kPa)		GRAIN SIZE DISTRIBUTION
DEPTH	DESCRIPTION	ATA	1BER	ш	BLO 0.3	NND	CONDITION		l	\geq				WA	TER CC		т (%)	90 00	NTUR (X	(%)
297.8		STR	NUMBER	ТҮРЕ	ż	GRO	ELE		2	4	6	8	10				30 30		z	GR SA SI CL
0.0	TOPSOIL: 450 mm	<u>x 1/</u>				<u>کا</u>		F												IBL in ppm
297.3		4	1	SS	5			Ē								0				
0.5	FILL: silty sand, trace gravel, dark brown, very moist to moist	\mathbb{X}	}				00	ļ												
1	becomig sandy silt, trace clay,	\bigotimes					29	1										1		
E	brown, moist	\otimes	2	SS	6			Ē							0					
296.3			<u>}</u>					F												
- 1.5	SANDY SILT TO SILTY SAND: trace clay, brown, moist, loose to				-		29	a È						-						
2	compact		3	SS	7		20	Ť						0						
								E												
								Ē												
			4	SS	18		29	5						0						1 61 30 8
- <u>3</u> 294.8 - 3.0	SAND: trace gravel, trace silt, light	<u> </u>	-			≵∶∟	 	Ē												
- 3.0	brown, very moist, compact to very		5	ss	16	ΙE		F							c	>				
E	dense		Ľ					Ē												
						E	294	4[-		
4						ΙE		F												
-						E		F												
-								Ē												
Ē	trace to some gravel, trace cobbles		6	SS	29			3						0						
-			ľ		23			Ē						ľ						
						1:E		F												
Ē						E		E												
6						ĿΕ	29	2										1		
Ē			<u> </u>			F		F												
			7	SS	40			Ē						0						
E						l	© 29	1È												
7			i				8 ²³	'												
							X	Ē												
111							X	F												
2						R	29	oF												
<u>6</u>			8	SS	31	R	5	Ē						0						
			-			R	S a	F												
						R	B	F												
0			i			R	28	9[-		
334-1						R	S a	F												
<u>6 - 288.5</u>			9	SS	50/	þq	8	-				_		0						
9.3 9	END OF BOREHOLE:				<u>mm</u>	1													1	
-201	Notes: 1. Borehole was open and dry upon																			
PPM-2016	completion of drilling.																			
~12	2. Monitoring well was installed in the borehole upon completion of																			
	drilling. 3. Monitoring well was dry on																			
SOIL LOG /W VOC 0~12	3. Monitoring well was dry on November 1, 2018 and																		1	
00	November 28, 2018.																		1	
																			1	
s																			1	
SPC																			1	
				-	-	GRAF			Numbe			8=3%		•		-		•	•	-

					LC	DG O	F BO	REH	OLE	BH	4									1 OF 1
CLIEI PRO. DATU	JECT: Proposed Residential Developme NT: Palgrave Estate JECT LOCATION: Mount Hope Rd., Cale JM: Geodetic DCATION: See Drawing 1		Onta	ario				Metho Diam Date:	LING D od: Soli eter: 18 Oct/18	d Stei 50 mm 8/2018	1 3	ers					EF. NC			334-10
DITE	SOIL PROFILE		5	AMPL	ES							huntik						1		
(m) <u>ELEV</u> DEPTH 298.5	DESCRIPTION	STRATA PLOT	NUMBER	ТҮРЕ	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION		ead S Va 2 4	apor F	Readi om)	ng	01e	W _P	TER CO	W O ONTENT	LIQUID LIMIT WL (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
- 0.0	FILL: sandy silt, trace gravel, trace topsoil, brown, moist	\bigotimes	1	SS	7		298	-							0			_		IBL in ppm
-	POSSIBLE FILL: silty sand, trace cobbles, trace gravel, brown		2	SS	32			-						0						
- 297.0 - 1.5 	SAND: light brown, moist, compact to dense	X	3	SS	27		297	- - - - -						0				-		
			4	SS	32		296	- - - - -						0				-		
3 - - - -	trace gravel		5	SS	17	-	295	- - - - - -						0				-		
- 4 - - - - -						-	294	-										-		
- - - - - - -			6	SS	12	-	293	-						0				-		
- - - - - - - - -	some gravel		7	SS	31	-	292	-						0				-		
/5/18 1 1 1 1 1 1 1 1 1							291	-												
SP18.334-10.GPJ SPCL.GDT 12/5/18			8	SS	40	-		-						0						
	trace gravel					-	290	- - - - -												
	END OF BOREHOLE:		9	SS	26		289	-						0						
SPCL SOIL LOG // VOC 0-12 PPM-2016 8882 2.6 888 2.9 88	Notes: 1. Borehole was open and dry upon completion of drilling.																			
SPCL SOIL						GRAPH			Numbers			8 =3%								

 $\begin{array}{c} \underline{\text{GROUNDWATER ELEVATIONS}} \\ \underline{\text{Measurement}} & \underline{\overset{1st}{\underline{V}}} & \underline{\overset{2nd}{\underline{V}}} & \underline{\overset{3rd}{\underline{V}}} & \underline{\overset{4th}{\underline{V}}} \end{array}$



					LC	og of	F BO	REH	IOLE	BH	5										1 OF 1
PROJ	ECT: Proposed Residential Developme	ent						DRIL	LING D	ΑΤΑ											
CLIEN	IT: Palgrave Estate							Meth	od: Sol	id Ste	m Aug	jers									
PROJ	ECT LOCATION: Mount Hope Rd., Cal	ledon	, Ont	ario				Diam	eter: 1	50 mm	ı					R	EF. NC).: S	P18-	334-10	C
DATU	M: Geodetic							Date:	Oct/1	8/2018	3					E١	NCL N	O.: 6			
BHLC	OCATION: See Drawing 1							Drillir	ng Cont	ractor	:										
	SOIL PROFILE		5	SAMPL	.ES			н	ead S	pace	Com	bustil	ole		ΝΑΤΙ				L	RE	MARKS
(m) <u>ELEV</u> DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	щ	BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION		Va	apor F	Readi om)	ng		W _P		v 5	LIQUID LIMIT W _L T (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	GR/	AND AIN SIZE RIBUTION (%)
294.1			Ŋ	ТҮРЕ	ż	GR			2 4	Ļ (6	8	10	1	0 2	20 3	30		[⁻	GR S	A SI C
0.0 293.8	TOPSOIL: 300 mm	<u>x1 1/</u>			_		294	-													
0.3	FILL: silty sand, yellowish brown,	\boxtimes	1	SS	5										o						
F	moist	\otimes						Ē													
1	sandy silt, trace clay, light brown, very moist to wet	\mathbb{X}			_		000	-													
	very moist to wet		2	SS	5		293	-								0					
- 292.6	CLAYEY SILT some sand, trace	- Hiti						Ē													
-	cobbles, trace gravel, light brown,		3	SS	7										H	-1				2 1	7 54 2
-	very moist, firm		1				292	-													
291.8	SAND: light brown, moist, compact	111				-		-													
E			4	SS	13			F						0							
-			<u> </u>					-													
-							291	-						-							
E			5	SS	11			-						0							
-			1					Ē													
4								-													
E							290	5													
								-													
	trace gravel							Ē													
5			6	SS	16		289	-						0							
			-				203	-													
								-													
E			1																		
-			<u> </u>				288														
E			7	SS	28									0							
E			Ľ		20			-						Ē							
7			-					-													
							287														
- 7.6	SANDY SILT: brown, moist, dense		1					Ē													
3			8	SS	32									0						1 2	8 65 6
2			<u> </u>			-	286	E													
<u>-</u>								-													
5								Ē													
285.0							285	-													
<u>6</u> 9.1	SILT: trace sand, brown, very moist, very dense		9	SS	50		200	ŧ						1	0			1	1		0809
0 □ 284.4	, , .		۹ ۱	33	52			-													0 00 8
286.5 7.6 286.5 7.6 285.0 9.1 285.0 9.1 285.0 9.1 285.0 9.1 285.0 9.1 285.0 9.1 1.5 285.0 9.1 1.5 285.0 9.1 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1	END OF BOREHOLE:		1											1				1	1		
	Notes:																				
	 Borehole was dry and open upon completion of drilling. 		1											1				1	1		
3	prosion of anning.																				
\geq			1											1				1	1		
3			1											1				1	1		
5			1											1				1	1		
ĥ																					
	DWATER ELEVATIONS					<u>GRAPH</u>	, 3	×3.	Number	s refer	~	8 =3%		at Failu							

Gh $\frac{1}{Measurement} \quad \underbrace{\overset{1st}{\underline{\nabla}}}_{\underline{\underline{\nabla}}} \quad \underbrace{\overset{2nd}{\underline{\nabla}}}_{\underline{\underline{\nabla}}} \quad \underbrace{\overset{3rd}{\underline{\nabla}}}_{\underline{\underline{\nabla}}} \quad \underbrace{\overset{4th}{\underline{\nabla}}}_{\underline{\underline{\nabla}}}$

					LC	og ol	F BO	REHO	LE B	sh e	5									1 OF 1
PROJ	ECT: Proposed Residential Developme	nt						DRILLI	NG DA	ГА										
CLIEN	IT: Palgrave Estate							Method	Solid	Stem	n Auge	ers								
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon,	Ont	ario				Diamete	er: 150	mm						RE	EF. NC).: S	P18-:	334-10
DATU	M: Geodetic							Date: C)ct/18/2	2018						E١		0.: 7		
BHLC	OCATION: See Drawing 1							Drilling	Contrac	ctor:										
	SOIL PROFILE		s	SAMPL	ES			Hea	id Spa	ice (Comb	oustib	le		ΝΔΤ				_	REMARKS
(m)		F				GROUND WATER CONDITIONS			Vapo	or R	eadir	ng		PLASTI LIMIT	C NAT MOIS CON	TURE	LIQUID LIMIT	Ľ.	NATURAL UNIT WT (kN/m ³)	AND
(m) ELEV		PLO.			S	o WA	NO		_	(ppr	n)			WP		N 0	WL	POCKET PEN. (Cu) (kPa)	AL UN N/m ³)	GRAIN SIZE DISTRIBUTION
DEPTH	DESCRIPTION	ATA	NUMBER		BLOWS 0.3 m		ELEVATION							\\/\AT			г (%)	DO DO	ATUR (k	(%)
299.9		STRATA PLOT	NUN	ТҮРЕ	ż	GRO CON	ELEV	2	4	6	- 8	10)				30			GR SA SI CL
- 0.0 - 299.6	TOPSOIL: 300 mm	<u>x1 1/</u>		-																IBL in ppm
- 0.3	SAND (REWORKED): trace		1	SS	5										0					
	gravel, trace silt, dark brown, moist	\bigotimes																		
F.	becoming brown	\bigotimes					299													
Ē		\otimes	2	SS	2		200							0						
E		\mathbb{X}						-												
F	becoming light brown	\otimes																		
2		\mathbb{X}	3	SS	0		298	-						0						
297.6		\otimes																		
- 2.3	FILL: silty sand, light brown, moist	\bigotimes		~~																
			4	SS	2										0					
- <u>3296.9</u> - 3.0	SAND: some silt, light brown,						297													
- 3.0	moist, compact to very dense		5	SS	11										0					
								-												
F																				
-							296													
E																				
-																				
E I			6	SS	19		295	-						0						
-			ľ	00			295	-												
E I																				
-							294													
E							-													
E			7	SS	36			-						0						
F						i i i i i i i i i i i i i i i i i i i														
7						R659	293	-												
_E			1																	
81/6/						6659														
						6659														
			8	SS	59	RG3	292							0						
			-			6659														
						R665		Ē												
						R655														
100- 100- 100- 100- 100- 100- 100- 100-	hooming groutely because of the				<u> </u>	R655	291											1		
9F16-334-10.GFJ 9FU	beoming greyish brown, very moist		9	SS	52	R655									o					
<u>e 290.2</u>			Ĺ			6659														
9.7	END OF BOREHOLE:																			
	Notes:																			
SOIL LOG /W VOC U~12 PPM	1. Borehole was dry and open upon completion of drilling.																			
3	Monitoring well was installed in																			
< l	the borehole upon completion of drilling.																			
3	3. Borehole was dry on November 1, 2018 and November 27, 2018.																			
	1, 2010 and NOVETIDE $21, 2010$.																			
n L																				
7																				L
						<u>GRAPH</u>	. 3	×3. Nu	mhore re	fer		8=3%								

 $\begin{array}{c} \underline{\text{GROUNDWATER ELEVATIONS}} \\ \text{Measurement} \quad \underbrace{\stackrel{1 \text{st}}{\underline{\bigvee}} \quad \underbrace{\stackrel{2 \text{nd}}{\underline{\bigvee}} \quad \underbrace{\stackrel{3 \text{rd}}{\underline{\bigvee}} \quad \underbrace{\stackrel{4 \text{th}}{\underline{\bigvee}}} \end{array}$



					LC	og ol	F BO	REH	IOLE	BH	7									1 OF 1
PROJ	ECT: Proposed Residential Developme	ent						DRIL	LING	DATA										
CLIEN	NT: Palgrave Estate							Meth	od: Sol	id Ste	m Aug	jers								
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon,	, Ont	ario				Diam	eter: 1	50 mn	n					RI	EF. NC).: S	P18-	334-10
DATU	IM: Geodetic							Date:	Oct/1	9/201	8					E١		O.: 8		
BH LC	DCATION: See Drawing 1							Drillir	ng Con	tractor	r:								1	
	SOIL PROFILE		s	SAMPL	ES	~		Н	ead S	pace	Com	bustil	ole	DIASTI		URAL	LIQUID		F	REMARKS
(m)		1				GROUND WATER CONDITIONS			Va	apor l (pr	Readi pm)	ng		PLASTI LIMIT			LIMIT	a) EN	NATURAL UNIT WT (KN/m ³)	
ELEV	DESCRIPTION	A PLO	~		BLOWS 0.3 m	NOI-	NOL				_			W _P	(א כ	WL	Э К Е Е	(kN/m	GRAIN SIZE DISTRIBUTION
DEPTH		STRATA PLOT	NUMBER	ТҮРЕ	- Blo		ELEVATION			\geq				WA	TER CC	NTEN	T (%)	0 ⁰ 0	NATU	(%)
298.9			R	Ł	ż	GR CO	ELI		2 4	4	6	8	10	1	0 2	20 3	30			GR SA SI CI
- 0.0 - 298.6	TOPSOIL: 300 mm	<u>×1/</u>	1	SS	5										0					
- 0.3	SAND (REWORKED): trace silt, brown, moist, loose] '	00				-							Č					
298.1		\mathbb{X}																		
- <u>1</u> 0.8	SAND: trace silt, light brown, moist		2	SS	11		298													
E								-												
Ē																				
-			3	SS	16		297							•						
-																				
								-												
-			4	SS	24										0					
3	some silt, wet		<u> </u>				296	-												
-	some sin, wet		5	SS	32									0						
-			·					-												
E							295	-												
-							295	-												
- -								-												
-294.3	SANDY SILT: brown, wet, compact	ΗΠ																		
5			6	SS	30		294									•		-		
-			<u> </u>			-		-												
-								-												
6							293	-												
-				~~	10			-												
-			1	SS	18			Ē								Í				
-							292	-												
-								Ē												
291.3								F												
7.6	SAND: brown, wet to moist,		-					E												
	compact		8	SS	23		291	 			-							1		
								Ē												
								Ē												
0- - -							200													
289.8		L	-				290	-												
<u>8</u> - 9.1	GRAVELLY SAND: brown, moist, very dense	ه () اه ()	9	SS	60			Ē						0						
n_ <u>●</u> 289.2		0	Ľ					-												
9.7 9.7	END OF BOREHOLE:																			
291.3 7.6 289.8 289.8 9.1 289.9 9.7 289.9 9.7 9.7 9.7 9.7 9.7 9.7 9.7	Notes: 1. Borehole was dry and open upon																			
20	 Borehole was dry and open upon completion of drilling. 																			
N N																				
٥ ــــــ ا		1	I	I	I	<u>GRAPH</u>			Number	e rof	1	e -201		at Failur	I	I	I	1	1	I
CROUN												· · · · · · · · · · · //								

					L	DG O	F BO	RE	HOLE	E BH	8									1 OF	- 1
	ECT: Proposed Residential Developme	ent							LLING hod: So			jers									
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon	, Ont	ario				Dia	meter:	150 mr	n					R	EF. NO	D.: S	P18-	334-10	
	M: Geodetic								e: Oct/							E	NCL N	O.: 9			
BHLC	DCATION: See Drawing 1		-						ling Co									-	-	i	
	SOIL PROFILE		5	SAMPL	ES	~			Head	Space	e Com Readi	bustil	ole	PLAST	IC NAT MOIS CON		LIQUID LIMIT		¥	REMARK	S
(m) <u>ELEV</u> DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	Щ	BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION		v	(p	pm)	ng		W _P		<i>N</i> O	WL	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	AND GRAIN SIZ DISTRIBUT (%)	
302.8			₽	ТҮРЕ	ż	R C	Ē		2	4	6	8	10	1	0 2	20	30			GR SA SI	CL
- 0.0	TOPSOIL: 450 mm	<u>× 1,</u> 1, ,	1	SS	6			-							0						
<u>302.3</u> 0.5	FILL: silty sand, trace topsoil, dark brown, moist	\mathbb{X}	<u> </u>			-	302	-													
- - -			2	SS	2		302	-						0							
-			3	SS	2		301	-													
-		\otimes		33	2			-													
- <u>300.5</u> - 2.3	SAND: trace cobbbles, trace		-					-													
-	gravel, light brown, moist, compact		4	SS	16		300	-						0							
- <u>3</u> 299.8 - 3.0	SILTY SAND: light brown, very		-					-													
-	moist, compact		5	SS	14			-							0						
			<u> </u>				299	-	_					-							
-								-													
								-													
-298.2 4.6	SAND: light brown, moist, compact					-	298	-	_									1			
-	to dense		6	SS	12			-						0							
-								-													
-							297	-	_					-							
<u>-</u> 6 -			_					-													
-			7	SS	9			-							o						
			-				296	-	_					-							
-								-													
-								-													
-			8	SS	42		295	-						0							
-				33	42			-						ľ							
-								Ē													
							294	-	_					<u> </u>							
9			-					-													
- - 293.1			9	SS	31			-						0							
9.7	END OF BOREHOLE:		1											1				\square			
- <u>293.1</u> 9.7	Notes: 1. Borehole was dry and open upon completion of drilling.																				
		-	-	1		GRAPH	·	<u></u>	Numbe		1	8=3%	1		I	I	_			1	

		DG O	F BO	REHC	LE E	BH 9									1 OF 1				
PROJ	ECT: Proposed Residential Developme	nt						DRILLI	NG DA	ТА									
CLIEN	T: Palgrave Estate							Method	: Solid	Stem A	Augers								
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon,	Onta	ario				Diamet	er: 150	mm					RE	EF. NC).: SI	P18-3	334-10
DATU	M: Geodetic							Date: 0	Oct/19/2	2018					EN		O.: 10	0	
BH LC	CATION: See Drawing 1							Drilling	Contra	ctor:									
	SOIL PROFILE		s	AMPL	ES	~		Hea	ad Spa	ace_Co	ombu	stible	DIAST	NAT	URAL	חוווסעו		F	REMARKS
(m)		5				GROUND WATER CONDITIONS			Vap	or Rea (ppm			LIMIT	IC NAT MOIS CON		LIQUID LIMIT	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	
ELEV	DESCRIPTION	A PLO	۲		BLOWS 0.3 m	NOI:	NOI				/		W _P		N 0	WL	i) (k	RAL L (kN/m	GRAIN SIZE
DEPTH		STRATA PLOT	NUMBER	ТҮРЕ		NNO	ELEVATION				X		WA	TER CO		Г (%)	9 Q Q	NATU	(%)
298.2			N	Τ	ž	88		2	4	6	8	10		10 2	20 3	30			GR SA SI CL
- 0.0 - 297.9	TOPSOIL: 300 mm	<u>x' /y</u>	1	SS	6	X	298	-					_						
- 0.3	FILL: sandy silt to silty sand, trace topsoil, dark brown, wet	\mathbb{X}	'	00															
-								-											
- <u>1</u>			2	SS	3										0				
		\otimes					297												
-		\bigotimes																	
-		\otimes	3	SS	4			-							0				
- 295.9		\otimes					296	-					_						
- 2.3	SAND: light brown, moist, compact																		
-			4	SS	15								0						
3								-											
-			5	SS	29		295	-					0						
-																			
-																			
-								-											
							294	-											
-293.6	GRAVELLY SAND : trace cobbles,	0.0																	
5	light brown, moist, dense	00	6	SS	35								0						
-		0					293	-					_						
-		° 0																	
Ē		0				ŀ∃:		-											
<u>6</u>		. O																	
-		20	7	~~~	25		292												
-		0.0	7	SS	25			-						Ť					
Ē		o.O					291	-											
		• ()																	
290.6	SAND: trace silt, light brown, moist,	0						-											
0.0	dense		8	SS	41								0						
						L L L L L L L L L L L L L L L L L L L	290	-					-						
334-10.GPJ								-											
- 2334-																			
	trace gravel		9	SS	43		289	-					0						
ກ_ ຼ288.5			0	00				-											
9.7	END OF BOREHOLE:						1												
	Notes: 1. Borehole was dry and open upon																		
0~1	1. Borehole was dry and open upon completion of drilling.																		
	2. Monitoring well was installed in the borehole upon completion of						1												
N N	drillina.																		
Ĭ	3. Monitoring well was dry on November 1, 2018, and November																		
SOIL LOG /W VOC 0~12	27, 2018.																		
shor																			
n 🗌 🗌		1		1		I	I						_	1		1	L		



					LO	g of	BOR	REH	OLE	BH 1	0B										1 0	ι F 1
PROJ	ECT: Proposed Residential Developme	ent						DR	LLING	DATA												
CLIEN	IT: Palgrave Estate							Met	hod: So	olid Ste	em Aug	gers										
PROJ	ECT LOCATION: Mount Hope Rd., Cal	edon	, Ont	ario				Dia	neter:	150 mr	n					R	EF. NC).: S	P18-	334-1	0	
DATU	IM: Geodetic							Dat	e: Oct/	19/201	8					E١	NCL N	0.: 1	1			
BHLC	OCATION: See Drawing 1							Dril	ing Co	ntracto	r:									•		
	SOIL PROFILE		5	SAMPL	ES	~			Head	Space	Com	busti	ble	DIAST	NAT	URAL			F	RI	EMAR	KS
(m)		-OT			<u>ଛ</u> -	GROUND WATER CONDITIONS	7		V	apor/ apor/	Readi pm)	ing		LIMIT WP	IC NAT MOIS CON	TURE TENT W	LIQUID LIMIT W _L	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	GF	AND RAIN SI	
ELEV DEPTH	DESCRIPTION	STRATA PLOT	Щ		BLOWS 0.3 m		ELEVATION							-		0		OCKE (DU)	(KN/	DIST		TION
		TRA-	NUMBER	ТҮРЕ	"Z	ROU OND	LEV		2	4	×	8	10		TER CO		. ,	_	Ā		(%)	
295.7	TOPSOIL: 300 MM	0	z	-	F		_	-		4		-	10		10 2	20 :	30		-	GR :	SA SI	
- 0.0 - 295.4 - 0.3	FILL: topsoil mixed with silty sand,	\boxtimes	1	SS	6	CANCINE -									0							
	reddish brown, very moist, loose SAND (REWORKED): brown, very	\otimes					295	; 	-													
-	moist, very loose		2	SS	0			-							0							
-						IN I	294															
- - -		\otimes	3	SS	0										¢							
- 293.4	SAND: brown, moist to very moist,							Ē														
E	very loose to very dense		4	SS	4		293	Ē						0								
3			_					Ē														
	trace cobbles		5	SS	5			Ē						•								
E			Ľ					Ē						ľ								
E							292	E										1				
-			1					Ē														
								Ē														
	trace gravel		-				291	-						_								
- - 5	·		6	SS	9			Ē						0								
			<u> </u>			目		F														
								Ē														
E			1				290	Ē														
<u>6</u>			_			H		Ē														
E			7	SS	18			Ē						0								
E							289	È										-				
- 7								Ē														
- I								Ē														
12/5/	some grouply trace day light brown		8	SS	50/		288	-												3	88 2	, ₇
	some gravel, trace clay, light brown				125		200	Έ													<i>J</i> O 2	
			1		\ <u>mm</u>			F														
<u>አ</u> ር								Ē														
0.0							287	' F														
334-1								Ē														
8	some silt (pockets)		9	SS	38			Ē														
SPCL SOIL LOG M VOC 0-12 PPM-2016 5 PPM-334-10.GPJ SPCL.GUT 12/5/18 6 87 1 9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				00	50		286	-						Ű								
9.7 9.7	END OF BOREHOLE:																					
12	Notes: 1. Borehole was dry and open upon																					
è	completion of drilling. 2. Monitoring well was installed in																					
	the borehole upon completion of																					
< 00	drilling. 3. Borehole was dry on November																					
	1, 2018 and November 27, 2018.																					
														1					1			
Ъ.																						
						RAPH	•	~	Numbe			8=3%	,									



O ^{8=3%} Strain at Failure

CLIEN	ECT: Proposed Residential Developme IT: Palgrave Estate ECT LOCATION: Mount Hope Rd., Ca		Ont	ario				Metho	od: So	DATA ild Ste 50 mr	em Au	ger				P	EE NC) · · ·	P10	334-00
DATU	M: Geodetic DCATION:	leuon	, Uni	ano				Date:									NCL N			334-00
	SOIL PROFILE		5	Sampl	ES	ER		DYNA RESIS					100	PLAST LIMIT			LIQUID LIMIT	ż	T WT	REMARKS AND
(m) <u>ELEV</u> DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	ТҮРЕ	"N" <u>BLOWS</u> 0.3 m	GROUND WATER CONDITIONS	ELEVATION	SHE/ ୦ UI ● QI	AR ST NCONF JICK T			KPa) FIELD & Sens LAB	VANE sitivity /ANE	WA	TER C	W O ONTEN		POCKET PI (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	
299.1 29 8.9	FILL: silty sand, trace rootlets,	s.	Ī	-	£	υõ	団 299	2	20 4	40	60	80	100		0 :	20	30	-	-	GR SA SI
0.2	brown, moist // SAND: brown, moist, loose	1	1	SS	7			-												
	,		i					-												
L	oxidated		2	SS	5		298	-												
							298	-												
297.6 1.5	SILTY SAND: brown, oxidated,	11						-												
2	moist, compact		3	SS	10		297	-												
296.8							297	-												
2.3	SANDY SILT: trace clay, brown, very moist, compact		4	SS	12			-												
296.1								-												
3.0	SAND: brown, moist, compact		5	SS	27		296	-												
								-												
							295	-												
								-												
	becoming grey and very moist							-												
			6	SS	26		294													
								-												
								-												
								-												
	becoming very dense		-				293	-						-						
	5 7		7	SS	63															
								-												
<u>.</u>							292	-												
2 3 290.0							292	-												
								-												
			8	SS	52															
							291							-						
								-												
								-												
290.0 9.1	SAND AND GRAVEL: trace	0.0	-				290	-										1		
	cobbles, brown, moist, very dense	0.0	9	SS	98															
9.1		。 0 0						ł												
	Continued Next Page	þ	1			GRAPH		× ³ :						ı at Failur	<u> </u>					

					LO	g of	BOR	EHO	OLE E	3H 19	9-1									2	OF 3
PROJ	ECT: Proposed Residential Developme	ent						DRIL	LING I	DATA											
	NT: Palgrave Estate							Meth	od: Soi	ld Ster	n Aug	er									
	IECT LOCATION: Mount Hope Rd., Cal	ledon	, Ont	ario					neter: 1		-					RI	EF. NC).: S	P19-	334-00	
	IM: Geodetic							Date	: Aug/2	23/201	9						NCL N				
BHLC	DCATION:								Ū												
	SOIL PROFILE		5	SAMPL	ES			DYN/	MIC CC		IETRAT	FION								DEM	
		Ι.				GROUND WATER CONDITIONS				0 6	<u> </u>	0 1	00	PLASTI LIMIT	C MOIS	URAL STURE TENT	LIQUID LIMIT	z	NATURAL UNIT WT (kN/m ³)	REM/	
(m)		STRATA PLOT			SIE	NS NS	z		AR ST	i i		I	1	W _P		N	WL	POCKET PEN. (Cu) (kPa)	VL UN	GRAIN	
ELEV DEPTH	DESCRIPTION	TAF	BER		BLOWS 0.3 m	UND DI DI	ATIC	οι	INCONF	INED	÷	FIÉLD V & Sensit	ivity			0		δ CCΓ Δ	TUR/	DISTRIE (%	
		STR/	NUMBER	ТҮРЕ	ż	SR0	ELEVATION		UICK TI 20 4	RIAXIAL 0 6		LAB VA	ANE 00		TER CC 0 2		1 (%) 30		Ž	GR SA	
-	SAND AND GRAVEL: trace	6. V			-		289		-				-			-	-	-			
-	cobbles, brown, moist, very dense(Continued)	0.0	Ś					E													
-	dense(Continued)	0						E													
288.4	SAND: trace gravel, some cobbles,	- 				┥╘		E .													
11	brown, very moist, very dense		10	SS	50/	目		È.													
					mm].≣.	288	-													
-								F													
-						目		F													
-								E													
12							287														
286.9	SAND AND GRAVEL: some					1:目:	201	È.													
_ 12.2	cobbles, brown, very moist, dense	0	11	SS	49			È													
-		0	<u> </u>			_:目:		F													
-		0.0						F													
-		0	1				W. L.														
-		о. О					Oct 08	3, 201 I	9												
-		0.0	Ś					F													
-		0				:• = :•		È.													
14		0.0	Ś					ŀ													
-		0	1				285	-													
-		0						F													
-								F													
-		, O						E													
15		0 C	Š				284														
283.9 5 15.2	SAND: grey, wet, dense	0	1				204														
- 10.2	GAND: grey, wet, dense		12	SS	36			È.													
- 283.4	──Ęnd of SPT sampling	<u> </u>	1					F													
15.7	Augered from 15.66 m to 18.3 m							È													
<u>16</u> -	below ground surface						283														
-								E													
-								E													
-								E													
6 17								F													
1221							282	: <u> </u>													
12								È.													
8								È.													
Lo Lo								F													
0 18							201	F													
ບ້- ອີ 280.8							281	-													
₹ <u>200.0</u> ₹ 18.3	END OF BOREHOLE:		1					1										1	1		
SPCL SOIL LOG SP18-334-10 - 2019.GPJ SPCLGDT 10/22/19 81 82 88 89 88 89 89 80 80 80 80 80 80 80 80 80 80 80 80 80	Note:		1			1	1	1						1							
-334	1. Borehole caved at 11.58 mbgs.		1			1	1	1						1							
SP18	2. Water was encountered at 15.2 mbgs upon completion of drilling.		1			1	1	1						1							
у́9	Monitoring well was installed		1			1	1	1						1							
L LC	upon completion of drilling. 4. Groundwater level was observed																				
sol	at 12.96 mbgs on August 30, 2019.		1			1	1	1						1							
PC	5. Groundwater level was observed at 13 mbgs on September 5, 2019.		1			1	1	1						1							
ري ا	Continued Next Page		L	I	I		<u> </u>	<u>_</u>	<u> </u>				L	1		L		1	I		
GROUN	IDWATER ELEVATIONS 1st 2nd 3rd 4th					<u>GRAPH</u> <u>NOTES</u>	+ 3	,×3:	Number to Sens	s reter tivity	0	s =3%	Strain	at Failur	e						

 $\begin{array}{c} 1 \\ \text{Measurement} & \underline{\underline{}}^{1\text{st}} & \underline{\underline{}}^{2\text{nd}} & \underline{\underline{}}^{3\text{rd}} \\ \underline{\underline{}}^{2\text{nd}} & \underline{\underline{}}^{2\text{rd}} & \underline{\underline{}}^{4\text{th}} \end{array}$



PROJECT: Proposed Residential Development

CLIENT: Palgrave Estate

PROJECT LOCATION: Mount Hope Rd., Caledon, Ontario

DATUM: Geodetic

BH LC	DCATION:		-									TION							i —	
	SOIL PROFILE		s	SAMPL	ES	œ.		RESIS		NE PEI PLOT		IUN		PLAST		URAL	LIQUID		۲	REMARKS
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	ТҮРЕ	v" <u>BLOWS</u> 0.3 m	GROUND WATER CONDITIONS	ELEVATION	SHE/ 0 U 0 Q	AR ST NCONF UICK TI	I RENG INED RIAXIAL	iTH (kl + - ×	BO 1 Pa) FIELD V & Sensit LAB V	ANE ivity ANE		TER CO			POCKET PEN. (Cu) (kPa)	NATURAL UNIT V (kN/m ³)	AND GRAIN SIZE DISTRIBUTION (%)
	6. Groundwater level was observed at 12.98 mbgs on October 8, 2019.	STF				CO														<u>GR SA SI CL</u>
						GRAPH														

REF. NO.: SP19-334-00

ENCL NO.: 2

DRILLING DATA

Date: Aug/23/2019

Method: Soild Stem Auger Diameter: 150 mm

PRACECT LOCATION: Mount Hope Rd., Caledon, Ontario Demeter: 200 mm REE: NO: SPI DATUME Goodies Demeter: Coll (2000) DEMELS: SPI SOIL PROFILE SAMPLES Demeter: Coll (2000) REE: NO: SPI (m) DESCRIPTION Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE Bit BASE (m) DESCRIPTION Bit BASE Bit BASE Bit BASE <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>iger</th> <th>em Au</th> <th>low St</th> <th>od: Hol</th> <th>Metho</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>IENT: Palgrave Estate</th> <th>CLIEN</th>								iger	em Au	low St	od: Hol	Metho							IENT: Palgrave Estate	CLIEN
BH LOCATION: SOLL PROFILE SAMPLES UPMANC CONFERENCE NOT UPMANC CONFERENCE NOT UPMANC CONFERENCE NOT UPMANC CONFERENCE NOT UPMANC PROFILE UPMANC PROFILE <t< td=""><td>9-334-00</td><td>P19-:</td><td>.: SF</td><td>F. NO</td><td>RE</td><td></td><td></td><td></td><td></td><td>00 mm</td><td>eter: 20</td><td>Diam</td><td></td><td></td><td>ario</td><td>Onta</td><td>don,</td><td>, Caled</td><td>ROJECT LOCATION: Mount Hope Rd., Ca</td><td>PROJ</td></t<>	9-334-00	P19-:	.: SF	F. NO	RE					00 mm	eter: 20	Diam			ario	Onta	don,	, Caled	ROJECT LOCATION: Mount Hope Rd., Ca	PROJ
SOIL PROFILE SAMPLES (m) ELCY 299.1 DESCRIPTION Image: Solution of the second s).: 3	CL NC	EN)	0/2019	Oct/1	Date:							ATUM: Geodetic	DATU
(m) BEEV DEPTH DESCRIPTION (m) (m) (m) (m) (m) (m) (m) (m) (m) (m)		—																		BH LC
Um Um<	REMARKS	₽			JRAL		PI ASTI	ION		PLOT	TANCE	RESIS		S	AMPL	s			SOIL PROFILE	
298.0 FLL: sity sand, trace rootlets, x 0.2 gown, moist SAND: brown, moist, loose oxidated	AND	JNIT V	PEN.	LIMIT	ΓENT	CON	LINNI		1				Ś				ЪТ))	(m)
298.0 FLL: sity sand, trace rootlets, x 0.2 gown, moist SAND: brown, moist, loose oxidated		KAL L (kN/m)	u) (kF	WL	v >	۷ (₩ _P	Pa)	TH (kP	RENG	AR STR	SHEA	NOI NOI	3 m		۲	V PLO			ELEV
298.0 FLL: sity sand, trace rootlets, x 0.2 gown, moist SAND: brown, moist, loose oxidated	(%)	NATU	90 00	(%)	NTENT	FER CC	WA	& Sensitiv	+ ;		ICK TE			BL0	щ	MBEI	RATA		TH DESCRIPTION	DEPTH
298.0 FLL: sity sand, trace rootlets, x 0.2 gown, moist SAND: brown, moist, loose oxidated	GR SA SI CL	_											ELE CO	ž	ΤL	NUN	STF		9.1	299.1
SAND: brown, moist, loose 0xidated 297.6 1.5 SILTY SAND: brown, oxidated, moist, compact 2.3 SANDY SILT: trace clay, brown, very moist, compact 3.0 SAND: brown, moist, compact 3.0 SAND: brown, moist, compact 296.1												-	29				\propto		9.9 FILL: silty sand, trace rootlets,	- 29 8.9
oxidated 297.6 1.5 SLTY SAND: brown, oxidated, molst, compact 298.8 SANDY SILT: trace clay, brown, very molst, compact 296.8 SAND: brown, molst, compact 296.1 296 3.0 SAND: brown, molst, compact 4 296 becoming grey and very molst a becoming very dense												-						 / :	SAND: brown, moist, loose	[0.2
297.6 1.5 SILTY SAND: brown, oxidated, moist, compact 296.8 296.1 3.0 SANDY SILT: trace clay, brown, very moist, compact 296.1 3.0 SAND: brown, moist, compact 4 becoming grey and very moist becoming very dense												-							- , , ,	-
297.6 1.5 SILTY SAND: brown, oxidated, moist, compact 296.8 296.1 3.0 SANDY SILT: trace clay, brown, very moist, compact 296.1 3.0 SAND: brown, moist, compact 4 becoming grey and very moist becoming very dense												-						÷		
297.6 Image: Subtry SAND: brown, oxidated, moist, compact 2 200.8 2.3 SANDY SILT: trace clay, brown, very moist, compact 290.1 3.0 3.0 SAND: brown, moist, compact 200.1 201 3.0 SAND: brown, moist, compact 201 202 202 203 203 204 204 204 205 204 204 204 205 204 204 204 205 204 204 204 205 204 204 204 204 204 204 204 205 204 204 204 205 204 204 204 205 204 204 204 203 204												-	20						oxidated	1
1.5 SILTY SAND: brown, oxidated, moist, compact 2 296.8 2.3 SANDY SILT: trace clay, brown, very moist, compact 296.1 3.0 3.0 SAND: brown, moist, compact 2 296 296 297 296.1 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 297 296 298 296 299 296 290 296 291 295 292 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294												-	20					:		-
1.5 SILTY SAND: brown, oxidated, moist, compact 2 296.8 2.3 SANDY SILT: trace clay, brown, very moist, compact 296.1 3.0 3.0 SAND: brown, moist, compact 2 296 296 297 296.1 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 296 297 296 298 296 299 296 290 296 291 295 292 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294 294												-							7.6	297.6
296.8 297 297 297 296.1 296 296 3.0 SAND: brown, moist, compact 296 4 296 296 6 becoming grey and very moist 4UGER 9 9 9 9 9												-							1.5 SILTY SAND: brown, oxidated,	_ 1.5
296.8 297 0 0 0 0 2.3 SANDY SILT: trace clay, brown, very moist, compact 296 296 0 <												-							moist, compact	-
2.3 SANDY SILT: trace clay, brown, very moist, compact 296.1 296 3.0 SAND: brown, moist, compact 4 296 6 296 9 9													29							-
AUGER very moist, compact 296.1 3.0 SAND: brown, moist, compact becoming grey and very moist becoming very dense												E						 		296.8
3.0 SAND: brown, moist, compact 4 296 6 becoming grey and very moist 6 becoming very dense												-					÷ŀ.		very moist, compact	- 2.3
3.0 SAND: brown, moist, compact 4 296 6 becoming grey and very moist 6 becoming very dense												-								-
AUGER AUGER becoming grey and very moist becoming very dense												-								
AUGER AUGER becoming grey and very moist becoming very dense												-	29						3.0 SAND: brown, moist, compact	_ 3.0
AUGER AUGER becoming grey and very moist becoming very dense												-								-
AUGER AUGER becoming grey and very moist becoming very dense												F								-
AUGER AUGER becoming grey and very moist becoming very dense												-								-
AUGER AUGER becoming grey and very moist becoming very dense												-	29					:		4
becoming grey and very moist												-						·.		-
becoming grey and very moist												-								-
6 294 294 1 1 1 1 1 6 becoming very dense 293 1 1 1 1 1												-			UGEH	A			becoming grey and very moist	-
becoming very dense												-								-
becoming very dense												-	29							-
becoming very dense												-						ŀ		-
becoming very dense												-						:		-
becoming very dense												-						:		-
becoming very dense												-								6
												-	29					÷	becoming very dense	-
												E								-
9 AS 9 AS 291 291 291 291 291 291 291 291												-						:		-
												-						·.		-
												-	29							52/18
												-						ŀ		10/2
												-								
9 AS 9 AS 291 												Ē						÷		-CL.
												-			AS	9				Ъ,
												-	29					:		Ð-
												-						·. 		2016
												-						:		- - -
												F						ŀ.		334-
	1											Ē	20					. .		P18
trace gravel, moist, very dense												-	29						trace gravel, moist, very dense	0 0
	1											È i		105	SS	10		.		9
	1											F					·* .*	· ·		log
												Ē						ŀ		124

PROJECT: Proposed Residential Development

DRILLING DATA

LOG OF BOREHOLE BH 19-1A

 Image: continued local continter continued local continued local continued local continued lo

<u>GRAPH</u> <u>NOTES</u> + ³, × ³: Numbers refer to Sensitivity

O ^{8=3%} Strain at Failure

	ECT: Proposed Residential Developme IT: Palgrave Estate	nt						DRILI	LING	DATA										
CLIEN	IT: Palarave Estate																			
										llow St		uger								
	ECT LOCATION: Mount Hope Rd., Cale	edon,	Onta	ario						00 mm									P19-3	334-00
	M: Geodetic							Date:	Oct/1	0/2019)					El	NCL N	0.: 3		
вн ГС	SOIL PROFILE		· ·	AMPL	Ed			DYNA		NE PEN PLOT	IETRA	TION								
			3	AIVIPL	.ES	н								PLASTI LIMIT	C MATI	URAL	LIQUID LIMIT	7	TW -	REMARKS AND
(m)		LOT			ଷ୍ଟ	GROUND WATER CONDITIONS	z					1	00	W _P	CON	TENT N	WL	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	GRAIN SIZE
ELEV DEPTH	DESCRIPTION	TA P	ER		BLOWS 0.3 m		ATIO		NCONF	RENG ⁻ INED	і н (кі +	FIELD V & Sensit	'ANE		(э——		(CU)	TURAI (kn	DISTRIBUTION (%)
		STRATA PLOT	NUMBER	түре	r Z	ROL	ELEVATION			RIAXIAL	×	LAB V/	ANE		TER CC 0 2		T (%) 30	L.		
	SAND: brown, moist,	S S	z	F	-	00	ш 289													GR SA SI C
	compact(Continued)							-												
								-												
	trace cobbles, trace gravel, trace							-												
.	silt, light brown		11	SS	64		288	-												
							200	-												
								-												
								-												
.								-												
							287													
			10		07			-												
			12	SS	67			-												
								-												
							286	-												
								-												
								E												
	some gravel, trace silt							F												
	some graver, trace sit		13	SS	50			-												
						」 目:	285	-												
						」 目:		-												
								-												
								ŀ												
							W. L. 2 Oct 21	284.2 r , 2019	n 											
	some silt, trace clay, wet		14	DO				E												
			14	00				-												
								-												
						目		-												
						l:∃:	283	-												
								-												
202.2								-												
282.3 16.8	END OF BOREHOLE:							-						-						
	Note:																			
	 Monitoring well was installed upon completion of drilling. 																			
	2. Groundwater level was observed at 14.84 mbgs on October 18, 2019.																			
	3. Groundwater level was observed																			
	at 14.85 mbgs on October 21, 2019.																			
- 1							1													

SPCL SOIL LOG SP18-334-10 - 2019.GPJ SPCL.GDT 10/22/19

 $\begin{array}{c} \underline{\text{GROUNDWATER ELEVATIONS}} \\ \text{Measurement} \quad \underbrace{\stackrel{1\text{st}}{\underline{\checkmark}} \quad \underbrace{\stackrel{2\text{nd}}{\underline{\checkmark}} \quad \underbrace{\stackrel{3\text{rd}}{\underline{\checkmark}} \quad \underbrace{\stackrel{4\text{th}}{\underline{\checkmark}}} \\ \end{array} \end{array}$

LOG OF BOREHOLE BH 19-1A

PROJ	ECT: Proposed Residential Developme	ent						DF	RILLI	NG	DATA										
CLIEN	NT: Palgrave Estate							Me	ethoc	l: Soi	ld Ste	em Au	ger								
PROJ	ECT LOCATION: Mount Hope Rd., Ca	ledon	, Ont	ario				Dia	amet	er: 1	50 mr	n					R	EF. NC).: S	P19-3	334-00
DATU	IM: Geodetic							Da	ate: /	Aug/2	23/201	19					E	NCL N	O.: 4		
BH LC	DCATION:		-				1		/N I A N A										—		
(m)	SOIL PROFILE	LOT	5	SAMPL		GROUND WATER CONDITIONS	z		20	4		60	80	100	PLAST LIMIT W _P		TURAL STURE NTENT W	LIQUID LIMIT W _L		NATURAL UNIT WT (kN/m ³)	REMARKS AND GRAIN SIZE
ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	TYPE	"N" <u>BLOWS</u> 0.3 m		ELEVATION	0	UNG	CONF	INED RIAXIA	+ L X	FIÉLD & Sen LAB	VANE sitivity VANE 100			-0 ONTEN 20	T (%)	POCK (cu)	NATURA (KN	DISTRIBUTION (%)
298.2	FILL: sand, some cobbles, some				-	00		+		4		+		100			20	1		$\left - \right $	GR SA SI C
297.9	gravel, brown to grey, moist		41	ss	5		298	3											1		
- 0.3 - -	SILTY SAND: brown, moist, loose		4	SS	21	-		-													
- - - -			2	SS	5		297	,													
-							201	-													
	trace cobbles, dark brown, very loose		3	SS	4			-													
-		뮘					296	Ē													
295.9	SANDY SILT: trace clay, brown,		· •				200	1													
 - -	very moist, compact							-													
<u>3</u> 295.2 - 3.0	SAND: brown, moist, compact							-													
-			5	SS	25		295	5													
-								-													
4			·					-													
-							294	1 <u>-</u>							-				1		
293.6								ŀ													
- 4.6	SILTY SAND: brown, very moist,							-													
5	very dense		6	SS	64			-													
-			÷				293	3											-		
-								F													
-								-													
6000 (Ē													
⁶ 292.1 - 6.1	SAND: some clay, brown, very	414					292	2							-				-		
-	moist, dense		7	SS	42			-													
-								-													
- ק-ת			·					Ē													
			:				291	1													
<u> </u>			:					-													
	brown to grey, wet, compact		·					_													
<u>}</u>	Siourito groy, not, compact		8	SS	27			ŀ													
- -			·]				290	Ē													
							. 290	1													
								F													
- 334			:					Ē													
								ŀ													
9.1	END OF BOREHOLE:								T												
	Note: 1. Borehole caved at 7 mbgs.																				
9	2. Water was encountered at 6.7 mbgs upon completion of drilling.																				
GROUN	Continued Next Page					<u>GRAPH</u>	+ 3	׳	3. Ni	umber	s refer		C ⁸ =3	[%] Strain	at Failu	re					



DRILLING DATA

Diameter: 150 mm

Date: Aug/23/2019

Method: Soild Stem Auger

PROJECT: Proposed Residential Development

CLIENT: Palgrave Estate

PROJECT LOCATION: Mount Hope Rd., Caledon, Ontario

DATUM: Geodetic

	SOIL PROFILE		s	AMPL	ES	~		DYNA RESIS	MIC CC	NE PE PLOT		TION		PLAST	NAT	URAL	LIQUID		Þ	RE	MAR	٢S
(m) <u>ELEV</u> DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	түре	"N" <u>BLOWS</u> 0.3 m	GROUND WATER CONDITIONS	ELEVATION	SHE/ 0 U 0 Q	AR ST NCONF UICK TI	I RENG INED RIAXIAI	⊥ GTH (kl + ∟ ×	I FIELD V & Sensit LAB V	ANE		ER CO	W O ONTEN		POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	GR/ DISTI	(%)	IZI TIC
	3. Monitoring well was installed upon completion of drilling. 4. Groundwater level was observed at 5.8 mbgs on August 30, 2019. 5. Groundwater level was observed to be dry on October 8, 2019. 7. Groundwater level was observed at 6.53 mbgs on October 18, 2019. 8. Groundwater level was observed at 6.52 mbgs on October 21, 2019.																					

REF. NO.: SP19-334-00 ENCL NO.: 4

PRC	JECT: Proposed Residential Developme	ent						DRII	LING	DATA										
CLIE	ENT: Palgrave Estate							Meth	od: So	oild Ste	em Aug	ger								
PRC	DJECT LOCATION: Mount Hope Rd., Cal	edon,	, Ont	ario				Dian	neter: ²	150 mi	n					R	EF. NC).: S	P19-	334-00
DAT	UM: Geodetic							Date	: Aug	23/20	19					El	NCL N	O.: 5		
BHI	LOCATION:					1														
	SOIL PROFILE		5	SAMPL	ES	~		RESI	STANC	E PLOT		-		PLAST	C NAT	URAL			⊧	REMARKS
(m)		1				GROUND WATER CONDITIONS			20	40	60	80	100	PLASTI LIMIT		TURE ITENT W	LIQUID LIMIT	DEN.	NATURAL UNIT WT (kN/m ³)	AND GRAIN SIZE
ELEV		STRATA PLOT	Ř		BLOWS 0.3 m		ELEVATION		AR ST		GTH (k	Pa) FIELD	VANE	W _P		w o	WL	CKET Su) (x	JRAL ((kN/m	DISTRIBUTION
DEPTH		RAT,	NUMBER	түре			EVA.		QUICK T		LX		sitivity VANE	WA	TER CO	ONTEN	T (%)	d S	NATU	(%)
298.2	2	st	Ĩ	∠	ž	<u></u> В С	Ц		20	40	60	80	100	1	0 2	20	30			GR SA SI CL
- 0.0	D POSSIBLE FILL: sand, brown, moist	\bigotimes					298													IBL in ppm
-			11	SS	12		200	-												
297.4								-												
- 0.8								E												
F.			2	SS	4			-												
			-				297	-												
-	trace cobbles, loose							Ē												
-			3	SS	5			-												
2								-												
-							296	-			-		_	-						
F			4	SS	5			[
-								-												
3								-												
-			5	SS	4		295													
-			Ľ				200	-												
-								-												
È.								Ē												
-								[
-]				294	-	-					+						
F								-												
-	becoming grey, very moist and very dense		6	SS	53			-												
5			Ľ	00	00			-												
Ē							293						_							
-								-												
F			1					E												
6								-												
Ē	becoming dense						000													
-			7	SS	41		292	-												
-																				
-								-												
22/19								-												
10/22/							291						_	-						
- GDT			·					-												
SPCL					200			-												
			8	SS	36			Ē												
19.GPJ							290													
- 2019.							200	-												
4 1 1								-												
sP18-334-10 . 			1					E										1	1	
0,								ŀ										1	1	
			1				289	-	1		1			1		1		1		
<u>s</u> t								E										1	1	
5 - -								F											1	
G 10	Continued Next Page	<u> </u>						ŀ										1		
<u>GROL</u>	INDWATER ELEVATIONS				<u>(</u>	<u>GRAPH</u> NOTES	+ 3,	× ³ :	Numbe to Sens	ers refer sitivity	Ċ	8 =3	[%] Strair	ı at Failur	e					
Measu	urement $\underline{\nabla}$ and $\underline{\nabla}$ and $\underline{\nabla}$ are $\underline{\nabla}$ and $\underline{\nabla}$									•										

PROJECT LOCATION: Mount Hope Rd., Caledon, Ontario										Diameter: 150 mm								REF. NO.: SP19-334-00								
	M: Geodetic							Date:	Aug/2	23/201	9					EN	ICL NO	D.: 5								
BH LC	DCATION:																		1	i						
(m) <u>ELEV</u> DEPTH	SOIL PROFILE	STRATA PLOT	NUMBER	AMPL BALL	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION	2 SHEA O UN	0 4 R STI NCONF	RENG	0 8 TH (kF + ×	Pa) FIELD V. & Sensiti LAB VA	ANE vity				LIQUID LIMIT W _L (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m ³)	REMARKS AND GRAIN SIZE DISTRIBUTION (%)						
	SAND: brown, moist, very loose(Continued)	8	2 9	AS	2		 287 286 285									0 3				GR SA SI C						
<u>14.0</u>	END OF BOREHOLE: Note: 1. Borehole caved at 7 mbgs. 2. Water was encountered at 12.2 mbgs upon completion of drilling. 3. Monitoring well was installed upon completion of drilling. 4. Monitoring well was observed to be dry on September 5, 2019. 5. Monitoring well observed to be dry on August 30, 2019. 5. Monitoring well observed to be dry on September 5, 2019. 6. Monitoring well observed to be dry on October 8, 2019.																									

DRILLING DATA

Method: Soild Stem Auger Diameter: 150

REF NO . SP10-334-00

LOG OF BOREHOLE BH 19-3

PROJECT: Proposed Residential Development CLIENT: Palgrave Estate

PRO IECT LOCATION: Mount Hope Rd Caledon Ontario

LOG C	OF BOREHO	LE BH 19-3A
-------	------------------	-------------

DRILLING DATA

Diameter: 200 mm

Method: Hollow Stem Auger

PROJECT: Proposed Residential Development

CLIENT: Palgrave Estate

PROJECT LOCATION: Mount Hope Rd., Caledon, Ontario

DATUM: Geodetic

	M: Geodetic							Date	: Oct/	11/201	9					E١	ICL N	0.: 6		
BHLC	CATION:							DYN/	AMIC CO	DNE PEI	NETRA	TION		1				1	T	
	SOIL PROFILE	-	L.	SAMPL	E2	Ë			AMIC CO STANCE					PLAST	IC NAT MOIS CON	URAL	LIQUID LIMIT	-	۲M.	REMARKS
(m)		10			<u>v</u> _	GROUND WATER CONDITIONS	7			1		1	00	W _P		TENT N	UMIT WL	POCKET PEN. (Cu) (KPa)	NATURAL UNIT WT (kN/m ³)	AND GRAIN SIZE
ELEV DEPTH	DESCRIPTION	STRATA PLOT	ĥ		BLOWS 0.3 m		ELEVATION		AR ST		TH (kl +	Pa) FIELD V & Sensit	ANE	ļ Ē		0	ī	DCKE DCKE	(KN/r	DISTRIBUTION
DEPTH		RAT	NUMBER	ТҮРЕ	= =		EVA	• (QUICK T	RIAXIAL	- ×	LAB V	ANE		TER CO			ď.	NAT	(%)
298.2		st	ž	۲ ۲	ŗ	50	Ш		20 4	40 6	60 E	30 1	00	1	0 2	20 3	30 			GR SA SI C
- 0.0	POSSIBLE FILL: sand, brown, moist	\otimes					298	-												IBL in ppm
-		\otimes					200													
- -		\bigotimes						-												
297.4	SAND: brown, moist, very loose	PY.	ł					-												
-	- , , , ,		i					[
-							297													
-	trace cobbles, loose							-												
Ē								-												
2								-												
-							296	-												
-							230	-												
Ē			1					F						1					1	
								F						1					1	
-								E						1					1	
-							295													
-								-												
-								-												
4								-												
-							204	-												
-							294	_												
-	becoming grey, very moist and very		÷																	
-	dense		1					-												
5								-												
-							293											-		
-			. '	AUGEF	ς.															
-								-												
6								-												
-	becoming dense		i				000	-												
-	U U						292	-										1		
-								[
-								-												
								F						1					1	
							291	<u> </u>		-				-				-		
<u>,</u>								ŀ												
			1					L												
δ[8								ļ.						1					1	
- -								F						1					1	
			1				290	E						1				1	1	
								E						1					1	
100-			1					F						1					1	
2 9								F												
2			1				289											-		
								E						1					1	
			i					Ł						1					1	
			1				L	Ł												
	Continued Next Page					GRAPH	1 , 3	√ 3.	Numbe	rs refer		8=3%	Chr!	at Failur						
GROUN	$\begin{array}{c} \begin{array}{c} \\ \hline \\ $					<u>GRAPH</u> NOTES	2	<u>~</u> ·	Numbe to Sens	itivity	C	,	Suan	ai Fallul	e					
Measure	ement $\underline{\vee}$ $\underline{\mathbf{V}}$ $\underline{\mathbf{V}}$ $\underline{\mathbf{V}}$																			

REF. NO.: SP19-334-00

LOG OF BOREHOLE	BH 19-3A
-----------------	----------

DRILLING DATA

Diameter: 200 mm

Date: Oct/11/2019

Method: Hollow Stem Auger

PROJECT: Proposed Residential Development

CLIENT: Palgrave Estate

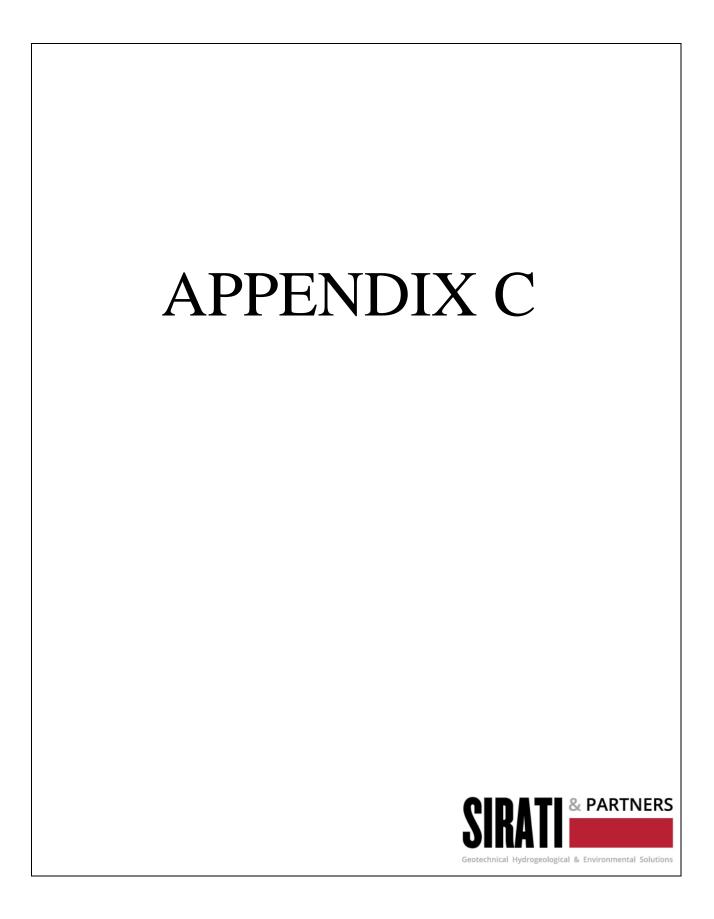
PROJECT LOCATION: Mount Hope Rd., Caledon, Ontario

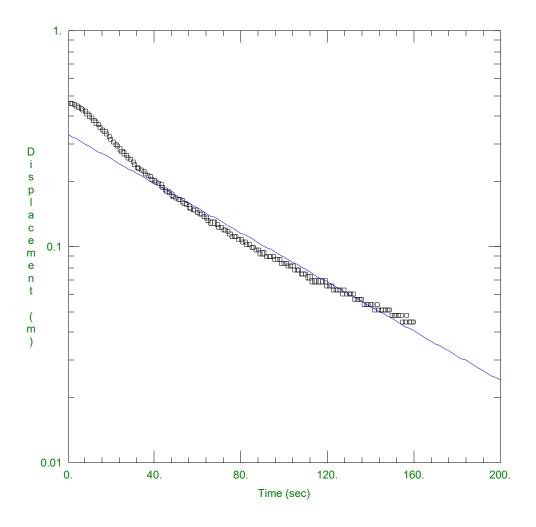
DATUM: Geodetic

BH LOCATION:

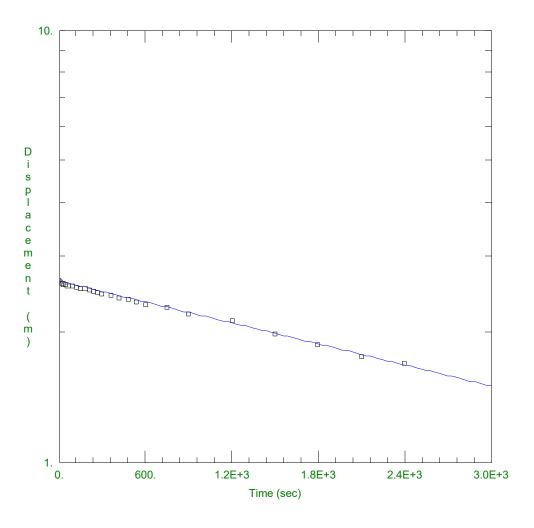
		SOIL PROFILE		5	SAMPL	FS			DYN/	AMIC CC STANCE		NETRA	TION								
							GROUND WATER CONDITIONS							00	PLASTI LIMIT	C NAT MOIS	URAL	LIQUID LIMIT	z	NATURAL UNIT WT (kN/m ³)	REMARKS AND
	m)		Lot			ଷ୍ଟ	NAT NS	z				1		1	W _P	CON	TENT N	WL	POCKET PEN. (Cu) (kPa)	-NN ()	GRAIN SIZE
EI	<u>EV</u> PTH	DESCRIPTION	AP	Ë		BLOWS 0.3 m		ŌĘ	SHE O L	AR ST	RENG	ін (кі +	-a) FIELD V	ANE	⊢		0	1	(CU)	URAI (KN	
			STRATA PLOT	NUMBER	ТҮРЕ			ELEVATION	• 0	QUICK T	RIAXIAL	. ×	LAB VA	ANE		TER CO		• •	ē.	TAT	(%)
			5	ž	F	ż	50	Щ	-	20 4	10 6	80 8	80 1	00	1	0 2	20 ;	30			GR SA SI CL
F		SAND: brown, moist, very loose(Continued)						288													
-								200	-												
F_2	37.5								-												
	10.7	SAND: light brown, moist							-												
11				8	DO				-												
2	37.0							287	-												
F	11.3	Augered to 16.8 mbgs							E .												
E									F												
E									-												
12									-												
Ł								286	-										-		
Ŀ									F												
-									-												
-								÷.	-												
<u>13</u>								·	F												
Ę								285											-		
Ē							日日	•	-												
Ē							日	· .	_												
14							ŀ∶≣∶	·	-												
-							ŀΕ	· . · .	-												
F								284	-												
F									L 283.8	m											
E							日日	Oct 2	, 201 F	9											
15							ĽΕ		F												
E									F												
-							l∶∃.	283	-										1		
F								÷.	E												
Ę							日日		F												
16							日日		Ł												
F								282	-												
F							l.∙ ⊟.	0_	-												
F.				1					ŧ												
	31.4 16.8	END OF BOREHOLE:							-	+										-	
SPCL SOIL LOG SP18-334-10 - 2019.GPJ SPCL.GDT 10/22/19																					
10/:		Note: 1. Borehole caved at 16.8 mbgs.																			
5DT		Monitoring well was installed																			
CL.O		upon completion of drilling. 3. Groundwater level was observed																			
с С		at 14.45 mbgs on October 18, 2019. 4. Groundwater level was observed																			
GPJ		at 14.44 mbgs on October 21, 2019.																			
019.				1																	
0 - 2																					
34-1																					
18-3				1																	
SP				1																	
90																					
ы С				1																	
SP(1																	

REF. NO.: SP19-334-00 ENCL NO.: 6

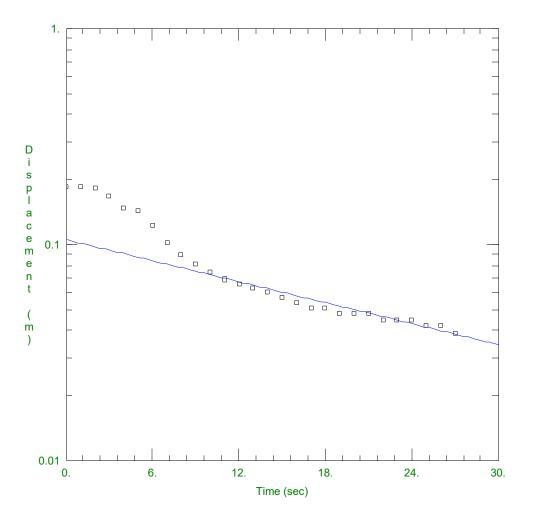




FALLING HEAD TEST RESULTS										
Data Set: Date: <u>10/22/19</u>	Time: <u>14:30:28</u>									
PROJECT INFORMATION										
Company: <u>Sirati and Partners</u> Client: <u>JOE TRIUMBARI</u> Project: <u>SP18-334-30</u> Location: <u>17791 MT HOPE ROAD, CALEDON</u> Test Well: BH19-1A Test Date: <u>October 21, 2109</u>										
AQUIFER DATA										
Saturated Thickness: <u>1.7</u> m	Anisotropy Ratio (Kz/Kr): <u>1.</u>									
WELL DATA (BH19-1A)										
Initial Displacement: <u>0.459</u> m Total Well Penetration Depth: <u>16.8</u> m Casing Radius: <u>0.05</u> m	Static Water Column Height: <u>1.7</u> m Screen Length: <u>3.</u> m Well Radius: <u>0.05</u> m									
SOLUTION										
Aquifer Model: Unconfined	Solution Method: Hvorslev									
K = <u>3.833E-5</u> m/sec	y0 = <u>0.3313</u> m									



RISING HEAD TEST RESULTS									
Data Set: Date: <u>10/22/19</u>									
PROJECT IN	FORMATION								
Company: <u>Sirati and Partners</u> Client: <u>JOE TRIUMBARI</u> Project: <u>SP18-334-30</u> Location: <u>17791 MT HOPE ROAD, CALEDON</u> Test Well: BH19-2 Test Date: <u>October 8, 2109</u>									
AQUIFER DATA									
Saturated Thickness: 2.82 m	Anisotropy Ratio (Kz/Kr): <u>1.</u>								
WELL DAT	A (BH19-2)								
Initial Displacement:2.63 mStatic Water Column Height:2.82 mTotal Well Penetration Depth:9.1 mScreen Length:3. mCasing Radius:0.05 mWell Radius:0.05 m									
SOLL	SOLUTION								
Aquifer Model: Unconfined	Solution Method: <u>Hvorslev</u>								
K = <u>3.285E-7</u> m/sec	y0 = <u>2.633</u> m								



Data Set:	S:\\BH19-3A.aqt
Date: <u>11/1</u>	9/19

Time: <u>09:06:04</u>

PROJECT INFORMATION

FALLING HEAD TEST RESULTS

Company: <u>Sirati and Partners</u> Client: <u>JOE TRIUMBARI</u> Project: <u>SP18-334-30</u> Location: <u>17791 MT HOPE ROAD, CALEDON</u> Test Well: BH19-3A Test Date: October 21, 2109

AQUIFER DATA

Saturated Thickness: 1.51 m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH19-3A)

Initial Displacement: <u>0.186</u> m Total Well Penetration Depth: <u>16.5</u> m Casing Radius: <u>0.05</u> m Static Water Column Height: <u>1.51</u> m Screen Length: <u>3.</u> m Well Radius: 0.05 m

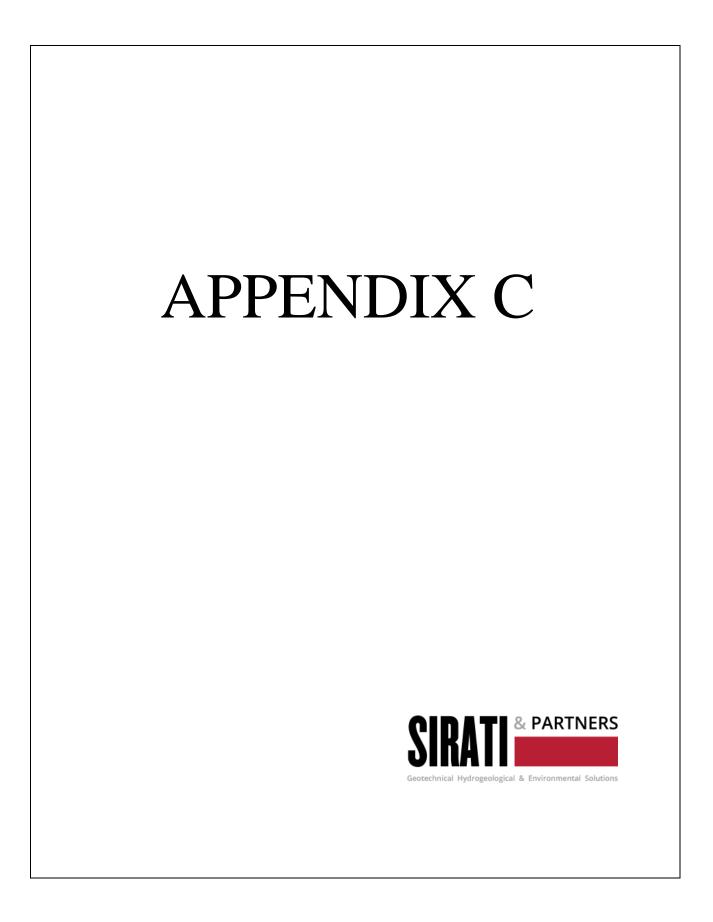
SOLUTION

Aquifer Model: Unconfined

Solution Method: Hvorslev

K = 0.0001226 m/sec

y0 = 0.1053 m



				Soil			Snow		
Date	PET	P =========	P-PET ========	Moisture		PET-AET ========	Storage	Surplus	ROtotal
Jan-62	8.1	48	-7.6	144.3	6.2	1.9	47.5	0	12.7
Feb-62	8.5	72.1	-7.5	138.9	6.4	2.1	118.6	0	6.4
Mar-62	16.4	12.2	18.7	157.6	16.4	0	95.4	0	3.5
Apr-62	33.8	42.2	54	200	33.8	0	47.7	11.5	9.5
May-62	66.9	58.4	12.4	200	66.9	0	23.8	12.4	12.8
Jun-62	78.7	50	-19.3	180.7	78.7	0	11.9	0	7.5
Jul-62	82.4	90.9	9.9	190.6	82.4	0	6	0	7
Aug-62	77.5	37.3	-36.1	156.2	75.8	1.7	0	0	3.1
Sep-62	45.1	86.4	37	193.2	45.1				4.9
Oct-62		134.4	96	200	31.7				51.6
Nov-62	15.5	113.5	79.6	200	15.5			79.6	66.8
Dec-62		71.1	18.3	200	9.6	0			41.2
Jan-63	7.6	25.7	-7.6	192.4	7.6	0	81.8		20.1
Feb-63	7.6	15	-7.6	185.1	7.3	0.3			10.1
Mar-63	18.2	57.2	49.8	200	18.2	0			24.2
Apr-63	33	59.4	65.6	200	33	0			47
May-63	51.2	95	60.1	200	51.2				56.8
Jun-63	80.7	14.5	-56.4	143.6	80.7		10.5		26.8
Jul-63	92.7	70.9	-20.1	129.2	87	5.7			16.6
Aug-63	69.8	80	11.5	140.7	69.8	0			10.5
Sep-63	43.7	65.8	18.8	159.5	43.7				6.5
Oct-63	37.7		-20.3	143.3	33.6				2.5
Nov-63	19.8	71.1	47.8	191.1	19.8		0		4.4
Dec-63	8.2	58.9	-1.3	189.8	8.1				0.7
Jan-64	10.3	72.6	25.9	200	10.3	0		15.7	9.2
Feb-64	9.6	31.8	4.2	200	9.6	0			6.4
Mar-64	17.6	73.4	59.4	200	17.6	0			34.8
Apr-64	31.2	71.9	86.7	200	31.2	0			63.3
May-64		59.7	19.1	200	62.5	0			42.4
Jun-64	77.7		-33.2	166.8	77.7				21.4
Jul-64	98			200	98	0			27.5
Aug-64	66.8	136.1	68.7	200	66.8	0			51.1
Sep-64		21.6	-27.8	172.2				-	23.2
Oct-64	27.8		6.7	178.9					12.9
Nov-64	18.1		12.6	191.5	18.1			0	7.1
Dec-64			22.8	200	10.7				11.3
Jan-65	8.6	88.9	3	200	8.6	0			6.8
Feb-65		96.5	24.8	200	9.8				16.6
Mar-65	14.8		36.4	200	14.8				26.9
Apr-65	26.2		87.8	200	26.2				59
May-65	61.7		58.7	200	61.7				61.6
Jun-65	73.5	35.3	-15.5	184.5	73.5				30.7
Jul-65				176.3					17.4
Aug-65	72.9	96.5	24.9	200	72.9	0	6.1	1.2	12.7

Sep-65	51.7	80.8	31.2	200	51.7	0	0	31.2	23.5
Oct-65	27.8	121.2	87.3	200	27.8	0	0	87.3	59.5
Nov-65	16.3	93.5	65.5	200	16.3	0	7.6	65.5	63.5
Dec-65	12.5	74.2	40.6	200	12.5	0	26.5	40.6	52.3
Jan-66	8.4	67.6	-1.9	198.1	8.4	0	87.4	0	25.2
Feb-66	10.5	37.1	14.5	200	10.5	0	98.9	12.6	19.3
Mar-66	19.6	60.2	59.9	200	19.6	0	77.6	59.9	41.4
Apr-66	28.8	48	54.2	200	28.8	0	40.2	54.2	49.2
May-66	47	69.1	38.8	200	47	0	20.1	38.8	46.3
Jun-66	81.2	101.3	25.1	200	81.2	0	10.1	25.1	39
Jul-66	96.2	40.4	-52.8	147.2	96.2	0	5	0	19
Aug-66	78.5	74.7	-2.5	145.4	77.8	0.7	0	0	12.2
Sep-66	45.1	87.9	38.4	183.8	45.1	0	0	0	8.6
Oct-66	28.8	52.3	20.8	200	28.8	0	0	4.6	7
Nov-66	17.7	112.5	89.4	200	17.7	0	0	89.4	52.3
Dec-66	10.3	72.1	22.8	200	10.3	0	37.8	22.8	36.1
Jan-67	11.1	66.3	29.4	200	11.1	0	62.2	29.4	33.5
Feb-67	8.1	46.7	-8.1	191.9	8.1	0	108.9	0	16.1
Mar-67	16.4	22.1	22	200	16.4	0	92	14	15.5
Apr-67	32	74.9	85.1	200	32	0	46	85.1	53.8
May-67	43.9	45.2	22.1	200	43.9	0	23	22.1	38.3
Jun-67	89.1	191.8	104.6	200	89.1	0	11.5	104.6	79.9
Jul-67	86	78.7	-5.5	194.5	86	0	5.7	0	39.1
Aug-67	70.7	111.8	41.3	200	70.7	0	0	35.8	41.1
Sep-67	45.4	75.9	26.7	200	45.4	0	0	26.7	34.9
Oct-67	29.6	99.8	65.2	200	29.6	0	0	65.2	53.2
Nov-67	14.5	66.8	37.6	200	14.5	0	12.3	37.6	45.3
Dec-67	11.4	88.1	42.9	200	11.4	0	44	42.9	45
Jan-68	7.9	65.5	-7.9	192.1	7.9	0	109.5	0	21.4
Feb-68	8.4	29.7	-8.4	184	8.1	0.3	139.2	0	10.7
Mar-68	19.9	55.4	72.2	200	19.9	0	100.5	56.3	35.4
Apr-68	36.4	40.4	52.2	200	36.4	0	50.3	52.2	44.9
May-68	49.4	92.7	63.8	200	49.4	0	25.1	63.8	58
Jun-68	78.2	62	-6.8	193.2	78.2	0	12.6	0	29.8
Jul-68	91.5	68.6	-20.1	173.8	90.9	0.7	6.3	0	16.8
Aug-68	78	150.4	71.1	200	78	0	0	45	36.7
Sep-68	56.1	125.5	63.2	200	56.1	0	0	63.2	52.4
Oct-68	33.9	67.3	30	200	33.9	0	0	30	41.5
Nov-68	16.2	107.7	77.6	200	16.2	0	9.3	77.6	62.4
Dec-68	9.5	73.7	17.3	200	9.5	0	55.2	17.3	38.6
Jan-69	9.5	63.5	13.5	200	9.5	0	95.1	13.5	26.2
Feb-69	10.1	39.4	12.5	200	10.1	0	111.5	12.5	19.4
Mar-69	15.9	56.1	38.5	200	15.9	0	111.9	38.5	30 86 8
Apr-69	34.2	117.3	133.1	200	34.2	0	55.9	133.1	86.8
May-69	54.5 72.6	84.1 85.6	53.4 22.7	200	54.5 72.6	0	28	53.4 22.7	71.4
Jun-69	72.6	85.6	22.7	200	72.6	0	14	22.7	49.2
Jul-69	88.7	56.1	-28.5	171.5	88.7	0	7	0	25.3

Aug-69	86.2	56.4	-25.6	149.6	82.5	3.6	0	0	14
Sep-69	51.4	15.7	-36.5	122.3	42.2	9.2	0	0	6.4
Oct-69	29.2	82	48.7	171	29.2	0	0	0	6.9
Nov-69	17.2	102.6	80.6	200	17.2	0	0	51.6	32
Dec-69	9.1	52.3	6.2	200	9.1	0	36.4	6.2	17.3
Jan-70	7.2	45.2	-7.2	192.8	7.2	0	81.6	0	8.3
Feb-70	9.1	23.1	-2	190.9	9.1	0.1	97.5	0	4.3
Mar-70	15.2	48.8	26.3	200	15.2	0	103.9	17.2	11.6
Apr-70	32.8	72.6	88.1	200	32.8	0	51.9	88.1	53
May-70	59.4	70.9	33.9	200	59.4	0	26	33.9	45.2
, Jun-70	79.2	53.6	-15.3	184.7	79.2	0	13	0	23.5
Jul-70	95.6	90.7	-2.9	182	95.4	0.2	6.5	0	14.9
Aug-70	80	86.4	8.6	190.6	80	0	0	0	9.5
Sep-70	52.4	110.7	52.8	200	52.4	0	0	43.4	29.8
Oct-70	32.5	101.1	63.6	200	32.5	0	0	63.6	49
Nov-70	17.8	57.2	36.7	200	17.8	0	0	36.7	43.1
Dec-70	9.4	80.3	17.1	200	9.4	0	52.9	17.1	29.7
Jan-71	7.7	44.2	-7.7	192.3	7.7	0	97.1	0	14.3
Feb-71	10.4	67.3	24.8	200	10.4	0	128.4	17.1	16.6
Mar-71	14.9	36.8	24.4	200	14.9	0	125.3	24.4	20.7
Apr-71	28.6	25.9	56.4	200	28.6	0	64.9	56.4	39.5
May-71	54.5	31	7.4	200	54.5	0	32.5	7.4	24.4
Jun-71	85.9	81.5	7.8	200	85.9	0	16.2	7.8	19.4
Jul-71	86.6	87.1	4.3	200	86.6	0	8.1	4.3	14.2
Aug-71	75.6	97.5	25.1	200	75.6	0	0	25.1	22.3
Sep-71	57.8	21.8	-37.1	162.9	57.8	0	0	0	9.8
Oct-71	39.1	37.8	-3.2	160.3	38.5	0.6	0	0	6.3
Nov-71	16.5	45	26.5	186.8	16.5	0	0	0	4.1
Dec-71	12	106.4	56.6	200	12	0	34.8	43.5	25.7
Jan-72	9.3	56.4	7.7	200	9.3	0	73.7	7.7	15.8
Feb-72	8.8	56.1	-2.7	197.3	8.8	0	123.5	0	7.8
Mar-72	14.1	87.6	36.3	200	14.1	0	159.5	33.6	21.9
Apr-72	25.9	59.9	94	200	25.9	0	96.9	94	59.9
May-72	61.3	56.1	40.4	200	61.3	0	48.5	40.4	51.7
Jun-72	71.3	109.5	57	200	71.3	0	24.2	57	58.4
Jul-72	92.7	36.8	-45.6	154.4	92.7	0	12.1	0	28.3
Aug-72	76.6	42.7	-30	131.3	69.8	6.8	6.1	0	15.4
Sep-72	53	50.5	1	132.3	53	0	0	0	9.1
Oct-72	25.8	116.3	84.7	200	25.8	0	0	17	17.6
Nov-72	14.9	62.2	35	200	14.9	0	10	35	25.7
Dec-72	10.9	124.2	57	200	10.9	0	63.6	57	42.9
Jan-73	10.3	44.2	17.1	200	10.3	0	79.7	17.1	29.4
Feb-73	8.7	42.9	-4.9	195.1	8.7	0	118.7	0	14.4
Mar-73	23.7	98	119.9	200	23.7	0	68.6	115	69.1
Apr-73	34	76.7	73.1	200	34	0	34.3	73.1	72.7
May-73	51.9	98	58.4	200	51.9	0	17.2	58.4	68.5
Jun-73	87.5	78	-4.8	195.2	87.5	0	8.6	0	35.7

Jul-73	97.4	46	-45.1	151.2	96.3	1.1	0	0	18.2
Aug-73	92.2	115.8	17.8	168.9	92.2	0	0	0	13.7
Sep-73	51.7	50	-4.2	165.4	51.1	0.7	0	0	6.5
Oct-73	34.1	95.3	56.4	200	34.1	0	0	21.8	17.7
Nov-73	17.1	89.2	68	200	17.1	0	0	68	44.5
Dec-73	10.3	75.7	25	200	10.3	0	39	25	34.1
Jan-74	9.8	102.1	28.6	200	9.8	0	101.4	28.6	31.9
Feb-74	8.5	69.6	-7.3	192.7	8.5	0	169.8	0	15.3
Mar-74	17.4	60.5	67.2	200	17.4	0	144.1	59.9	39.2
Apr-74	35.1	104.1	135.8	200	35.1	0	72.1	135.8	91.9
May-74	49.7	106.2	87.3	200	49.7	0	36	87.3	92.3
Jun-74	79.2	64.8	0.4	200	79.2	0	18	0.4	46.9
Jul-74	96.2	33.5	-55.4	144.6	96.2	0	9	0	23.5
Aug-74	84	96.5	16.6	161.3	84	0	0	0	15.7
Sep-74	47.7	51.1	0.8	162.1	47.7	0	0	0	8
Oct-74	28.3	36.6	6.5	168.6	28.3	0	0	0	4.6
Nov-74	17.4	100.1	78.1	200	17.4	0	0	46.6	29.4
Dec-74	12.2	41.7	15.5	200	12.2	0	12.8	15.5	21.3
Jan-75	11	52.6	16.8	200	11	0	36.5	16.8	19.5
Feb-75	10.9	90.7	31	200	10.9	0	84	31	26.1
Mar-75	15.5	66.3	35.1	200	15.5	0	98.3	35.1	31.2
Apr-75	25.3	57.9	64.4	200	25.3	0	64.2	64.4	49.5
May-75	71.6	59.4	16.9	200	71.6	0	32.1	16.9	35
Jun-75	86.9	66.5	-7.7	192.3	86.9	0	16	0	19.3
Jul-75	99.2	85.1	-10.4	182.3	98.8	0.4	8	0	12.3
Aug-75	81.5	117.9	38.5	200	81.5	0.1	0	20.9	20.3
Sep-75	45.1	65	16.6	200	45.1	0	0	16.6	18.8
Oct-75	32.5	28.4	-5.5	194.5	32.5	0	0	0.0	9.2
Nov-75	21.3	52.6	28.7	200	21.3	0	0	23.2	18.1
Dec-75	9.5	83.3	19.4	200	9.5	0	53.3	19.4	18.5
Jan-76	7.6	88.4	-7.6	192.4	7.6	0	141.7	10.4	8.7
Feb-76	12	91.2	66.7	200	12	0	152.3	59	35.8
Mar-76	19.3	106.4	111.1	200	19.3	0	124.8	111.1	76
Apr-76	35.8	72.6	95.6	200	35.8	0	62.4	95.6	87.7
May-76	52.2	85.3	60	200	52.2	0	31.2	60	76.3
Jun-76	89.1	76.5	-0.8	199.2	89.1	0	15.6	0	39.8
Jul-76	89.9	112.8	25.1	200	89.9	0	7.8	24.3	35.8
Aug-76	85.1	110.5	27.7	200	85.1	0	0.	27.7	34.4
Sep-76	48.9	116.8	62.1	200	48.9	0	0	62.1	51.3
Oct-76	25	74.4	45.7	200	25	0	0	45.7	49.3
Nov-76	13.5	35.8	12.1	200	13.5	0	9	12.1	30
Dec-76	8.2	41.7	-2.9	197.1	8.2	0	45.3	0	14.6
Jan-77	6.7	72.7	-6.7	190.5	6.6	0.1	118	0	7.2
Feb-77	9.7	33.3	8.3	190.5	9.7	0.1	133	0	3.9
Mar-77	21.3	85.2	0.3 102.1	200	21.3	0	91.3	101	55.7
Apr-77	36.2	40.7	48.1	200	36.2	0	91.5 45.7	48.1	52.2
May-77	65.6	23.7	-20.3	179.7	65.6	0	22.8	48.1	26.3
11109-77	0.00	23.7	-20.3	1/5./	05.0	0	22.0	0	20.5

Jun-77	74.4	73	6.3	186	74.4	0	11.4	0	16.2
Jul-77	99.8	90.3	-8.3	178.3	99.3	0.6	5.7	0	10.8
Aug-77	76.1	164	85.4	200	76.1	0	0	63.7	43.2
-				200				84.7	
Sep-77	51.1	142.9	84.7		51.1	0	0		67
Oct-77	28.5	86.8	54	200	28.5	0	0	54	61.2
Nov-77	17.6	85.9	64.3	200	17.6	0	0	64.3	64.6
Dec-77	9.7	114.6	34.1	200	9.7	0	69.1	34.1	49
Jan-78	8	82.5	-8	192	8	0	151.6	0	23.7
Feb-78	7.5	17.2	-7.5	184.8	7.2	0.3	168.8	0	11.8
Mar-78	14.7	50	35.5	200	14.7	0	167.7	20.4	16.9
Apr-78	27.9	61.6	107.7	200	27.9	0	90.9	107.7	64.8
May-78	60.2	100.2	80.4	200	60.2	0	45.4	80.4	76.2
Jun-78	79.7	64.8	4.6	200	79.7	0	22.7	4.6	41.1
Jul-78	94.4	23.8	-60.5	139.5	94.4	0	11.4	0	20.1
Aug-78	82.5	153.8	69.3	200	82.5	0	5.7	8.8	21.6
Sep-78	49.2	152.2	101.1	200	49.2	0	0	101.1	65.1
Oct-78	29	45.2	13.9	200	29	0	0	13.9	38
Nov-78	16.5	63.1	43.8	200	16.5	0	0	43.8	42.5
Dec-78	10.7	72.8	26.7	200	10.7	0	33.9	26.7	34.7
Jan-79	8.3	67.2	-3.2	196.8	8.3	0	95.8	0	16.8
Feb-79	7	29.2	-7	189.9	6.9	0.1	125	0	8.3
Mar-79	20.5	83.4	92.9	200	20.5	0	91.9	82.8	48.6
Apr-79	29.7	114.2	124.7	200	29.7	0	45.9	124.7	90.8
May-79	55.2	87.4	50.8	200	55.2	0	23	50.8	72.3
Jun-79	80.7	78.4	5.3	200	80.7	0	11.5	5.3	40.5
Jul-79	93.8	25	-64.3	135.7	93.8	0	5.7	0	19.6
Aug-79	74.7	89.2	15.8	151.4	74.7	0	0	0	13.6
Sep-79	50.8	37.4	-15.2	139.9	47.1	3.7	0	0	6.4
Oct-79	28.8	112.2	77.7	200	28.8	0	0	17.6	16.7
Nov-79	17.2	110.4	88	200	17.2	0	0	88	54.7
Dec-79	11.8	91.2	45.2	200	11.8	0	31.8	45.2	49.8
Jan-80	9.7	47.4	8.2	200	9.7	0	60.8	8.2	28.3
Feb-80	8.6	19.3	-7.3	192.7	8.6	0	78.8	0	13.9
Mar-80	16.3	64	38.3	200	16.3	0	86.6	31.1	24
Apr-80	32.4	111.7	117	200	32.4	0	43.3	117	75.3
May-80	62.5	34.2	-8.3	191.7	62.5	0	21.7	0	36.6
Jun-80	69.1	102	38.6	200	69.1	0	10.8	30.3	37.7
Jul-80	95	156.2	58.8	200	95	0	5.4	58.8	53.5
Aug-80	87.2	46	-38.1	161.9	87.2	0	0	0	25.1
Sep-80	48.9	86	32.8	194.7	48.9	0	0	0	15.7
Oct-80		66	37.4	200		0	0	32	25
	25.3				25.3				
Nov-80	15.2	50.8	26.6	200	15.2	0	7	26.6	26.1
Dec-80	8.4	59.4	1.9	200	8.4	0	55.7	1.9	13.4
Jan-81	7.2	13	-7.2	192.8	7.2	0	68.7	0	6.5
Feb-81	12.6	53.7	35.8	200	12.6	0	72.7	28.6	18.9
Mar-81	18.5	31.2	25.9	200	18.5	0	58.5	25.9	22.7
Apr-81	34.7	43.6	36	200	34.7	0	29.3	36	31
•									

May-81	53.5	60	18.1	200	53.5	0	14.6	18.1	26.5
Jun-81	82.2	101.8	21.8	200	82.2	0	7.3	21.8	27.8
Jul-81	95.6	115.4	21.3	200	95.6	0	0	21.3	27.8
Aug-81	78	136	51.2	200	78	0	0	51.2	43.4
Sep-81	48.9	88.8	35.5	200	48.9	0	0	35.5	40.5
Oct-81	26	91.6	61.1	200	26	0	0	61.1	53.1
Nov-81	16.9	61.8	42.1	200	16.9	0	0	42.1	48.1
Dec-81	10.8	44.6	12.5	200	10.8	0	20.4	12.5	29.8
Jan-82	7.1	63.2	-7.1	192.9	7.1	0	83.6	0	14.4
Feb-82	9.1	21	-2.6	190.3	9	0.1	98.1	0	7.3
Mar-82	16.4	70.2	47.1	200	16.4	0	103	37.5	24
Apr-82	29.2	56	75	200	29.2	0	52.1	75	51.4
May-82	69	60.6	14.6	200	69	0	26.1	14.6	34.7
Jun-82	70.4	132.6	68.6	200	70.4	0	13	68.6	56.8
Jul-82	96.2	83.8	-10.1	189.9	96.2	0	6.5	0	29.3
Aug-82	69.8	132.6	62.7	200	69.8	0	0	52.6	45.5
Sep-82	49.5	110.8	55.7	200	49.5	0	0	55.7	52.8
Oct-82	31.3	41.2	7.9	200	31.3	0	0	7.9	29.6
Nov-82	17.7	134.4	110.3	200	17.7	0	0	110.3	75.4
Dec-82	13.3	102.2	63.5	200	13.3	0	21.9	63.5	69.7
Jan-83	10.2	37.6	7.4	200	10.2	0	41.3	7.4	37.4
Feb-83	11.8	32.2	13.3	200	11.8	0	47.8	13.3	25.7
Mar-83	19.4	66	46.4	200	19.4	0	45.7	46.4	37.9
Apr-83	30.4	96.2	83.8	200	30.4	0	22.9	83.8	64.6
May-83	48.7	126.4	82.8	200	48.7	0	11.4	82.8	77.6
Jun-83	82.2	35	-43.2	156.8	82.2	0	5.7	0	37.4
Jul-83	104.3	75.4	-26.9	135.7	98.4	5.8	0	0	21.6
Aug-83	88.3	81.2	-11.2	128.1	84.7	3.6	0	0	13
Sep-83	56.4	43.4	-15.2	118.4	50.9	5.5	0	0	6.6
Oct-83	30.3	82	47.6	165.9	30.3	0	0	0	6.3
Nov-83	16.9	77	56.6	200	16.9	0	0	22.5	15.9
Dec-83	8.9	96.1	15.6	200	8.9	0	70.7	15.6	14.9
Jan-84	7.9	41.5	-7.9	192.1	7.9	0	112.2	0	7
Feb-84	13.3	74	66.1	200	13.3	0	104.7	58.3	34.7
Mar-84	14	75.8	28.2	200	14	0	137.2	28.2	31.5
Apr-84	34.7	50.2	81.6	200	34.7	0	68.6	81.6	58.5
May-84	55.2	82.2	57.2	200	55.2	0	34.3	57.2	60.7
Jun-84	84.3	54.6	-15.2	184.8	84.3	0	17.2	0	31
Jul-84	92.1	49.4	-36.6	150.9	89.3	2.8	8.6	0	16.6
Aug-84	88.9	73.8	-10.2	143.3	86.4	2.5	0	0	10.8
Sep-84	46.3	86.7	36.1	179.4	46.3	0	0	0	7.9
Oct-84	32.9	49.6	14.3	193.6	32.9	0	0	0	4.2
Nov-84	16.5	78.4	52.8	200	16.5	0	5.7	46.4	27.5
Dec-84	12.7	97.7	57.8	200	12.7	0	29.8	57.8	44
Jan-85	8.2	63.3	-5.8	194.2	8.2	0	90.7	0	20.6
Feb-85	9.9	108.9	27.8	200	9.9	0	160.8	22	22.3
Mar-85	18.4	95.7	97.4	200	18.4	0	137.9	97.4	62.2

Apr-85	37.4	43.3	72.7	200	37.4	0	68.9	72.7	68.2
May-85	60.6	91.6	60.9	200	60.6	0	34.5	60.9	68.1
, Jun-85	72.2	26.6	-29.7	170.3	72.2	0	17.2	0	33.1
Jul-85	92.7	59.6	-27.4	147	88.6	4.1	8.6	0	18.8
Aug-85	78.5	148.7	71.4	200	78.5	0	0.0	18.3	24.5
Sep-85	56.8	148.7	56.2	200	56.8	0	0	56.2	42.6
•									
Oct-85	30.7	62.4	28.6	200	30.7	0	0	28.6	35.7
Nov-85	16.4	114.6	84.4	200	16.4	0	8.8	84.4	63.5
Dec-85	9.4	58.8	11.6	200	9.4	0	45.9	11.6	35.8
Jan-86	9.5	30.8	3.6	200	9.5	0	63.3	3.6	19.6
Feb-86	9.4	64.3	6.1	200	9.4	0	111.7	6.1	13.1
Mar-86	19.8	59.3	64.9	200	19.8	0	84.3	64.9	40.9
Apr-86	37.1	63.1	64.9	200	37.1	0	42.1	64.9	55
May-86	66.5	72.1	23.1	200	66.5	0	21.1	23.1	41.1
Jun-86	74.9	106.9	37.2	200	74.9	0	10.5	37.2	42.7
Jul-86	101.1	122	20.1	200.0 1	1.1	0	5.3	20.1	34.8
Aug-86	75.2	145.6	68.4	200	75.2	0	0	68.4	55.8
Sep-86	50.8	218.9	157.2	200	50.8	0	0	157.2	113.8
Oct-86	29.8	64.4	31.4	200	29.8	0	0	31.4	70.4
Nov-86	15.1	33.4	17	200	15.1	0	0	17	43.3
Dec-86	11.6	53.8	21.2	200	11.6	0	19.6	21.2	33
Jan-87	9.9	70.4	16.4	200	9.9	0	62.8	16.4	24.9
Feb-87	10	20	2.3	200	10	0	70.3	2.3	13.3
Mar-87	20.5	68.7	60.7	200	20.5	0	55.2	60.7	39.5
Apr-87	20.3 39	85	69.3	200	39	0	27.6	69.3	57.4
•									
May-87	66.1	33.6	-20.3	179.7	66.1	0	13.8	0	28.2
Jun-87	88	62	-22.2	159.7	85.8	2.3	6.9	0	16.4
Jul-87	109.6	95.1		149.9 1	7.1	2.5	0	0	11.4
Aug-87	81.5	91.6	5.5	155.4	81.5	0	0	0	7.9
Sep-87	53.3	68.5	11.7	167.1	53.3	0	0	0	5.1
Oct-87	26.6	79.5	48.9	200	26.6	0	0	16	12.8
Nov-87	16.8	93.2	72.2	200	16.8	0	0	72.2	44.7
Dec-87	12.2	44.9	17.6	200	12.2	0	13.7	17.6	30.4
Jan-88	9.5	52.8	6.8	200	9.5	0	49.6	6.8	18.5
Feb-88	9.2	91.4	7.9	200	9.2	0	123.3	7.9	13.5
Mar-88	17.7	32.2	38.3	200	17.7	0	98.6	38.3	26.5
Apr-88	32.4	58.9	72.9	200	32.4	0	49.3	72.9	52.2
May-88	64.8	52.4	9.6	200	64.8	0	24.7	9.6	32
, Jun-88	80.7	48.2	-22.6	177.4	80.7	0	12.3	0	17.1
Jul-88	108.9	61.2		137.9 1	3.9	5	6.2	0	10.4
Aug-88	90	73	-14.5	127.9	85.5	4.5	0	0	7.3
Sep-88	50.8	106.1	50	177.9	50.8	۰.9 0	0	0	7.5
Oct-88	25.6	93.5	63.2	200	25.6	0	0	41.1	26.2
Nov-88	18.2	101.9	78.6	200	18.2	0	0	78.6	20.2 55.1
Dec-88	10.5	37	7.4	200	10.5	0	18.4	7.4	29.4
Jan-89	11.6	49	19.2	200	11.6	0	35.6	19.2	25.1
Feb-89	9.1	39.6	-2.4	197.6	9.1	0	68.4	0	12.2

Mar-89	15.8	58	29.5	200	15.8	0	79.8	27.1	20.8
Apr-89	29.7	47.5	55.3	200	29.7	0	39.9	55.3	39.8
May-89	57.6	106.2	63.2	200	57.6	0	19.9	63.2	55.6
Jun-89	84.8	132	50.6	200	84.8	0	10	50.6	57
Jul-89	99.2	26.2	-64.4	135.6	99.2	0	0	0	26.5
Aug-89	80	100.6	15.6	151.2	80	0	0	0	17.6
Sep-89	51.1	39.7	-13.4	141.1	47.8	3.3	0	0	8.3
Oct-89	30.7	75.2	40.7	181.9	30.7	0	0	0	6.9
Nov-89	15	154.6	110.6	200	15	0	23.1	92.5	53.7
Dec-89	6.8	41	-6.8	193.2	6.8	0	64.1	0	23.9
Jan-90	12.4	60.2	42.1	200	12.4	0	68.2	35.3	31.2
Feb-90	11.1	60.2	25.8	200	11.1	0	90.6	25.8	28.7
Mar-90	20	64.4	62.6	200	20	0	70	62.6	47.4
Apr-90	37.6	49.6	44.6	200	37.6	0	35	44.6	47.3
May-90	53.8	124	81.5	200	53.8	0	17.5	81.5	69.4
Jun-90	84.3	109.8	28.8	200	84.3	0	8.8	28.8	51.5
Jul-90	94.4	80.4	-9.3	190.7	94.4	0	0	0	27
Aug-90	82	88.1	1.7	192.4	82	0	0	0	15.9
Sep-90	50.8	90.9	35.6	200	50.8	0	0	28	24.3
Oct-90	29.6	134.4	98.1	200	29.6	0	0	98.1	65.7
Nov-90	18.6	83.1	60.4	200	18.6	0	0	60.4	63.8
Dec-90	12	98	51.2	200	12	0	32.1	51.2	58.1
Jan-91	9.1	48	3.8	200	9.1	0	66.8	3.8	30
Feb-91	12	63.3	35.8	200	12	0	80.9	35.8	34.1
Mar-91	20.2	106.1	92.4	200	20.2	0	70.6	92.4	66.4
Apr-91	37.6	133.6	124.6	200	37.6	0	35.3	124.6	100.3
May-91	72.9	65.6	7	200	72.9	0	17.6	7	53.6
Jun-91	91.3	44.1	-40.6	159.4	91.3	0	8.8	0	27.4
Jul-91	98.6	93	-1.4	158.2	98.3	0.3	0	0	17.2
Aug-91	87.8	76.6	-15	146.3	84.6	3.1	0	0	10.1
Sep-91	48.3	51.1	0.2	146.6	48.3	0	0	0	5.7
Oct-91	32.9	100.2	62.3	200	32.9	0	0	8.9	11
Nov-91	15.6	59.6	34.6	200	15.6	0	7	34.6	22.7
Dec-91	10.9	64	25	200	10.9	0	33.7	25	24
Jan-92	10.3	42.5	11.8	200	10.3	0	53.3	11.8	17.9
Feb-92	11.1	46.2	17.4	200	11.1	0	70.3	17.4	18.1
Mar-92	16.7	52.6	32.1	200	16.7	0	72.8	32.1	26
Apr-92	30.6	107.2	107.6	200	30.6	0	36.4	107.6	71.5
May-92	56.6	85.6	42.9	200	56.6	0	18.2	42.9	58.8
Jun-92	71.3	55.4	-9.6	190.4	71.3	0	9.1	0	30
Jul-92	79.9	126.6	49.5	200	79.9	0	0	39.9	39.9
Aug-92	70.7	173.8	94.5	200	70.7	0	0	94.5	72.7
Sep-92	49.8	91.4	37	200	49.8	0	0	37	55.1
Oct-92	26.5	48.6	19.7	200	26.5	0	0	19.7	37.5
Nov-92	15.9	167.2	127.4	200	15.9	0	17	127.4	88.2
Dec-92	11.6	70.6	35.8	200	11.6	0	38.3	35.8	60.3
Jan-93	10.3	85.9	29.1	200	10.3	0	83.6	29.1	45.1

Feb-93	7.9	34	-7.9	192.1	7.9	0	117.6	0	21.9
Mar-93	15.7	33.2	26.3	200	15.7	0	108	18.4	20.9
Apr-93	33	75.6	92.8	200	33	0	54	92.8	60.3
May-93	55.2	61.6	30.3	200	55.2	0	27	30.3	46.5
Jun-93	76.3	111.4	43	200	76.3	0	13.5	43	48.8
Jul-93	98.6	80.4	-15.5	184.5	98.6	0	6.7	45 0	25.6
Aug-93	86.2	54.4	-27.7	158.9	84	2.1	0	0	13.5
Sep-93	44	68.8	21.3	180.3	44	0	0	0	8.8
Oct-93	27.8	72.3	40.9	200	27.8	0	0	21.2	16.9
Nov-93	15.9	63.4	38.4	200	15.9	0	6.4	38.4	28.5
Dec-93	10.8	20	1	200	10.8	0	14.2	1	13.8
Jan-94	6.2	69	-6.2	193.8	6.2	0	83.2	0	6.7
Feb-94	8.1	18	-8.1	185.9	7.9	0.3	101.2	0	3.4
Mar-94	17.2	35.4	31.6	200	17.2	0	86.9	17.5	11.4
Apr-94	34.5	87.4	92	200	34.5	0	43.5	92	55.6
May-94	52.5	107.6	71.4	200	52.5	0	21.7	71.4	66.7
Jun-94	85.3	49.4	-27.5	172.5	85.3	0	10.9	0	33.1
Jul-94	98	54.7	-40.6	137.5	92.4	5.6	5.4	0	18.1
Aug-94	74.2	50	-21.3	122.8	67.6	6.7	0.4 0	0	10.1
-									
Sep-94	50.4	63	9.4	132.2	50.4	0	0	0	7
Oct-94	30.9	59.5	25.6	157.9	30.9	0	0	0	4.9
Nov-94	18.9	71	48.5	200	18.9	0	0	6.4	7.7
Dec-94	12.5	38.4	13.8	200	12.5	0	11	13.8	10.1
Jan-95	11.2	116.9	50.2	200	11.2	0	64.1	50.2	32
Feb-95	8.7	24.2	-6.2	193.8	8.7	0	85.7	0	14.8
Mar-95	20.4	3.4	13.9	200	20.4	0	54.7	7.7	11.4
Apr-95	27.4	93.2	82.4	200	27.4	0	33.8	82.4	51.1
May-95	56.9	76.2	32.4	200	56.9	0	16.9	32.4	43.4
Jun-95	90.2	79.8	-6	194	90.2	0	8.4	0	23.8
Jul-95	101.1	65.9		164.9 1	0.2	0.9	0	0	13.2
Aug-95	91.1	62.2	-32	138.5	85.5	5.6	0	0	8.1
Sep-95	46.5	45.4	-3.4	136.1	45.5	5.0	0	0	4.7
•						0	0		29.5
Oct-95	33.3	146.3	105.7	200	33.3			41.9	
Nov-95	13.8	142.2	90.7	200	13.8	0	33.1	90.7	61.1
Dec-95	9.1	34	4.4	200	9.1	0	53.2	4.4	30.8
Jan-96	8.6	66.8	2.2	200	8.6	0	108.8	2.2	16.6
Feb-96	9.8	43.8	10.8	200	9.8	0	131.6	10.8	13.9
Mar-96	15.5	62.8	43	200	15.5	0	134.7	43	29.5
Apr-96	28.1	97.8	126.2	200	28.1	0	73.6	126.2	81.8
May-96	53.2	93	72	200	53.2	0	36.8	72	79.2
Jun-96	85.9	149.2	74.3	200	85.9	0	18.4	74.3	81.9
Jul-96	88.7	112.4	27.2	200	88.7	0	9.2	27.2	56.5
Aug-96	83.5	52	-24.9	175.1	83.5	0	0	0	28
Sep-96	51.7	155.1	95.6	200	51.7	0	0	70.7	55.8
Oct-96	29.9	65.2	32	200	29.9	0	0	32	43.3
Nov-96	29.9 14.2	40.6	16.8	200	14.2	0	8.2	16.8	43.3 29.8
Dec-96	12.5	107.7	64.4	200	12.5	0	35.7	64.4	49.6

Jan-97	8.9	87.5	7.9	200	8.9	0	105.9	7.9	27.7
Feb-97	11.5	77.6	45.2	200	11.5	0	125.3	45.2	37.7
Mar-97	16.7	84	64.5	200	16.7	0	126.1	64.5	52.4
Apr-97	30.8	29.6	60.3	200	30.8	0	63	60.3	56.8
May-97	45.5	90.8	72.2	200	45.5	0	31.5	72.2	68.3
Jun-97	91.9	79.7	-0.4	199.6	91.9	0	15.8	0	35.9
Jul-97	92.7	58.3	-29.4	170.2	92.6	0.1	7.9	0	18.9
Aug-97	73.3	72.5	3.4	173.6	73.3	0	0	0	11.6
Sep-97	51.7	63.7	8.8	182.4	51.7	0	0	0	7.2
Oct-97	29.6	50.6	18.5	200	29.6	0	0	0.9	5
Nov-97	15.4	70.6	43.4	200	15.4	0	9	43.4	25.7
Dec-97	11.9	30.6	10	200	11.9	0	16.8	10	17.3
Jan-98	11.3	129.2	58.6	200	11.3	0	73.4	58.6	40.2
Feb-98	13.6	40.8	35.1	200	13.6	0	64.4	35.1	37.5
Mar-98	20	117.8	94.9	200	20	0	63	94.9	69.8
Apr-98	36	39.2	32.8	200	36	0	31.5	32.8	51.1
May-98	74.8	56.7	-5.2	194.8	74.8	0	15.8	0	27.4
Jun-98	83.8	57	-21.7	173.7	83.2	0.6	7.9	0	15.1
Jul-98	95	47.8	-41.7	137.5	89.5	5.5	0	0	8.5
Aug-98	85.1	96.4	6.5	143.9	85.1	0	0	0	7.9
Sep-98	55	44	-13.2	134.4	51.3	3.7	0	0	3.7
Oct-98	31.1	13.6	-13.2	122.2	25.1	6	0	0	5.7 1.4
Nov-98	17.6	45.2	25.5	147.7	17.6	0	0	0	2.5
Dec-98	17.0	4J.2 51	23.3	169.7	17.0	0	15.3	0	2.5 1.7
Jan-99	8.6	125.5	8	169.7	8.6	0	123.6	0	0.7
		40.6	ہ 48.2			0	123.6		
Feb-99	13.4			200	13.4			25.8	14.1
Mar-99	19.2	15.5	25.9	200	19.2	0	71.4	25.9	19.9
Apr-99	33.2	48.2	48.3	200	33.2	0	35.7	48.3	36.3
May-99	73.9	85	24.7	200	73.9	0	17.9	24.7	33.5
Jun-99	89.1	68.7	-14.9	185.1	89.1	0	8.9	0	18.1
Jul-99	113	63.4		L44.5 1	9.7	3.3	0	0	10.5
Aug-99	77.1	70.7	-9.9	137.3	74.3	2.7	0	0	7.2
Sep-99	55.4	71.6	12.7	150	55.4	0	0	0	5.4
Oct-99	29.9	64.1	31	181	29.9	0	0	0	4.1
Nov-99	19.2	102.4	78.1	200	19.2	0	0	59.1	35.1
Dec-99	11.9	55	23.2	200	11.9	0	18.4	23.2	28.1
Jan-00	9.6	46.1	5.9	200	9.6	0	48.5	5.9	16.7
Feb-00	12.1	55.7	29	200	12.1	0	61.9	29	23.8
Mar-00	23.7	40.6	41.6	200	23.7	0	35.3	41.6	34
Apr-00	32	67	49.3	200	32	0	17.7	49.3	44.1
May-00	62.5	164.9	103	200	62.5	0	8.8	103	80.1
Jun-00	83.8	219	133.1	200	83.8	0	0	133.1	113.4
Jul-00	87.1	91.2	-0.5	199.5	87.1	0	0	0	55.8
Aug-00	78.5	68.4	-13.5	186	78.5	0	0	0	29
Sep-00	51.7	110.6	53.4	200	51.7	0	0	39.4	38
Oct-00	33.7	29.6	-5.6	194.4	33.7	0	0	0	17.7
Nov-00	16.4	65.4	46.2	200	16.4	0	0	40.6	31.3

Dec-00	8.2	74.7	0.5	200	8.2	0	65.7	0.5	14.8
Jan-01	10.4	11.5	4.7	200	10.4	0	61.9	4.7	9.7
Feb-01	10.9	70.5	27.6	200	10.9	0	92.8	27.6	19.7
Mar-01	17.4	30.5	27.2	200	17.4	0	77.9	27.2	23.7
Apr-01	35.5	41	42.3	200	35.5	0	38.9	42.3	34.7
May-01	63.2	79.2	31.5	200	63.2	0	19.5	31.5	36
Jun-01	84.3	94.4	15.1	200	84.3	0	9.7	15.1	28.3
Jul-01	89.9	73.6	-10.2	189.8	89.9	0	0	0	15.5
Aug-01	91.7	48.6	-45.5	146.6	89.4	2.3	0	0	8.3
Sep-01	51.7	84.4	28.5	175.1	51.7	0	0	0	7.2
Oct-01	31.1	150.3	111.7	200	31.1	0	0	86.8	52.4
Nov-01	21.2	85.6	60.2	200	21.2	0	0	60.2	56.8
Dec-01	13.8	37	15.1	200	13.8	0	6.8	15.1	35.1
Jan-02	12.5	31.5	9.4	200	12.5	0	15.5	9.4	22.5
Feb-02	12.4	45.8	17.5	200	12.4	0	30.2	17.5	20.7
Mar-02	18.8	54.1	29.9	200	18.8	0	33.8	29.9	26.5
Apr-02	34.7	112.5	89.1	200	34.7	0	16.9	89.1	62.6
May-02	49.7	111.4	64.6	200	49.7	0	8.5	64.6	66.4
Jun-02	85.3	74.8	-5.8	194.2	85.3	0	0	0	34.1
Jul-02	110.3	66.6	-47 14		8.9	1.4	0	0	18.5
Aug-02	87.2	35	-54	108.5	73.4	13.9	0	0	9.3
Sep-02	62.7	52.5	-12.8	100.5	56.8	5.9	0	0	6.4
Oct-02	27.8	78	46.3	147.8	27.8	0	0	0	5.8
Nov-02	15.7	56	40.5 31.8	179.6	15.7	0	6.3	0	3.2
Dec-02	11.1	33.5	51.8 9	179.0	11.1	0	18.8	0	1.2
Jan-03	8	29	-8	188.0	7.5	0.5	47.8	0	0.2
Feb-03	8.5	50.5	-8 -7.1	174.7	7.9	0.3	47.8 96.8	0	0.2
Mar-03	8.5 17.9	38.6	-7.1 36	200	7.9 17.9	0.7	90.8 80.3	10.7	0.2 6.5
Apr-03						0			
•	30.4	33.6	41.6	200	30.4		40.2	41.6	25.2
May-03	54.8	95.6	56.1	200	54.8	0	20.1	56.1	44.6
Jun-03	82.7	77.3	0.8	200	82.7	0	10	0.8	24.1
Jul-03	96.2	38.8	-54.3	145.7	96.2	0	5	0	12.1
Aug-03	87.8	99.9	12.1	157.8	87.8	0	0	0	10.1
Sep-03	52.7	107.8	49.7	200	52.7	0	0	7.6	11.7
Oct-03	28.3	95.6	62.5	200	28.3	0	0	62.5	39.2
Nov-03	18.2	116.4	92.4	200	18.2	0	0	92.4	69.2
Dec-03	12.2	80.6	40.9	200	12.2	0	25.2	40.9	54.4
Jan-04	7.4	72.7	-7.4	192.6	7.4	0	97.9	0	26.1
Feb-04	10.8	20	11.8	200	10.8	0	95.1	4.3	15.5
Mar-04	20.3	110	101.5	200	20.3	0	79.3	101.5	62.3
Apr-04	32.8	71.8	75.1	200	32.8	0	39.7	75.1	70.3
May-04	57.6	117	73.4	200	57.6	0	19.8	73.4	75.9
Jun-04	77.7	57.5	-13.2	186.8	77.7	0	9.9	0	37.9
Jul-04	92.1	116.7	28.7	200	92.1	0	0	15.5	31.1
Aug-04	72.9	60.8	-15.1	184.9	72.9	0	0	0	15.7
Sep-04	57.1	39.7	-19.4	167	55.6	1.5	0	0	8.3
Oct-04	30.9	58.4	24.6	191.6	30.9	0	0	0	6.1

Nov-04	17.9	73.9	52.4	200	17.9	0	0	44	27.1
Dec-04	10	103.7	33.2	200	10	0	58.8	33.2	30.1
Jan-05	8.5	64	0.5	200	8.5	0	113.6	0.5	14.7
Feb-05	10.7	78.5	34.9	200	10.7	0	145.4	34.9	25.8
Mar-05	16.3	56.5	49.6	200	16.3	0	134.7	49.6	38.5
Apr-05	33.4	97.1	126.2	200	33.4	0	67.3	126.2	86.5
May-05	50.3	47.2	28.2	200	50.3	0	33.7	28.2	57.3
Jun-05	103.4	32.3	-55.9	144.1 1	3.4	0	16.8	0	29.1
Jul-05	106.9	141.7	36.1	180.3 1	6.9	0	8.4	0	20.8
Aug-05	88.3	148.3	61	200	88.3	0	0	41.2	34.9
Sep-05	58.5	76.6	14.2	200	58.5	0	0	14.2	24.7
Oct-05	32.7	51	15.8	200	32.7	0	0	15.8	20.9
Nov-05	18.2	122.9	98.6	200	18.2	0	0	98.6	64.5
Dec-05	10.2	97.3	32.8	200	10.2	0	52.6	32.8	47.3
Jan-06	13	109.9	77	200	13	0	69.3	77	64.5
Feb-06	10.5	141	47.6	200	10.5	0	150.4	47.6	56.3
Mar-06	19.1	62.9	76.3	200	19.1	0	115.9	76.3	67.4
Apr-06	35.3	95.1	113	200	35.3	0	57.9	113	93.9
May-06	61.3	106.1	68.4	200	61.3	0	29	68.4	84.1
Jun-06	86.4	53.9	-20.7	179.3	86.4	0	14.5	0	42.1
Jul-06	108.2	100.4	-5.6	174.3 1	7.6	0.6	7.2	0	24.7
Aug-06	83.5	52.6	-26.3	151.3	80.1	3.4	0	0	12.5
Sep-06	49.2	163.3	105.9	200	49.2	0	0	57.3	41.7
Oct-06	28.8	122.2	87.2	200	28.8	0	0	87.2	66.5
Nov-06	19.3	50.8	29	200	19.3	0	0	29	47.2
Dec-06	14.5	59.6	34.6	200	14.5	0	8.2	34.6	42
	506	901	358	2295	482	4	425	361	395

TABLE 1 : DETAILED WATER BALANCE CALCULATIONS- MT. HOPE ROAD, TOWN OF CALEDON

1 Climate Information

Precipitation	901 mm/a
Evapotranspiration (Actual)	482 mm/a
Water Surplus	419 mm/a

2 Infiltration Rates

Table 2 Approach - Infiltration factors

Soil: medium cobinations of clay and loam to	0.1			
Open sand loam	0.3			
Cover: Cultivated lands	0.1			
Total	0.6			
Infiltration (0.6 x 419)	251.4	mm/a		
Run-off (419-251)	168	mm/a		
Table 3 Approach - Typical Recharge Rates				
Coarse Sand and gravel	>250	mm/a		
Fine to medium sand	200-250	mm/a		
Silty sand to Sandy Silt	150 - 200	mm/a		
Silt	125 - 150	mm/a		
Clayey Silt	100- 125	mm/a		
Clay	<100	mm/a		
Site development area is underlain by sand and	gravel with s	ilty soils		
Based on the above, the recharge rate is typically	y	200)-250	mm/a
Run-off				mm/a
3 Pre-Development Property Statistics	ha	m2		
3 Pre-Development Property Statistics Paved Area	ha O		0	
			0 0	
Paved Area	0			
Paved Area Roof Area	0		0	
Paved Area Roof Area Landscape Area	0 0 41		0 410000	
Paved Area Roof Area Landscape Area Total	0 0 41 41	m2	0 410000	
Paved Area Roof Area Landscape Area Total 4 Post-Development Property Statistics	0 0 41 41 ha	m2	0 410000 410000	
Paved Area Roof Area Landscape Area Total 4 Post-Development Property Statistics Paved Area	0 0 41 41 ha 1.5	m2	0 410000 410000 15000	

5.0 Annual Pre-Development Water Balance

Land Use	Area (m2)	Precipitation (m3)	Evapotranspiration (m3)	Infiltration (m3)	Run-off (m3)
Paved Area	0	0	0	0	0
Roof Area	0	0	0	0	0
Landscape Area	410000	369410	197620	103074	68716
	410000	369410	197620	103074	68716

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

6. Annual Post-Development Water Balance

Land Use	Area (m2)	Precipitation (m3)	Evapotranspiration (m3)	Infiltration (m3)	Run-off (m3)
Paved Area	15000	13515	1352	0	12164
Roof Area	5000	4505	451	0	4055
Landscape Area	390000	351390	187980	98046	65364
	410000	369410	189782	98046	81582

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

7. Comparision of Pre- and Post -Development

Precipitation (m3)	Evapotraspiration (m3)	Infiltration (m3)	Run-off (m3)

Pre-Development	369410	197620	103074	68716
Post-Development	369410	189782	98046	81582
% Change			-5	19

8. Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration	103074
Volume of Post-Development Infiltration	98046
Deficit from Pre to Post Development Infiltration	5028
Percentage of Roof Runoff required to match the pre-development infiltration	124