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Palgrave Estate Subdivision Town of Caledon

Environmental and Engineering Summary Report

January 2020

MAEL Project 2018-951



Environmental and Engineering Summary Report

Palgrave Estate Subdivision Town of Caledon

For

Castlemore Corp.

January 2020

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1. INTRODUCTION

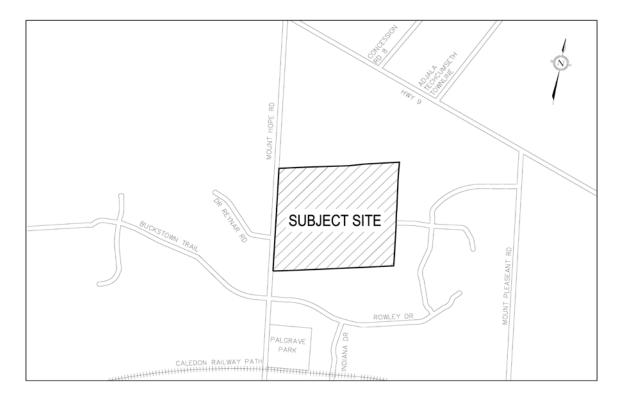
Masongsong Associates Engineering limited has been retained by Castlemore Corp. to prepare this Environmental and Engineering Summary Report in support of development application for Palgrave Estate subdivision, in the Town of Caledon, Regional Municipality of Peel.

This report is prepared to meet the requirements of Section 7.1.18.11 of the Town of Caledon Official Plan. As outlined in Town official plan, the purpose of this report is to integrate and summarize the environmental site investigations, mapping and analysis with the findings and recommendations of the preliminary engineering, stormwater management reports pertaining the subject development.

The report will provide a general assessment of the proposed development potential environmental impact and recommended mitigation measures. It will also highlight the recommendations outlined in site-specific reports to be used as design and construction guidelines for the subject estate residential development.

1.1 Background

The subject land is situated within Palgrave Estate Residential Community within Town Caledon, Regional Municipality of Peel. Refer to below for proposed site location plan:



It is located south of Highway No. 9 between Mount Hope Road and Mount Pleasant Road. The proposed development land is abutted by Mount Hope Road to the west and existing residentials and vacant lands to other directions. The total site area is approximately 41.32 ha (102.10 ac) in size and is legally described as part of Lot 28

Concession 8, Town of Caledon. The site is currently characterized mainly as agriculture land with variety of woodlands and vegetation communities.

Refer to Appendix-A for site legal survey map.

1.2 Topography and Landform

The subject site land is located within a physiographic region referred to as the Oak Rudge Moraine (ORM). Details pertaining the geology, hydrology and hydrogeology features of ORM is provided in Landform Conservation Assessment report prepared in support of the subject development.

From the topographic survey observation, the subject land terrain generally remains undisturbed at natural state. The existing landform feature slope ranges from moderate to steep. Existing woodlands, watercourses and vegetations concludes key natural features found throughout the subject site.

Refer to Appendix-A for site topographic map.

1.3 Proposed Development

Proposed development draft plan of subdivision consists of 29 estate lots and municipal roads namely Street A, B, and C. The draft plan also delineates areas to be designated as green spaces and woodlands.

The proposed density is calculated in Planning Justification Report, dated December 2019 as follows:

The density allowance for the subject site is 26 units per 40.5ha (100acres) as described in policy 7.1.6.4 of the Town of Caledon Official Plan. The subject site is approximately 41.32 ha, and the permitted density for the subject site would be 26 units (rounded from 26units \div 40.5 $ha \times 41.32ha = 26.5$ units). Policy 7.1.9.12 of the Town's Official Plans permits a density bonus of 1 unit over and above the density allotment in Section 7.1.6 will be given for each 4 hectares (9.9 acres) in the development suitably protected, managed or reforested by the applicant up to a maximum of 40 percent of the area of the subdivision plan. The proposed 13.16ha of the reforestation area would allow the subject development to have 3 units (rounded from 13.16 $haa \div 4ha = 3.29$ units) above the permitted density of 26 units. The total permitted density of the subject development is 29units (26units + 3units), and the proposed number of units of the subject development is 29units which meets the Town's Official Plan policies.

A significant portion of the subject land is identified as woodland (a Key Natural Heritage Feature) with 30 m minimum undisturbed buffer zone which results in reduction to the net developable area.

Major development works for the initial stage of proposed development will be construction of municipal roads, installation of watermain, utilities and ultimately construction of the proposed houses. Refer to Appendix-A for the proposed draft plan of subdivision.

2 ENVIRONMETAL MAPPING AND DRAFT PLAN

Environmental and draft Plans are prepared by Sirati & Partners Limited in support of the proposed development in order to meet the requirements of Section 7.1.18.2 of the Town of Caledon Official Plan.

As stated in the report, the subdivision land is located in a Category 2 Land Conservation Area. Therefore, the proposed development must conform to the requirements of Oak Ridges Moraine Conservation Plan (ORMCP). The development within a Category 2 Landform Conservation Area requires planning, design and construction practices that will minimize disturbance to landform character. These practices include:

- Maintaining significant landform features such as steep slopes, kames, kettles, ravines and ridges in their natural undisturbed form.
- Limiting the portion of the net developable area of the site that is disturbed to not more than 50% of the total area of the site
- Limiting the portion of the net developable area of the site that has impervious surfaces to not more than 20% of the total area of the site

The maps are also provided full size 1:1000 scale a separate stand-alone document entitled "Environmental Mapping and Draft Plan" which includes the following:

- Legal Boundary Survey & Air Photo Enlargement
- Topographic Map
- Slope Analysis Map
- Soil and Soil Drainage Classification Map
- Surface Hydrology Map
- Vegetation and Wildlife Ecology Map
- Environmental Summary Map
- Draft Subdivision Plan

The above listed maps outline the followings:

- Elevation contours in enough detail to show the basic topographic character of the site
- Analysis of the site by slope type
- Significant landform features including kames, kettles, ravines and ridges
- All water bodies including intermittent streams and ponds

Further details are provided in the Section 6 of this repot.

Refer to Appendix-A for the above listed Maps.

3 PRELIMINARY GEOTECHNICAL INVESTIGATION

Sirati & Partners Limited is tasked to undertake a preliminary geotechnical investigation in support of the proposed development application.

The purpose of this report is to obtain the subsurface conditions in order to provide preliminary recommendation for site architect, engineer and other consultants for the design of proposed roads, installation of utilities and construction of foundation structures.

The report generally meets followings Town of Caledon Official Plan requirements:

- Characterizes the soil and groundwater conditions encountered in the
- boreholes;
- Makes recommendations concerning the geotechnical design of
- septic tank and soil absorption field systems;
- Makes recommendations related to the design of structures and
- streets; and,
- Identifies any other geotechnical conclusions pertinent to facilities designs

The report identifies 300mm to 450mm topsoil thickness. It is noted in the report, that below the topsoil, fill/disturbed native material is encountered in all boreholes. The fill layer was found extending to depths varying between 0.8 m to 3 m. The report also describes the characteristic of thin layers of cohesive and cohesionless layer as part of the investigation.

Groundwater monitoring was conducted in the monitoring wells installed at the site in 2018 at shallow depth were observed to be dried during the groundwater monitoring in all the monitoring events. However, the groundwater levels measured in monitoring wells installed deeper in October 2019 ranged from 6.33 metre below ground surface (mbgs) to 14.85 mbgs at 283.73 mASL to 291.65 mASL elevation respectively.

The report recommends the following road minimum pavement structure design:

40 mm HL3 Asphaltic Concrete 50 mm HL8 Asphaltic Concrete 150 mm Granular 'A' 300 mm Granular 'B'

For further details, refer to Geotechnical Investigation report dated December 06, 2018, prepared by Sirati & Partners Limited.

Refer to Appendix-A for Boreholes Location map.

4 HYDROGEOLOGICAL INVESTIGATION

Hydrogeological investigation report is prepared by Sirati & Partners Limited in support of the proposed development application to meet the requirements outlined in Section 7.1.18.5 of Town official plan.

As stated in the report, the purposes of the investigation is to characterize the soil and groundwater conditions at the site, assess the pre- and post-construction water balances, identify the potential impacts of land development on local groundwater and surface water resources and/or natural environments, and provide mitigative options.

4.1 Scope of Work

Following summarize the scope work in this report:

- Review of the available background information
- Review of the available investigation report
- Site inspection
- Completion of Boreholes and monitoring wells
- Groundwater monitoring
- In-situ hydraulic conductivity tests
- Water balance assessment

4.2 Environmental Features

The report has identified the following site environmental features:

- The property is in Oak Ridge Moraine area
- Most of the site is located within Nottawasaga Valley watershed. A small portion of site at north-west corner is located within Humber River watershed.
- The site is in an area identified as Significant Groundwater Recharge Area (SGRAs)
- The site is located within Wellhead Protection Area (WHPA)-D and part of the property is in WHPA-C area.
- A small portion of the located in an area identified as Highly Vulnerable Aquifer (HVA)

4.3 Water Balance Assessment

Based on the Thornthwaite and Mather methodology (1957), the water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from ground or evapotranspiration by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage (Δ S).

The annual water budget can be expressed as:

 $P = ET + R + I + \Delta S$

Where:

P = Precipitation (mm/year)

ET = Evapotranspiration (mm/year)

R = Run-off (mm/year)

I = Infiltration (mm/year)

 ΔS = Change in groundwater storage (taken as zero) (mm/year)

A summary of water balance is provided below:

- 1) Without implementation of mitigation measures, there is a net increase in run-off at the Site of about 12,866 m3/annum (or 19% increase), from 68,716 m3/annum to 81,582 m3/annum. This increase is a result of the development of the Site with more impervious areas such as roof and paved areas, and reduction in pervious areas.
- 2) Without implementation of mitigation measures, there is a net deficit of about 5,028 m3 /annum (or 5 % decrease) in the post-development infiltration from 103,074 m3 to 98,046 m3 on a yearly basis.
- 3) There is a volume of 4,055 m3/annum collected from the roof area, which can be used for the enhanced infiltration for the purpose of implementing the Low Impact Development (LID) measures. However, it is not enough to compensate for the deficit in infiltration.

4.4 Report Conclusions and Recommendations

The report states that, based on the hydrogeological investigation conducted on the Site, the following conclusions are presented:

- The Site geographically falls within the Innisfil Creek subwatershed under the jurisdiction of Nottawasaga Valley Conservation Authority (NVCA).
- As per the Oak Ridges Moraine Conservation Plan, 2017, the Subject Property falls within the Palgrave Estate Residential Community (a component of Countryside Area) land use designation area.
- The Site lies within the physiographic region termed as Oak Ridges Moraine and is located in an area characterized by coarse-grained ice-contact glaciolacustrine sediments consisting of sand, gravelly sand and gravel.
- The Site is located within the Palgrave municipal supply well No. 3 wellhead protection area, within a Significant Groundwater Recharge Area (SGRA) and partly in a Highly Vulnerable Aquifer (HVA) area.
- The soil stratigraphy of the Site as revealed in the boreholes generally consisted of topsoil and fill materials, underlain by native soils predominantly comprised of

cohesionless soils, locally with cohesive soils. No bedrock was encountered at the maximum explored depth of 18.3 mbgs.

- The groundwater levels measured in the monitoring wells at the Site ranged from 6.33 mbgs to 14.85 mbgs and elevations ranged from 283.79 mASL to 291.65 mASL. The groundwater flow was inferred to be generally in a northerly direction.
- The hydraulic conductivity estimated for the screened soils E ranged from 1.23 x 10-4 m/s to $3.28 \times 10-7$ m/s, with a geometric average value of $1.15 \times 10-5$ m/s, which is in the typical range of hydraulic conductivity for sand and gravelly sand, as observed in the boreholes.
- Given the relatively deep groundwater levels found at the Site, construction dewatering (short-term or long-term) will not be required for the proposed development.
- A preliminary water balance assessment indicated that an infiltration deficit of approximately 5,028 m3/annum (about 13 m3 /day) will occur due to the proposed development.

Based on the findings of this hydrogeological investigation, the following measures would be considered and recommended to protect and preserve the SGRAs and HVAs,

- Incompatible land uses such as storage of chemicals and/or liquids should be avoided and directed away from the SGRAs
- Since the proposed development is a major development (> 500 m²) within the SGRAs, an Infiltration Management Plan that demonstrates pre-development recharge rates will be maintained, may be a requirement.
- Low-impact development (LID) measures would be designed and implemented to maintain or improve the post-development infiltration and/or groundwater recharge conditions. The measures may include, but are not limited to the following:
 - -Use of infiltration trenches or bio-swales at selected areas to maximize the infiltration.
 - Provision of pervious road or parking areas to enhance infiltration. Provision of an extra thickness of topsoil at the Site (approximately 0.3 m) on open areas to promote water storage in surficial soil and infiltration.
 - Provision of gradual slopes to open areas and back-yards in order to allow extra time for surficial run-off to infiltrate into the topsoil.
- It would be necessary to promote awareness of the importance of SGRAs and HVAs by means of sign boards explaining the linkage between surface activities and their impact on groundwater quality and quantity.
- A salt management plan may be considered to be developed and implemented.

- As the Subject Lands fall within the areas of high aquifer vulnerability, the following uses are prohibited with respect to land in Areas of High Aquifer Vulnerability.
 - Generation and storage of hazardous waste or liquid industrial waste;
 - Waste disposal sites and facilities, organic soil conditioning sites, and snow storage and disposal facilities;
 - Underground and above-ground storage tanks that are not equipped with approved secondary containment device; and,
 - Storage of a contaminant listed in Schedule 3 (Severely Toxic Contaminants) to Regulation 347 of the Revised Regulations of Ontario, 1990.

For further details, refence should be made to Sirati & Partners Hydrogeological Investigation report, dated November 22, 2019.

5 SERVICING AND STORMWATER MANAGEMENT

Servicing and Stormwater Management report is prepared by Masongsong Associates Engineering Limited to meet the requirements of Sections 7.1.18.7 & 7.1.18.8 of Town official plan. The report describes the proposed development stormwater management scheme, water distribution, sanitary servicing and utility requirements.

5.1 Water Distribution

The subject site is within Peel Region Pressure District 8A water distribution system. A 200mm diameter PVC watermain is proposed along the proposed municipal roads namely Street A, B, and C. and service connections to proposed lots will be made from this main. The proposed watermain requires looping; therefore, a 150mm watermain extension beyond the limit of proposed development is proposed be installed to connect to existing 150mm watermain in McGuire Trail in order to form a strong looped system.

Hydraulic analysis of proposed water distribution system is conducted using EPANET 2 modeling software to ensure the system delivers desired pressures and flows for the proposed development under various demand scenarios.

The summary of analysis result is provided in the following Table:

No	Scenarios	EPANET	Region
		Results	Criteria
1	Max. pressure during min. hour demand (kpa)	629	< 690 (Ok)
2	Min. pressure during max. hour demand (kpa)	570	> 275 (OK)
3	Min. pressure during max. day demand + fire (kpa)	242	> 140 (OK)

The above summary of EPANET modeling result shows that proposed watermain system meets Region standard criteria for required pressures for noted scenarios.

5.2 Sanitary Servicing

As noted, there is no municipal sanitary sewer system available for this development. Typically, due to the nature/type of proposed estate lots, septic system for individual lot is considered feasible alternative for sanitary servicing. Proposed septic system typical size and approximate location of disposal area / leaching bed is show on proposed development engineering plans. The actual size and location of the system (in consideration with the individual lot landform constraints) will be detailed during individual site grading and siting plan preparation stages.

The design of septic system is to be coordinated with site mechanical consultant during detailed design stages. The nitrate loading analysis presented in Nitrate Impact Assessment report for private on-site sewage servicing (septic system) shows added

nitrate concentration in groundwater. However, the report concludes that the adverse impact minor or negligible.

Refer to Drawing GP1 enclosed in Appendix-A for illustration of proposed septic system and water servicing.

5.3 Stormwater Management - Quantity

Proposed municipal roads are considered major change to existing landform yet in comparison to overall site area it accounts only for 6.5% of development area. In addition, disturbance to existing landform and minor increase in hard surface (driveways and roof) within the proposed lots will not result in significant increase to post-development runoffs. This is due to size of disturbed area in comparison to overall size of the proposed lots which will largely remain unchanged at pre-development condition as vegetated surface with minimum grading changes.

In the context of the proposed estate lot development, there will be no significant increase in post-development peak runoff; therefore, design of new end-of-pipe stormwater management facility/feature is not feasible or recommended.

Nonetheless, as part of the Low Impact Development (LID) measures, Enhanced Grass Swale is proposed as lot-level and conveyance controls for attenuation of stormwater runoff from proposed roads which helps in peak runoff reduction. It also conveys the runoff to existing watercourse which qualifies as drainage receiving system.

In addition, the Low Impact Development measures proposed as lot-level infiltration-based controls for each individual lot (as outlined in MECP Stormwater Management Planning and Design Manual) to include the followings:

- reduced grading to allow greater ponding of stormwater and natural infiltration;
- directing roof leaders to rear yard ponding areas, soakaway pits, or to cisterns or rain barrels;
- sump pumping foundation drains to rear yard ponding areas;
- infiltration trenches;
- grassed swales;
- pervious pipe systems;
- · vegetated filter strips; and
- stream and valley corridor buffer strips.

Design and Implementation of the above lot-level quantity controls will be applicable during detailed individual lot grading and siting plans preparation.

Efforts should be made during road and lot design/construction stages so that the existing overall drainage pattern is to be maintained at original conditions to the extent possible).

Furthermore, from the topographical survey contours observation, it is evident that the proposed development receives external drainage from lands to the north of the subject site. Drainage from this external area which is estimated to be 15.10 ha is tributary to existing watercourse that traverses the subject site.

As part the proposed development, Street 'A' crosses the existing watercourse and therefore, a drainage culvert is proposed to safely convey flows to downstream receiving system. Proposed culvert is sized to convey 25-year event flows. The total flow to culvert is calculated to be 1.03 m³/s and the proposed culvert is sized to covey the flows at about 70 % full flow capacity. Proposed culvert full flows capacity is 1.51 m³/s larger than the required flow of 1.03 m³/s.

A 20.00m wide watercourse with 0.30m depth will have sufficient conveyance capacity to convey post-development flows from downstream of proposed culvert to existing conveyance system beyond the limits of the proposed development. The flows from 25-year event tributary to downstream segment of watercourse is calculated to be 1.26 m³/s and the watercourse conveyance capacity is 1.31 m³/s.

5.4 Stormwater Management-Quality

Proposed Enhanced Grass Swale as roadside ditches not only convey and attenuate stormwater runoff it also provides effective quality control functionality. To this end, the proposed enhanced grass swale along proposed municipal roads provide quality treatment for stormwater runoff from roads. Road drainage is directed to enhanced grass swale by providing gutter outlets at certain interval which will convey road drainage to enhanced grass swale. Detailed design of Enhanced Grass Swale will be provided at detailed design stages.

Refer to Drawing DR1 & Dr2 enclosed in Appendix-A for conceptual grading plan.

5.5 Grading

Effort is made to preserves the existing landform and grades to the extent possible. To achieve this, proposed lot grading boundaries and corner grades matches existing grades minimizing any grading disturbances along proposed lot boundaries. Typically, main grading will be within/around the proposed house envelope and driveways. Typical house envelop and septic system for individual lots are shown within areas where slopes are minimum to avoid major landform alteration. However, more detail grating will require coordination with site architect at detailed lot grading design stages and the house can be designed/tailored to better fit to existing landform to minimize grading works.

Refer to Drawing GR1 enclosed in Appendix-A for conceptual grading plan.

5.6 Report Conclusions and Recommendations

This report has demonstrated that the subject development can be serviced by existing and proposed servicing infrastructures. More specifically, servicing and SWM design analysis for proposed development is summarized as follows:

- A 200mm watermain is proposed to service the subject development. A 150mm proposed watermain is further extended east to connect to existing 150mm main on McGuire Trail to form a strong looped system.
- All proposed lots are provided with septic system. Design of septic system will be finalized during individual lot detailed design stages.
- No quantity control is required for subject site as the proposed development does not significantly changes the quantitative nature of existing drainage scheme. Nonetheless, as part of Low Impact Development (LID) measures, enhanced grass swale is proposed to reduce peak post-development runoff.
- Enhanced Grass Swale (as roadside ditch) is proposed for treatment of road derange. Similar LID measures are to be considered for individual lots during detailed lot siting and grading stages.
- Water balance essential components do not experience significant changes due to proposed development. However, as indicated in the site Hydrogeological Investigation report, a volume of 4,055 m³/annum collected from the roof area, which can be used for the enhanced infiltration for the purpose of implementing the Low Impact Development (LID) measures. The implementation of LID for the purpose water balance is practical only at site plan stage for individual lots.

For further details, refer to Masongsong Associates Engineering Limited, Servicing and SWM report, dated October 2019.

6 LANDFORM CONSERVATION ASSESSMENT

Landform Conservation assessment report dated December 2019 is prepared by Crozier Consulting Engineers. As stated, the subject site is located within Oak Ridge Moraine and as per Town of Caledon Official plan Section 6.6.3.1.3 the subject development is required to conform to the policies of the Oak Ridge Moraine Conservation Plan (ORMCP). The report is prepared to demonstrate how proposed development strategies meet the requirements of the Town and ORMCP.

As stated in the report, landform conservation is the protection of the unique visual qualities of the area and environmentally sensitive features. It is an approach that encourages planning, design and construction practices which:

- minimize disruption to landform and landscape
- minimize grading and changes to topography
- encourage clustering development on less sensitive areas of the site

6.1 Report Objectives

A portion of the Subject Development is located in the Category 2 Landform Conservation Area (Appendix A1). The development application must therefore conform to the requirements of Section 30 of the ORMCP (2017). Section 30(6) of the ORMCP (2017) states that development within a Category 2 Landform Conservation Area requires planning, design and construction practices that will minimize disturbance to landform character. These practices include:

- Maintaining significant landform features such as steep slopes, kames, kettles, ravines and ridges in their natural undisturbed form
- Limiting the portion of the net developable area of the site that is disturbed to not more than 50% of the total area of the site
- Limiting the portion of the net developable area of the site that has impervious surfaces to not more than 20% of the total area of the site.

The following maps (enclosed in Appendix-A) are prepared as part of landform conservation assessment studies:

- Legal Boundary Survey & Air Photo Enlargement
- Topographic Map
- Slope Analysis Map
- Soil and Soil Drainage Classification Map
- Surface Hydrology Map
- Vegetation and Wildlife Ecology Map
- Environmental Summary Map

6.2 Report Conclusion & Recommendations

Based on the information provided in this report, it is concluded that the proposed residential development aims to minimize disruption to the landform character and the natural heritage lands. The majority of the area that falls within the Oak Ridges Moraine Landform Conservation Areas Category 2, containing slopes greater that 15%, will not be developed. All of the key natural heritage features and steep sloped lands will be protected and preserved because the development will occur outside the limits of the natural heritage and landform features. The Subject Development meets the requirements outlined in the ORMCP (2017) and in the Town of Caledon's Official Plan (2018).

For further details refer to Crozier Consulting Engineers Landform Conservation Assessment report dated December 2019.

7 NATURAL HERITAGE EVALUATION

Natural Heritage Evaluation report dated December 2019 is prepared by Beacon Environmental Limited. As stated in the report, the purpose of this NHE is to determine the location of any Key Natural Heritage Features (KNHFs) and Key Hydrologic Features (KHFs) on and within the 120m area of influence of the subject property. The NHE is used to determine the limits of the proposed development so as to not adversely affect the ecological integrity of the ORMCP area. This NHE is prepared to ensure conformity with the applicable natural heritage policies of the ORMCP, Town of Caledon and Peel Region Official Plans as well the Nottawasaga Valley Conservation Authority (NVCA) and Endangered Species Act (ESA). This NHE also provides recommendations for appropriate mitigation measures in order to reduce potential impacts on KNHFs and KHFs.

Refer to Figure 2 & 3 showing the subject development land existing conditions and proposed development concept.

7.1 Policy Review

The following policy documents were reviewed with respect to natural heritage on the subject property in order to determine the applicable policy framework:

- Oak Ridges Moraine Conservation Plan (2017)
- Regional Municipality of Peel Official Plan (Office Consolidation 2018)
- Town of Caledon Official Plan (Office Consolidation 2018)
- Nottawasaga Valley Conservation Authority Regulations (2006)
- Endangered Species Act (2007)

7.2 Potential Impacts and Mitigation

The report has identified potential impact due to proposed development and they are listed as follows:

- Removal of Vegetation
- Loss of Agricultural Habitats
- Increase in Impervious Surfaces
- Sediment and Erosion Control
- Noise and Light Effects on Wildlife
- Human Encroachment and their Companion Animals

7.3 Headwater Drainage Feature Assessment

A watercourse (the central drainage feature) is mapped through the provincial Land Information Ontario (LIO) system, traversing the subject property. Field studies revealed there is no feature present at this location. During the time of field investigations this portion of the subject property was found to be completely dry. Due to surrounding topography and evidence of erosion, it is believed that this central depression may convey overland flows contributed by rain events and has therefore been depicted as an

ephemeral drainage feature on Figure 2. However, there is no indication of an intermittent tributary or headwater drainage feature present.

According to the Evaluation, Classification and Management of Headwater Drainage Features Guidelines (Toronto and Region Conservation Area and Credit Valley Conservation 2014), no management is required.

7.4 Mitigation Measures

The report details the anticipated impacts of the proposed development and identify mitigation and compensation measures to be utilized to minimize effects of the project.

The proposed development is situated within an area that has been transforming from an agricultural landscape to an estate residential landscape, which inevitably reduces natural heritage functions of any particular site within that larger landscape area. However, these kinds of landscape level changes cannot be wholly mitigated on a site-by-site basis, and a shift in the natural heritage values towards an urban tolerant system will continue to occur. Following are list of recommended Mitigation Measures proposed:

- Mitigation by Design
- Feature Buffers
- Forestry Management Area and Restoration
- Fencing Installation
- Low Impact Development Measures
- Salt Management Plan
- Land Dedication
- Timing of Vegetation Removal
- Sediment and Erosion Control Plan
- Tree Inventory and Protection Plan

7.5 Report Conclusions and Recommendations

Beacon has reviewed the existing natural heritage policies as they pertain to the subject property. A field program was developed to understand the site conditions, context and function with respect to natural heritage features. The proposed development of the subject property demonstrates compliance with the relevant policies of the ORMCP, and those particularly pertaining to the Palgrave Estates Residential Community. Staff developed a field program based on input from the NVCA and researched the terrestrial and aquatic conditions on the subject property.

A Significant Woodland was identified on the subject property based on the ORMCP Technical Paper Series and corresponds to the Town's EZ1 designation as well as the NVCA staked line. A 30 m MVPZ will be applied along the entirety of the dripline, and a robust reforestation plan will be implemented beyond this. A minor encroachment of

0.15 ha is proposed to support the extension of Street A and will be offset by a restoration area proposed through the reforestation plan.

A number of mitigation measures have been provided through this report and should be adhered to in order to minimize impacts of this proposed development on the natural system.

For further detail, refer to Beacon Environmental Natural Heritage Evaluation report dated December 2019.

8 PHASE ONE ENVIRONMETAL SITE ASSESSMENT

Sirati & Partners was tasked to prepare Phase 1 Environmental Assessment report in support of development application plan. It is understood that the site will be developed as a residential subdivision with residential houses with one (1) level of basement. The report is prepared in general accordance with O. Reg. 153/04 as amended.

According to the report, two (2) areas of potential environmental concern (APECs) is identified on the subject Property. A summary of the APECs and associated contaminants of potential concern (COPCs) is summarized as follows:

- APEC-1 Importation of Fill Material of Unknown Quality
- APEC-2- Pesticides (including Herbicides, Fungicides and Anti-Fouling Agents)
 Manufacturing, Processing, Bulk Storage and Large-Scale Applications

Phase 2 ESA is required to investigate the above noted potential environmental concern which may have resulted in adverse impact to the environmental condition of the subject site.

According to O. Reg. 153/04, a Record of Site Condition (RSC) is required for the proposed municipal roads ROW land conveyance to the local municipality.

8.1 Report Conclusions & Recommendations

According to O. Reg. 153/04, an RSC would not be required for the proposed residential development. However, if the development involves that land conveyance to the local municipality is due, an RSC for the conveyed property may be required.

Based upon the review and evaluation of the information gathered from the Phase One ESA, areas of potential environmental concern were identified on the Phase One Property, which were associated with the potentially contaminating activities identified on the Phase One Property. Therefore, an intrusive investigation in the form of Phase Two ESA shall be conducted to confirm the absence or presence of the actual contamination.

As a result, a Record of Site Condition cannot be filed based upon a Phase One ESA alone.

As discussed above, in support of filing an RSC, a Phase Two Environmental Site Assessment will be required to investigate the areas of potential environmental concern identified in the Phase One Property, which may have resulted in adverse impact to the environmental condition of the Phase One Property.

For further detail, refer to Sirati & Partners Phase One Environmental Site assessment repot dated June 25, 2019.

9 NITRATE IMPACT ASSESSMENT

Sirati & Partners (SIRATI) was retained to conduct a Nitrate Impact Assessment study. This study was performed in conformance with the Town of Caledon Official Plan Secondary Plan Policies Section 7.1.8.3.

The purpose of the report is to assess the termination probability of contaminants on wells in nearby properties by septic system leachate or other sources of contaminants likely to be caused by the proposed development.

9.1 Nitrate Concentrations and Impact

As part of water quality assessment, nitrate was analyzed in three (3) groundwater samples taken from BH/MW19-1A, BH/MW19-2 and BH/MW19-3A. The results are presented in Table 7-2 below.

Nitrate Concentration in Groundwater from On-Site Wells.

Monitoring Well	Nitrate as N (mg/L)	ODWQS Standard (mg/L)	
BH/MW19-1A	21.5	10	
BH/MW19-2	29.3	10	
BH/MW19-3A	27.0	10	

Chemical analysis indicates, nitrate concentrations in the analyzed groundwater samples were found to be raised, which are above the ODWQS standards.

The report has evaluated the potential groundwater nitrate concentration impact on surface water, on-site water well, off-site wells and water supply aquifer and found no adverse effect.

9.2 Report Conclusion and Recommendation

Based on the information obtained from previous investigations and the results of nitrate impact assessment conducted at the Site, the following findings or conclusions can be presented:

- Groundwater quality assessment indicated that groundwater samples may not meet the Ontario Drinking Water Quality Standards (ODWQS) due to the elevated concentrations of aluminum, iron, manganese, total hardness and turbidity and nitrate.
- The background nitrate concentrations were found to exceed the ODWQS standard of 10 mg/L for nitrate. The elevated background nitrate concentrations may have resulted from the farming activities occurring at the development area and/or from the operations of private septic systems on the upgradient

properties, which might be minimized after the residential development when no farming activities will take place.

- A water balance analysis indicated that there is a net water surplus of 419 mm/annum (or about 0.4 m/year) occurring at the Site, which can either infiltrate into subsurface or go as run-off.
- On-site nitrate loading calculation have resulted that, a total load of 1,160,000 mg/day of nitrate will be added on the proposed development lots, which will result in an added concentration of 3.5 mg/L for nitrate in groundwater in the development area.
- Based on the nitration dilution assessment, an extra 2.6 mg/L of nitrate will be added to the local groundwater at the site property boundary.
- The local water supply aquifer is hydrogeologicaly isolated from the naturally elevated nitrate concentrations across the Site.
- The nitrate concentration levels will decline after the development, within a reasonable time frame, as the application of fertilizers for agricultural purposes will completely be eliminated with the development of the Subject Property for residential purposes.
- Given the site features and the site-specific soil and groundwater conditions, the impact on the local surface water and/or groundwater or use of the water wells due to the proposed development will be minor or negligible.

For further detail, refer to Sirati & Partners Nitrate Impact Assessment report dated November 25, 2019.

10 ARCHAELOGICAL ASSESSMENT

Lincoln Environmental Consulting Corp. (LEC) was retained by Palgrave Estates to complete a Stage 1-2 archaeological assessment.

As stated in the report, Stage 1 & 2 assessment was conducted from May 28th to May 31st, 2019 under archaeological consulting license P344 issued to Derek Lincoln, MA, of LEC by the MTCS. No archaeological resources were identified during the Stage 2 archaeological assessment of the study area, and as such no further archaeological assessment of the property is recommended.

References

Design Plan Services. Planning Rationale Report dated December 2019.

C.F.Crozier & Associates. Landform Conservation Assessment Report dated December 2019.

Sirati & Partners Consultants Limited. Preliminary Geotechnical Investigation report dated December 06, 2019.

Sirati & Partners Consultants Limited. Hydrogeological Investigation report dated November 22, 2019.

Sirati & Partners Consultants Limited. Hydrogeological Investigation report dated November 22, 2019.

Masongsong Associates Engineering Limited. Preliminary Engineering and Stormwater Management Report dated October 2019.

Beacon Environmental. Natural Heritage Evaluation report dated December 2019.

Sirati & Partners Consultants Limited. Phase One Environmental Site Assessment report dated June 25, 2019.

Sirati & Partners Consultants Limited. Nitrate Impact Assessment report dated November 25, 2019.

Lincoln Environmental Consulting Corp. Stage 1 & 2 Archaeological Assessment report dated June 2019.

Ministry of Environment. 2003. Stormwater Management Practices Planning and Design Manual. March 2003.

Region of Peel. 2010. Public Works Design, Specifications and Procedures Manual, Linear Infrastructure, Watermain Design Criteria. Revised June 2010.

Toronto and Region Conservation Authority and Credit Valley Conservation. 2014. Evaluation, Classification and Management of Headwater Drainage Features Guidelines. January 2014.

Town of Caledon. 2018. Town of Caledon Official Plan. April 2018 Consolidation.

Town of Caledon. 2009. Development Standards, Policies & Guidelines, Town of Caledon. Prepared by the Town of Caledon Public Works & Engineering Department, January 2009.

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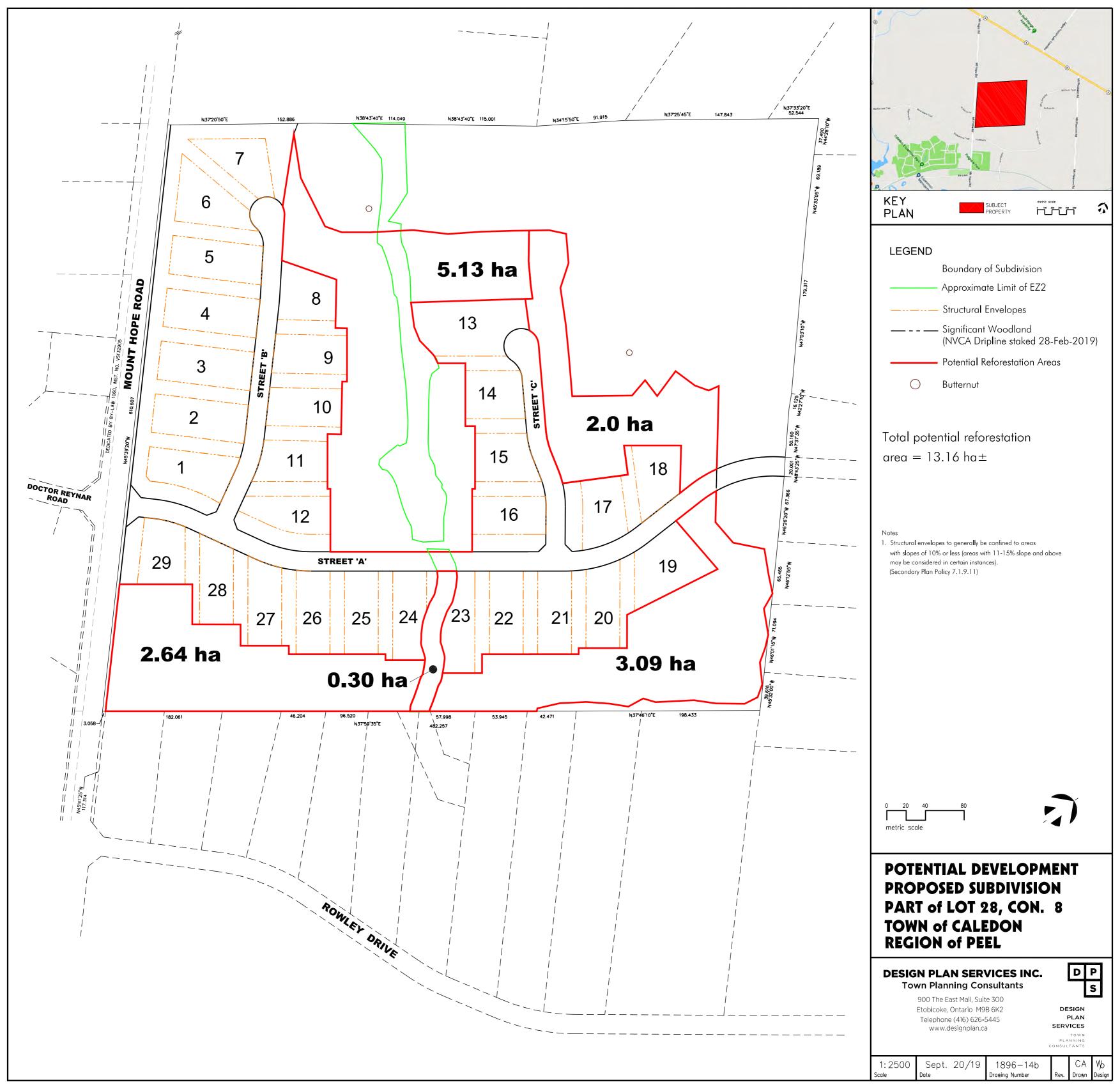
The supporting studies/plans illustrates that the proposed development conforms to Provincial Policy /Plans. It also meets the objectives of Region of Peel and Town of Caledon official plans.

We trust you will find this submission complete and in order. Should you have any questions, please contact the undersigned.

Respectfully Submitted,
MASONGSONG ASSOCIATES ENGINEERING LIMITED

Mansoor Nooristani, C.E.T. Senior Project Technologist Lucila Ensuncho, M.A.Sc., P.Eng Principal

Appendix A











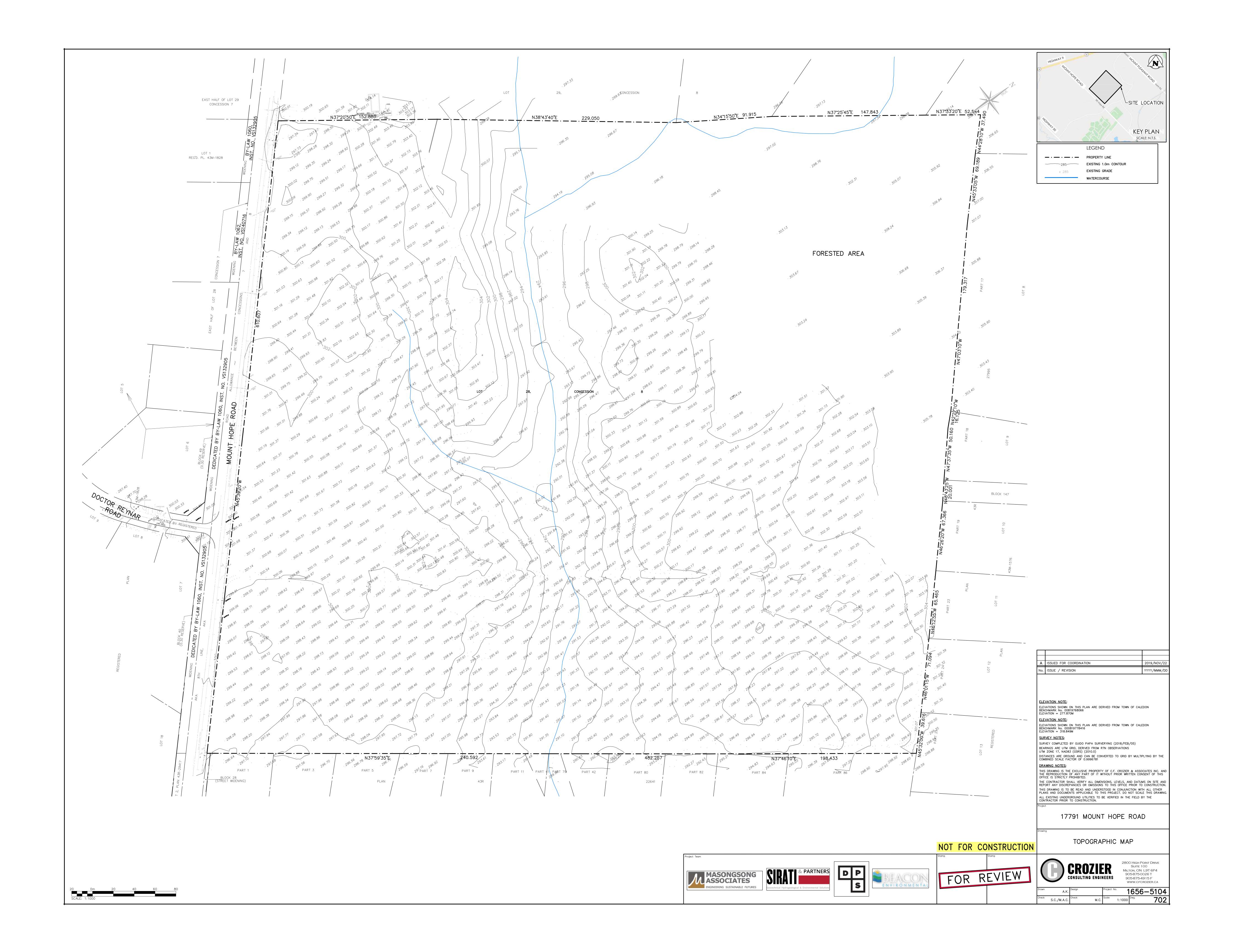


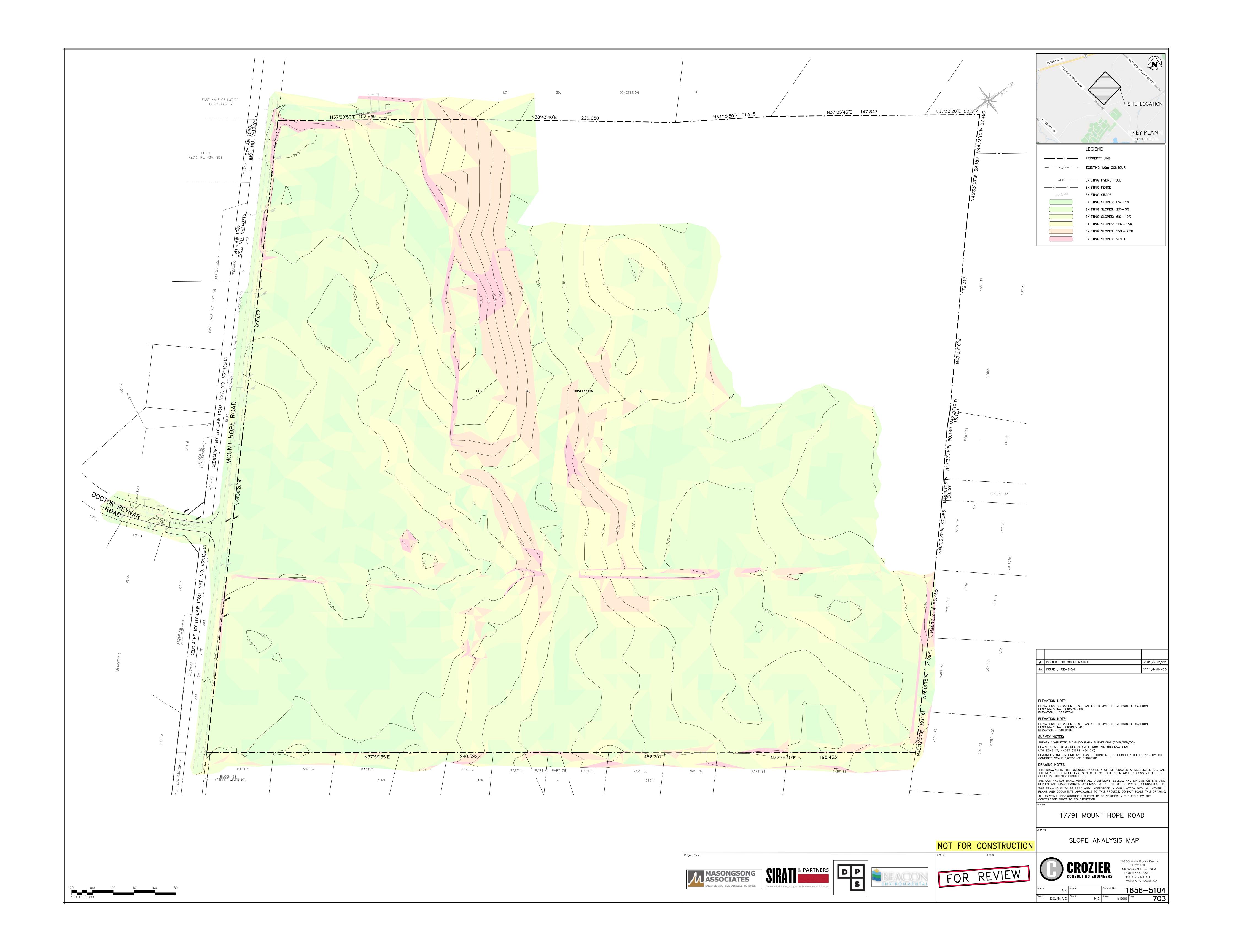


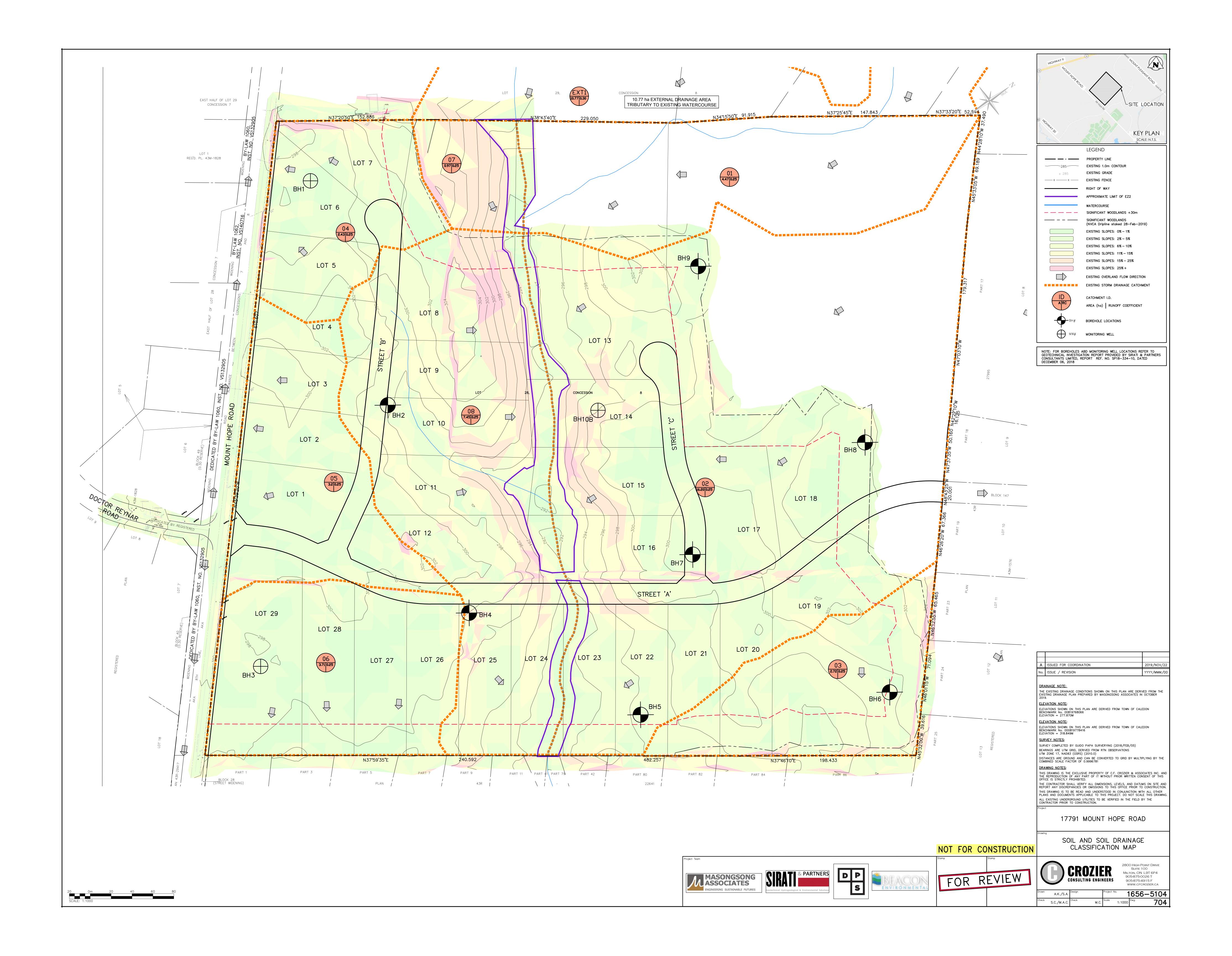
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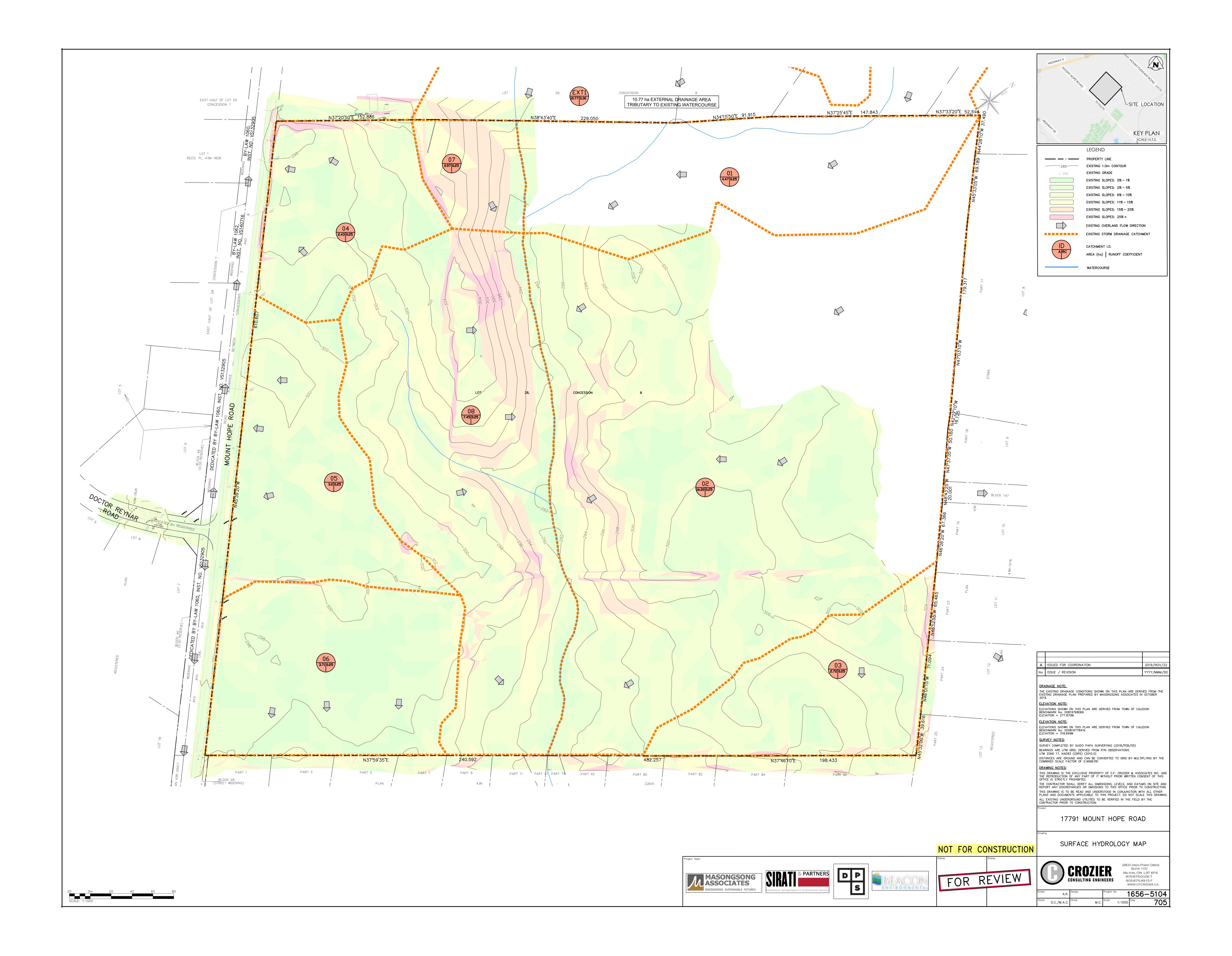
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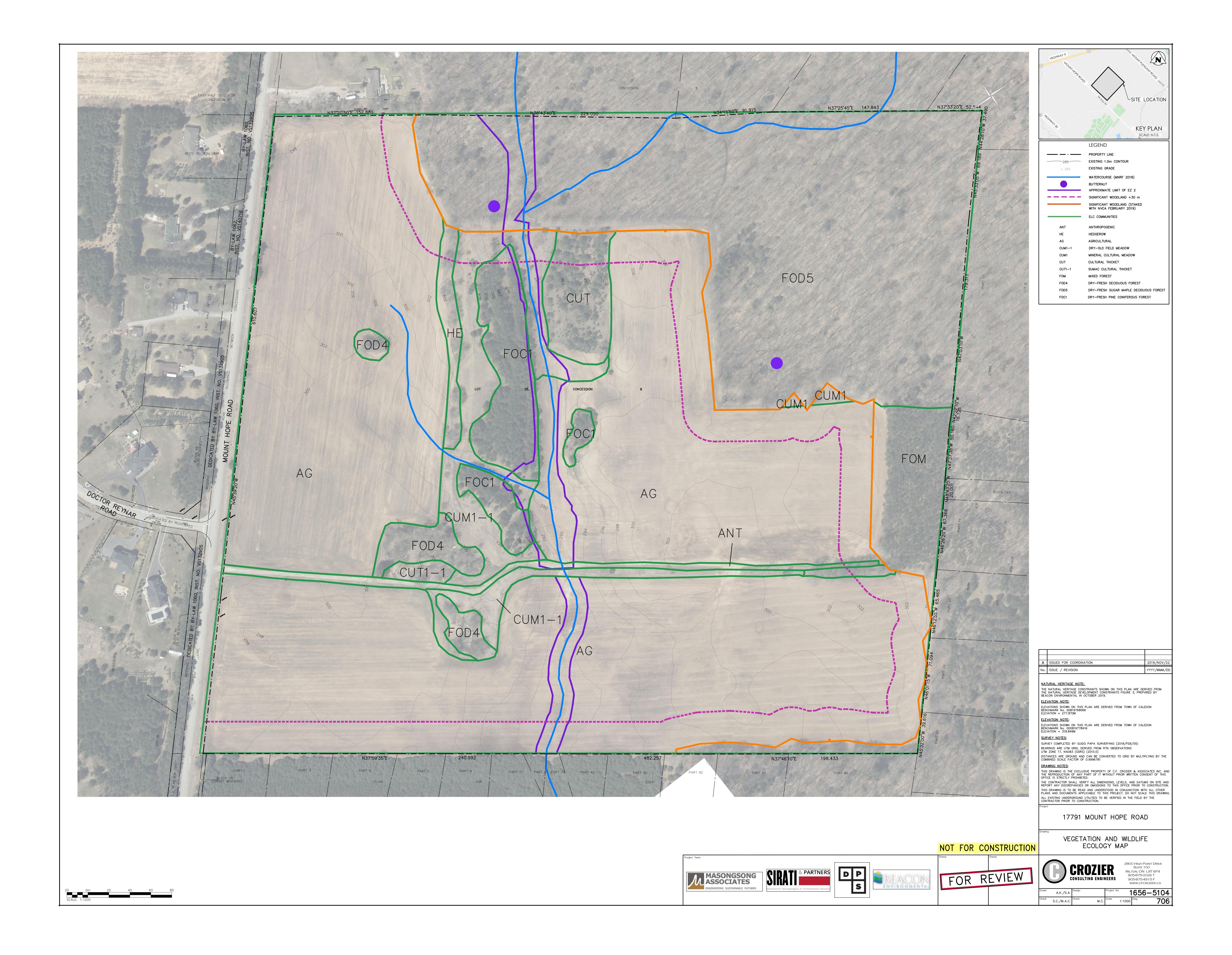
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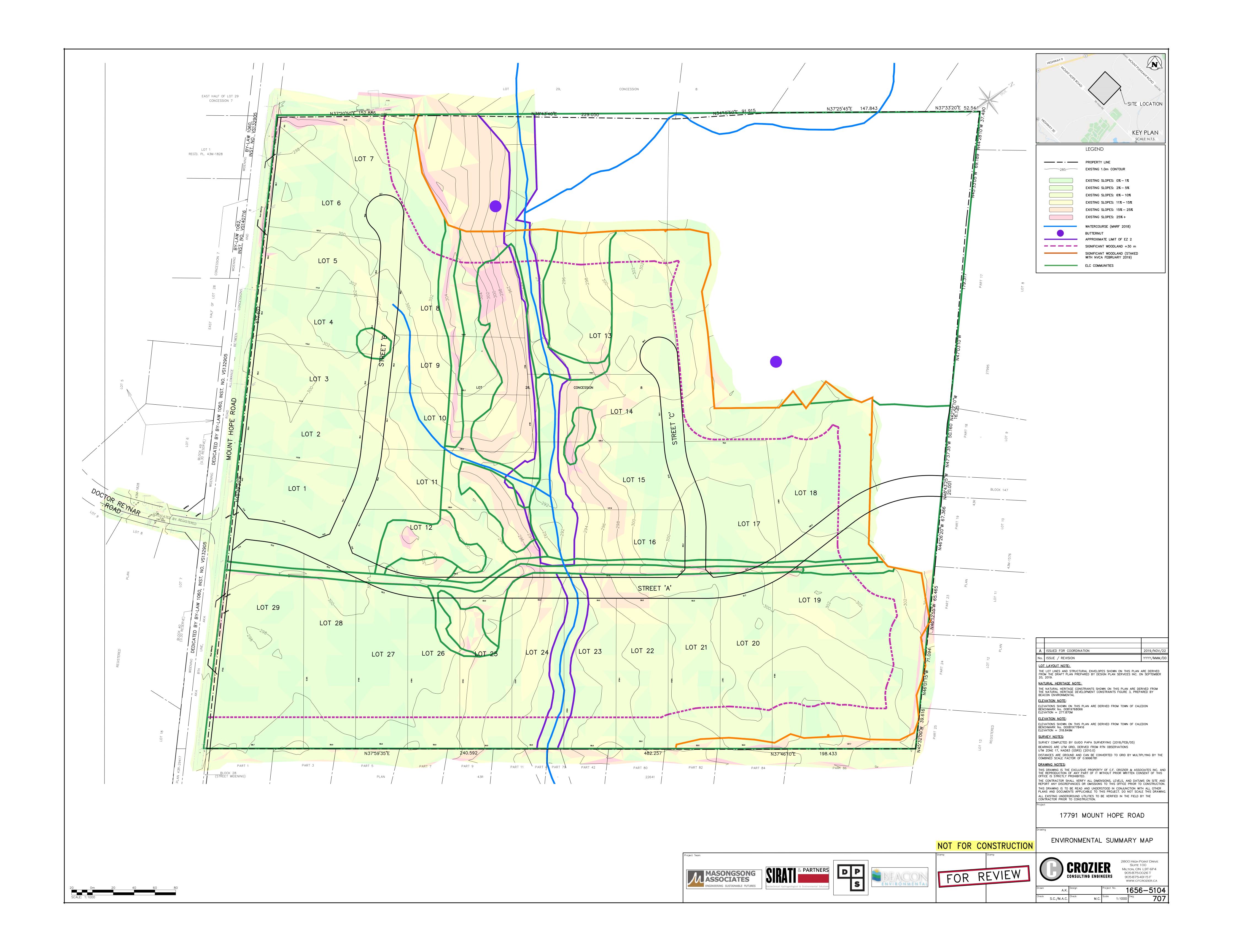


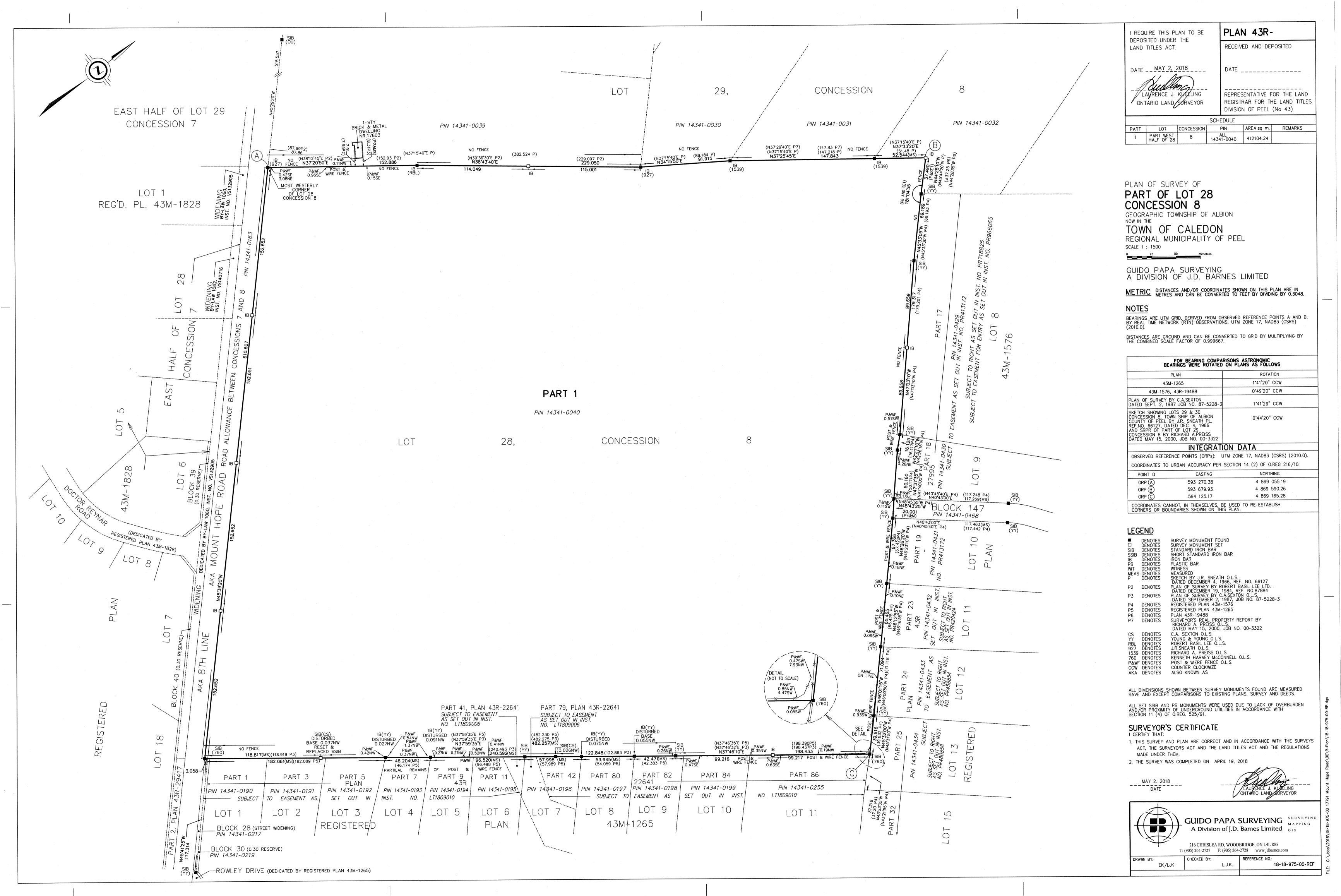


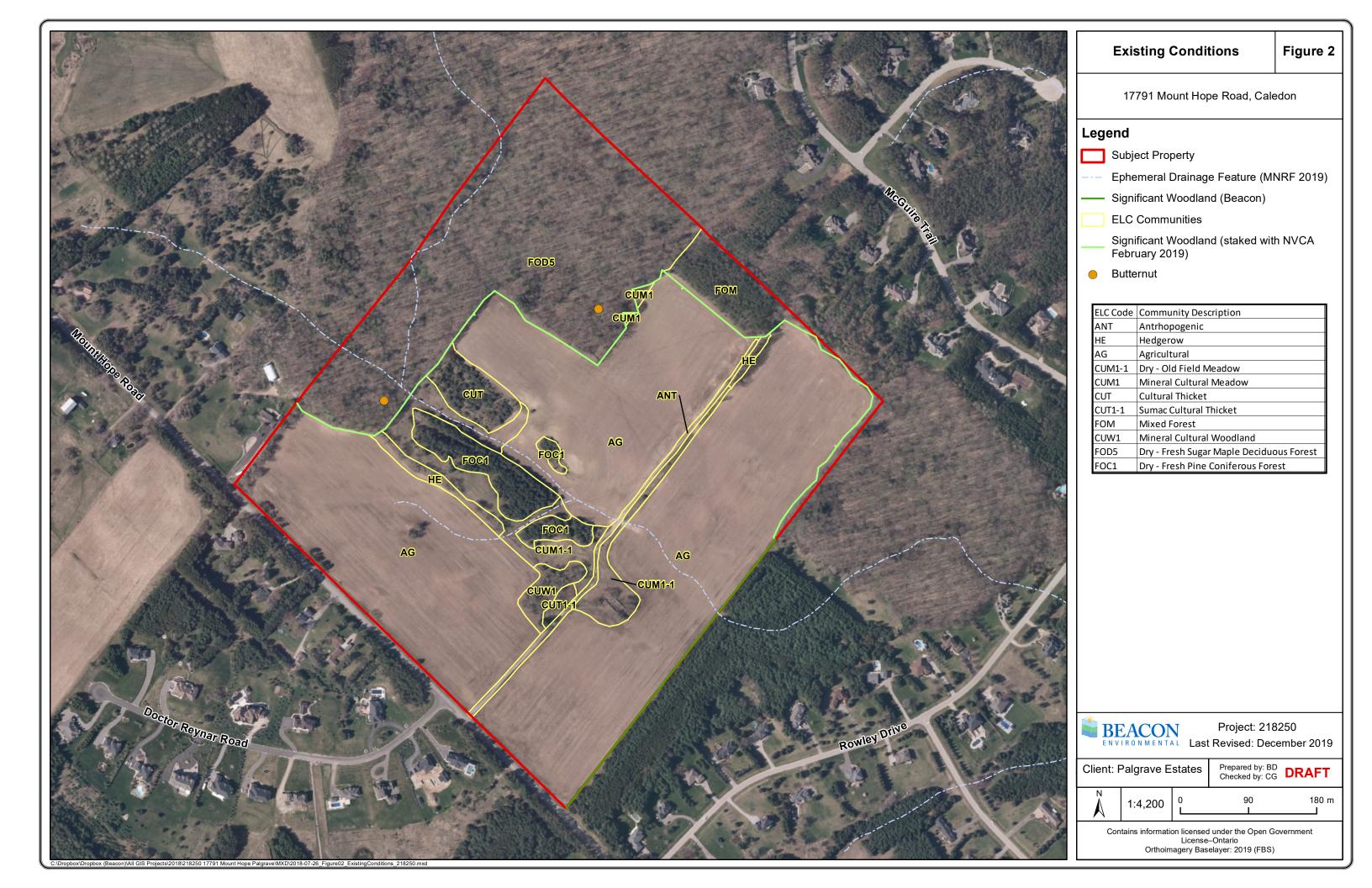


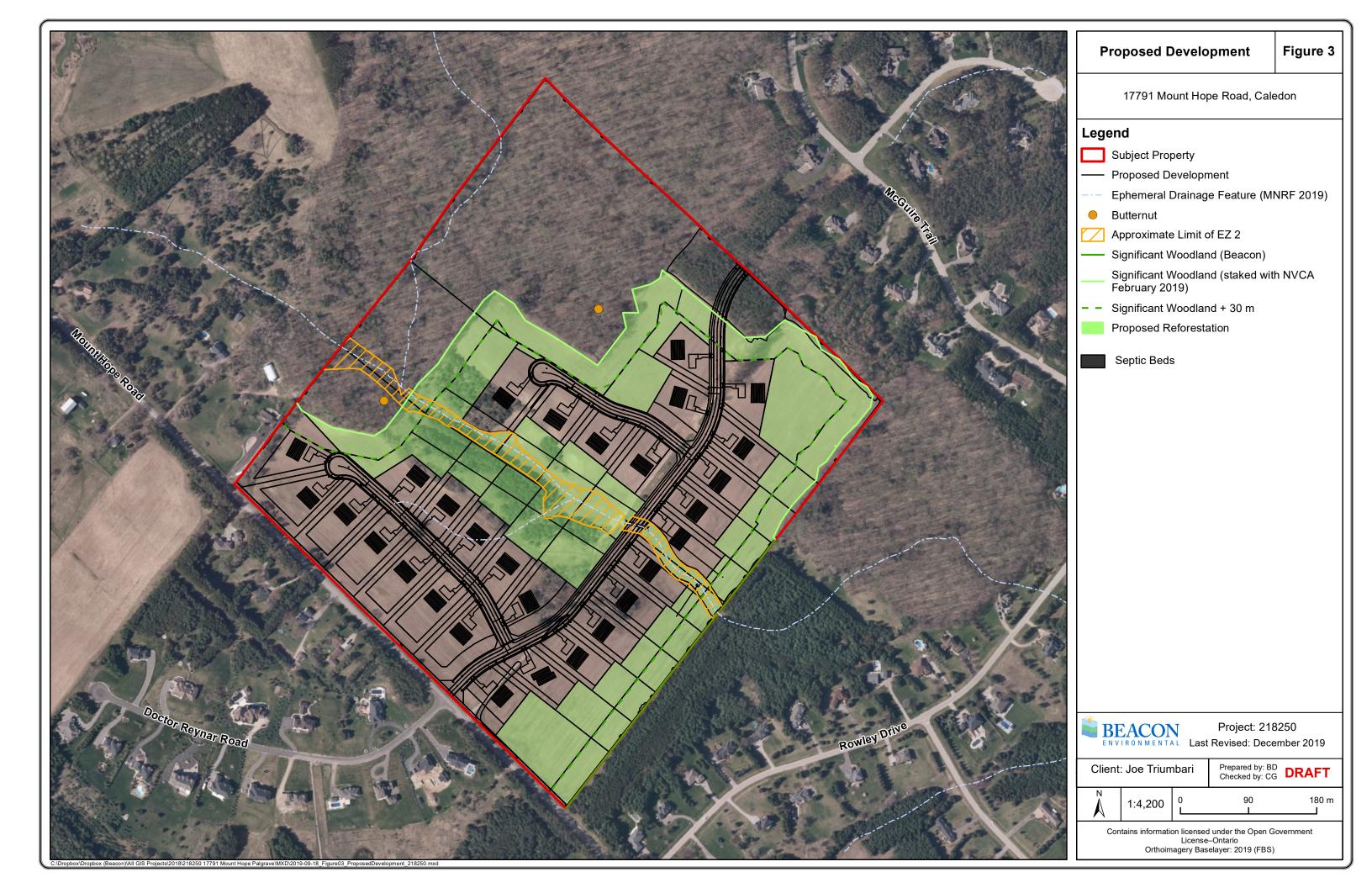


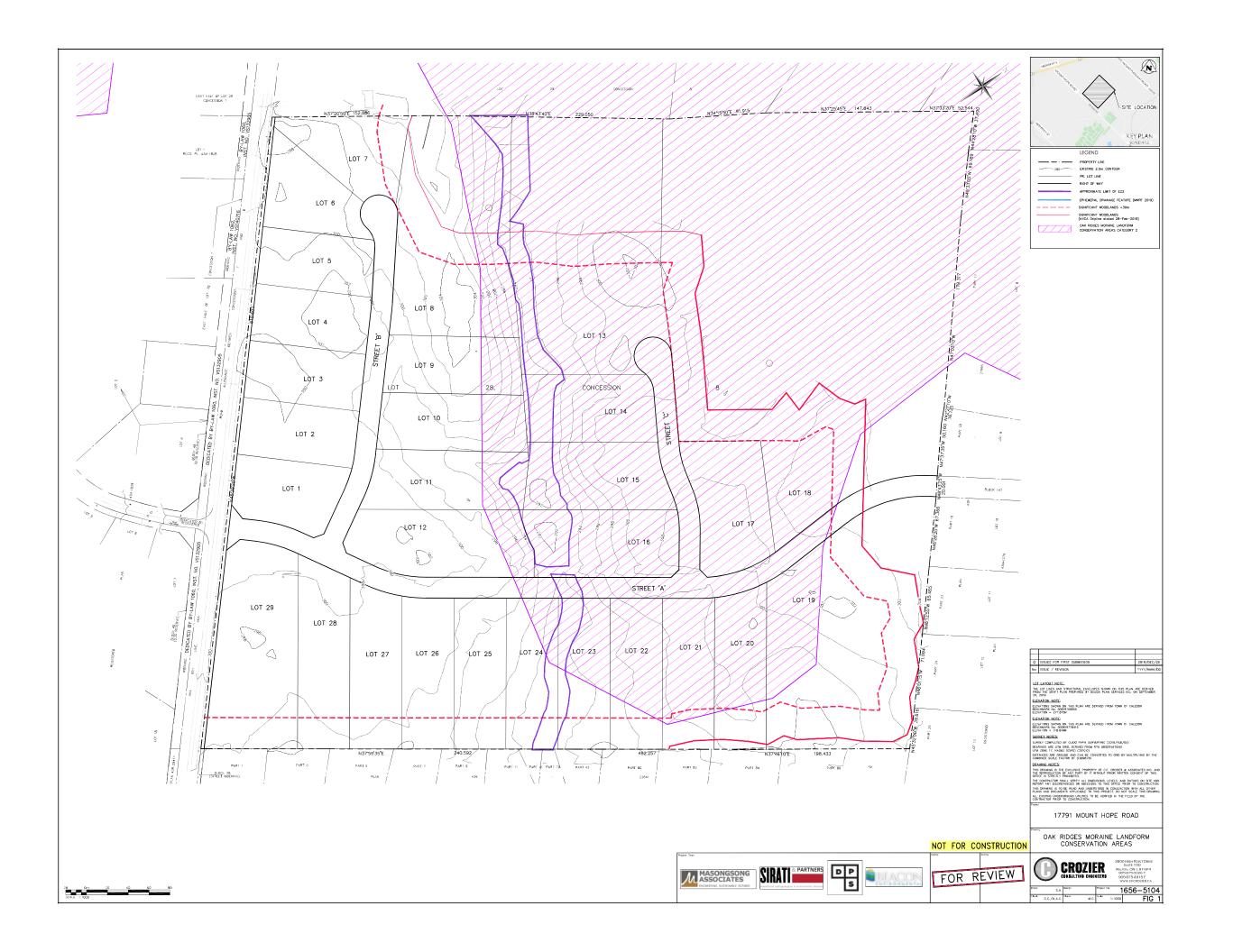




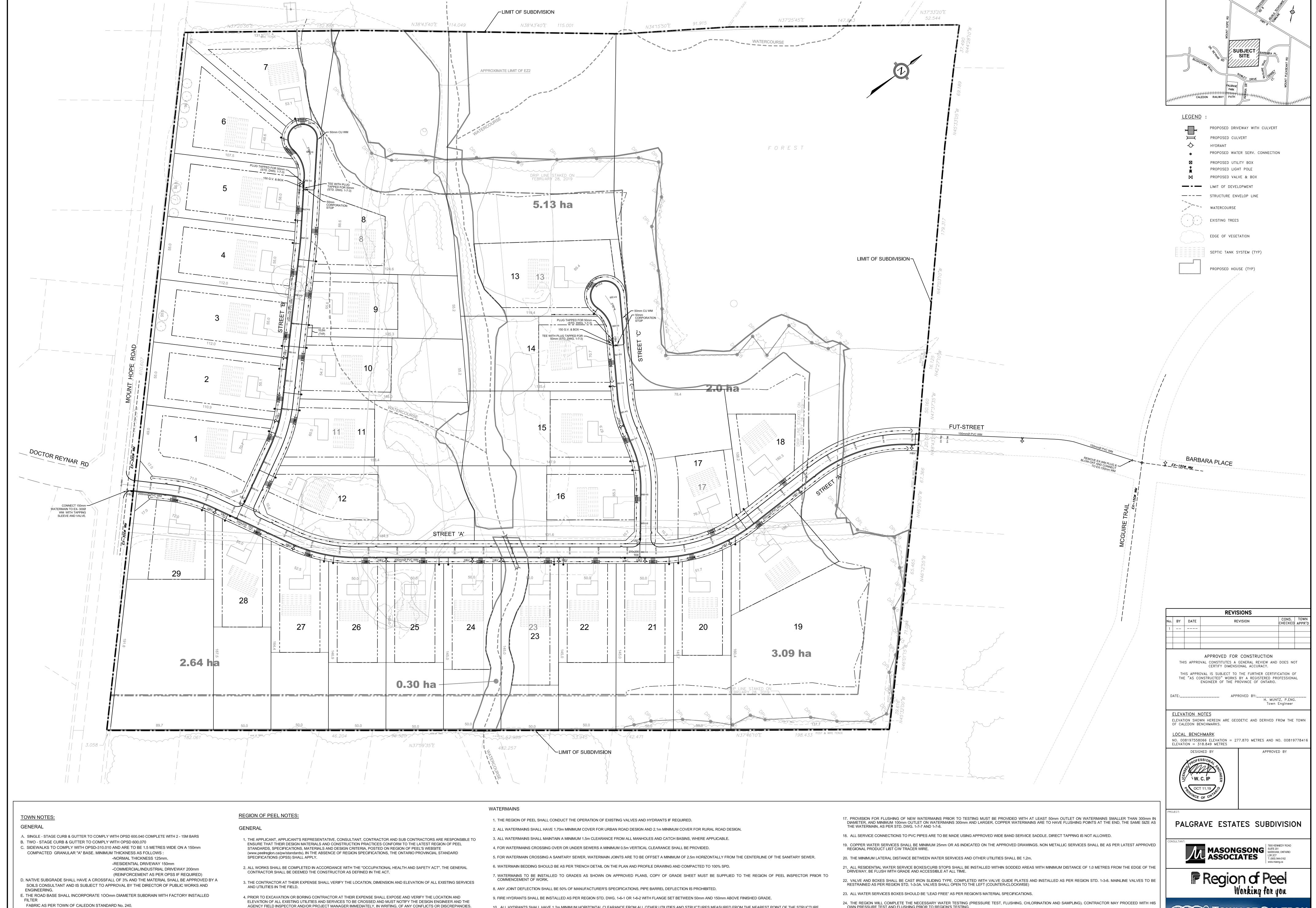












- F. ALL CURB RADII TO BE MINIMUM OF 10.0 METRES RESIDENTIAL AND 15.0 METRES INDUSTRIAL AT THE EDGE OF ASPHALT. G. NATIVE SUBGRADE TO BE COMPACTED TO MINIMUM 95% STANDARD PROCTOR MAXIMUM DRY DENSITY
- AND SHALL BE PROOF ROLLED. H. GRADE AND CROSS FALL ADJUSTMENT OF MAINTENANCE HOLE AND CATCH BASIN FRAMES WILL BE
- MADE USING PRODUCTS SPECIFICALLY MANUFACTURED FOR THAT PURPOSE AS PER OPSD 704.010. NON-COMPRESSIBLE BACK FILL WILL BE USED DURING REBUILDING, ADJUSTI NG, OR ANY OTHER APPLICABLE CATCH BASIN OR MAINTENANCE HOLE WORKS. J. CURB AND SIDEWALK CONCRETE SHALL BE 30MPa AT 28 DAYS WITH 7% +/- 1.5% ENTRAINED AIR

AGENCY FIELD INSPECTOR AND/OR PROJECT MANAGER IMMEDIATELY, IN WRITING, OF ANY CONFLICTS OR DISCREPANCIES. CONTRACTOR SHALL BE RESPONSIBLE FOR EXPOSING THE EXISTING UTILITIES FAR ENOUGH IN ADVANCE OF CONSTRUCTION TO MAKE NECESSARY DESIGN MODIFICATIONS FOR REVIEW AND APPROVAL, IF REQUIRED, WITHOUT DELAYING THE WORK.

5. THE CONTRACTOR, AT THEIR EXPENSE AND TO THE SATISFACTION OF THE REGION OF PEEL, SHALL BE RESPONSIBLE FOR THE RESTORATION AND THE REPAIR OF THE EXISTING UTILITIES AND ALL AREAS BEYOND THE PLAN OF SUBDIVISION DISTURBED DURING CONSTRUCTION.

7. ALL BACKFILL FOR SEWERS, WATERMAINS AND UTILITIES ON THE ROAD ALLOWANCE MUST BE MECHANICALLY COMPACTED.

6. THE SUPPORT OF ALL UTILITIES SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE AUTHORITY HAVING

8. ALL BOREHOLES SHOWN ON DRAWING ARE FOR INFORMATION ONLY. REFER TO GEOTECHNICAL REPORT. 9. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE SPECIFIED.

10. ALL HYDRANTS SHALL HAVE 1.2m MINIMUM HORIZONTAL CLEARANCE FROM ALL OTHER UTILITIES AND STRUCTURES MEASURED FROM THE NEAREST POINT OF THE STRUCTURE. 11. MECHANICAL RESTRAINERS ARE REQUIRED FOR ALL FITTINGS, VALVES, DEAD ENDS, CAPS AND HYDRANTS ON ALL PVC WATERMAINS; MINIMUM RESTRAINED PIPE LENGTH AS PER REGION'S STANDARD.

12. STAINLESS STEEL NUTS AND BOLTS ARE TO BE USED ON ALL METALLIC FITTINGS AND JOINT RESTRAINTS.

STD. DWG 1-7-7 AND 1-7-8.

13. ALL METALLIC VALVES, FITTINGS, THROUGH WALL METAL PIPING AND JOINT RESTRAINTS TO BE C/W DENSO PASTE, DENSO MASTIC & DENSO TAPE OR APPROVED EQUAL APPLIED TO MANUFACTURER'S RECOMMENDATIONS.

15. 50mm DIAMETER WATERMAIN SHALL BE TYPE K SOFT COPPER. WATERMAIN INSTALLATION IN CUL-DE-SACS AS PER REGION STD. DWG. 1-7-4.

14. WHERE PLASTIC PIPE IS USED, INSTALL A 12 GAUGE TWU STRANDED COPPER, LIGHT COLOURED, PLASTIC COATED TRACER WIRE ATTACHED TO THE PIPE WITH APPROVED WIRE SPLICE. THE WIRE SHOULD BE BROUGHT TO THE SURFACE AT EACH SERVICE & VALVE BOX AND HYDRANT VALVES.

16. A PHYSICAL SEPARATION MUST BE MAINTAINED AT ALL CONNECTION POINTS OF NEW WATERMAIN TO THE EXISTING SYSTEM UNTIL BACTERIOLOGICAL TESTS HAVE PASSED, AS PER

WATERMAIN IN FILL AREA

1. NO WATERMAIN TO BE LAID ON FILL UNTIL THE FIELD DENSITY TEST REPORTS HAVE BEEN SUBMITTED TO AND APPROVED BY THE REGION OF PEEL OR THE CONSULTING

2. PIPE JOINTS DEFLECTIONS ARE NOT ALLOWED IN FILL AREA. 3. JOINTS SHALL BE MECHANICALLY RESTRAINED THE WHOLE LENGTH.

OWN PRESSURE TEST AND FLUSHING PRIOR TO REGION'S TESTING.

4. ALL HYDRANTS, TEE BRANCH VALVES AND HORIZONTAL BENDS ARE TO BE MECHANICALLY RESTRAINED WITH TIE RODS.

5. IN EXISTING MUNICIPAL RIGHT-OF-WAY OR EASEMENT, FILL TO BE PLACED TO 600mm MINIMUM ABOVE THE OBVERT OF THE WATERMAIN AND TO 300mm EITHER SIDE, COMPACTED TO MINIMUM 100% STANDARD PROCTOR DENSITY IN 300mm LIFTS; AND THEREAFTER, FOR EVERY 300mm LIFT ALONG THE CENTERLINE, AND 1.5m TO EITHER SIDE, OF WATERMAIN AT MAXIMUM INTERVAL OF 30.0m. TEST RESULTS MUST BE SUBMITTED TO AND APPROVED BY THE CONSULTANT OR AGENCY.

TOWN OF CALEDON

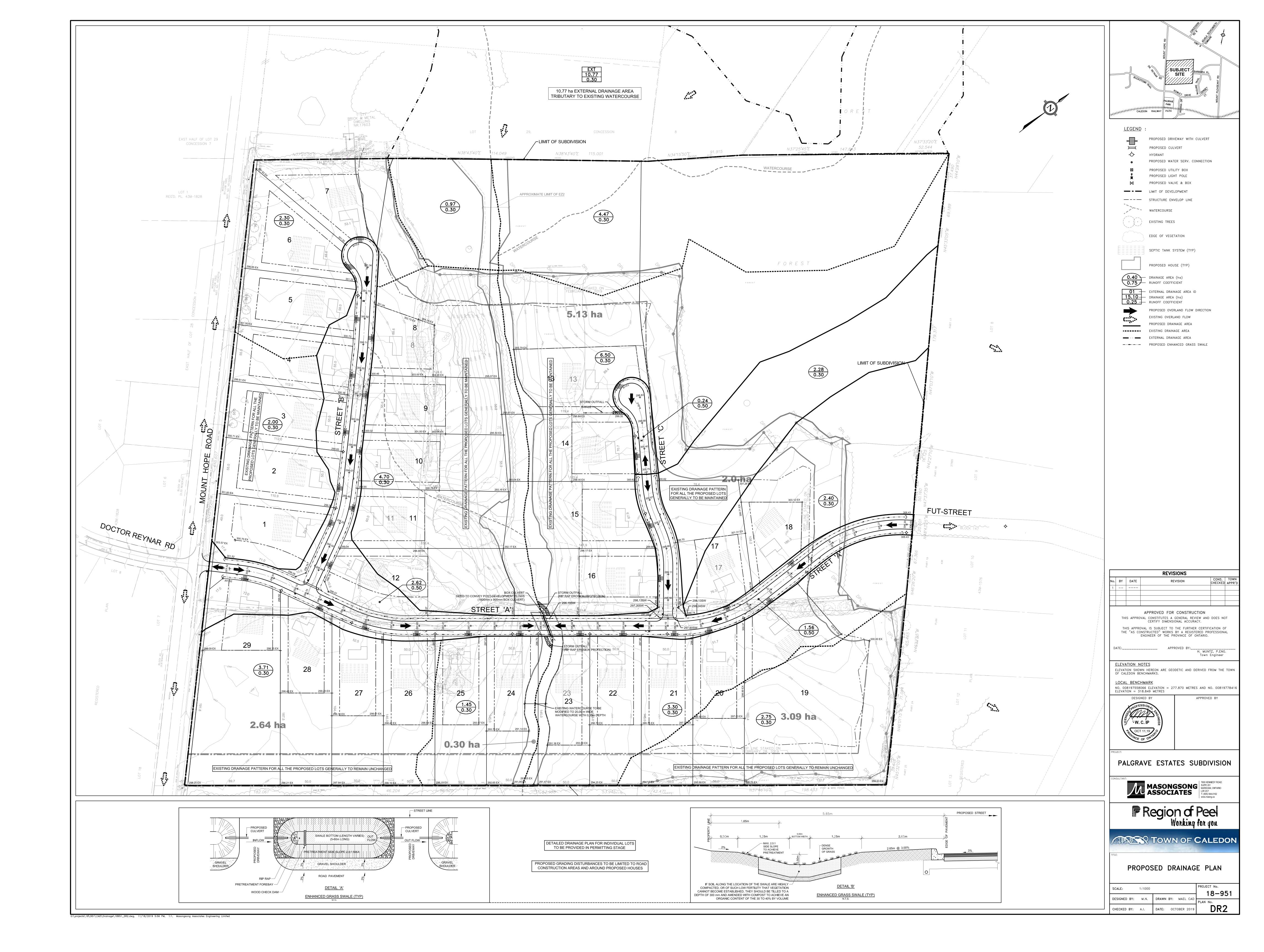
GENERAL PLAN

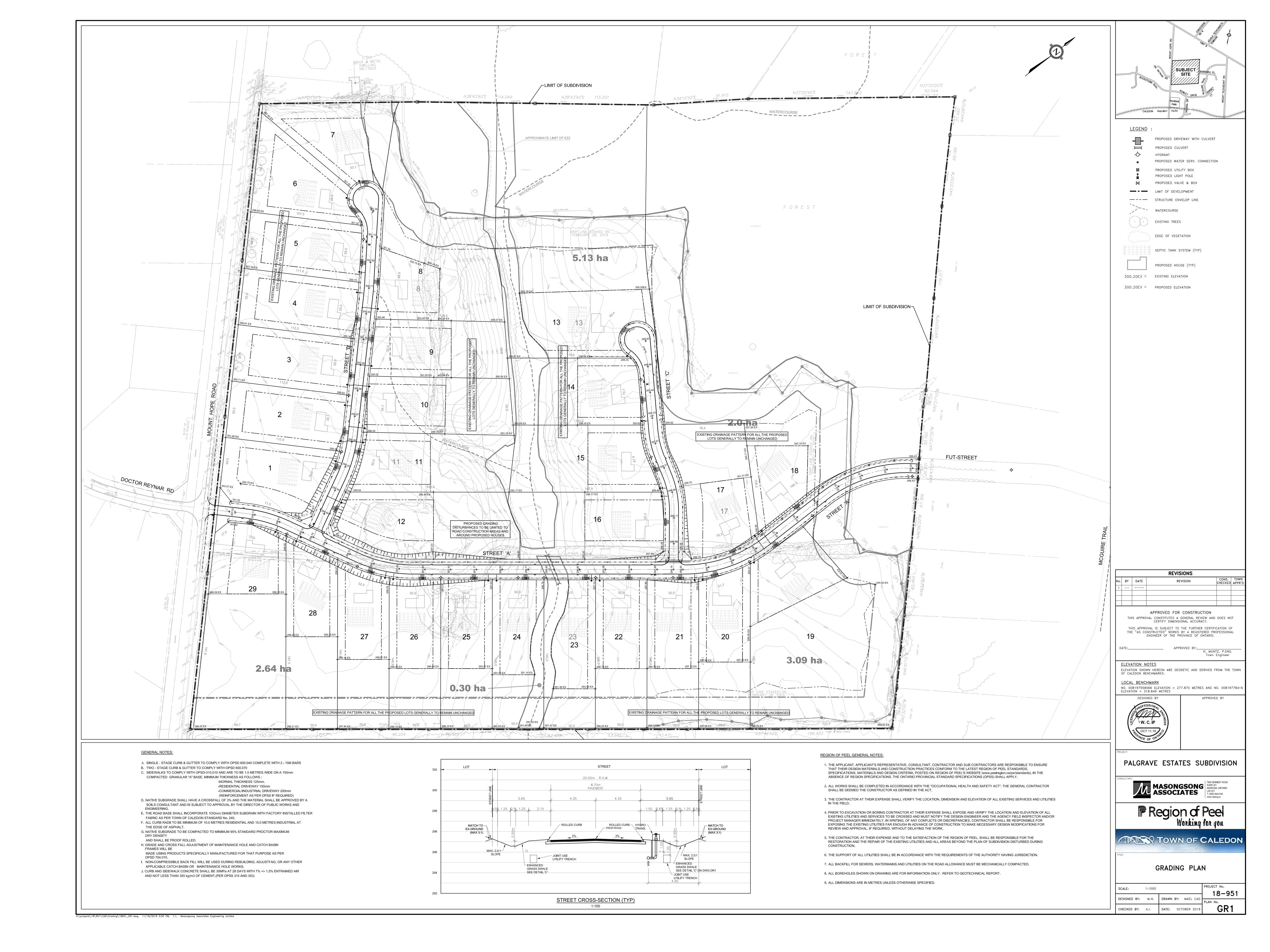
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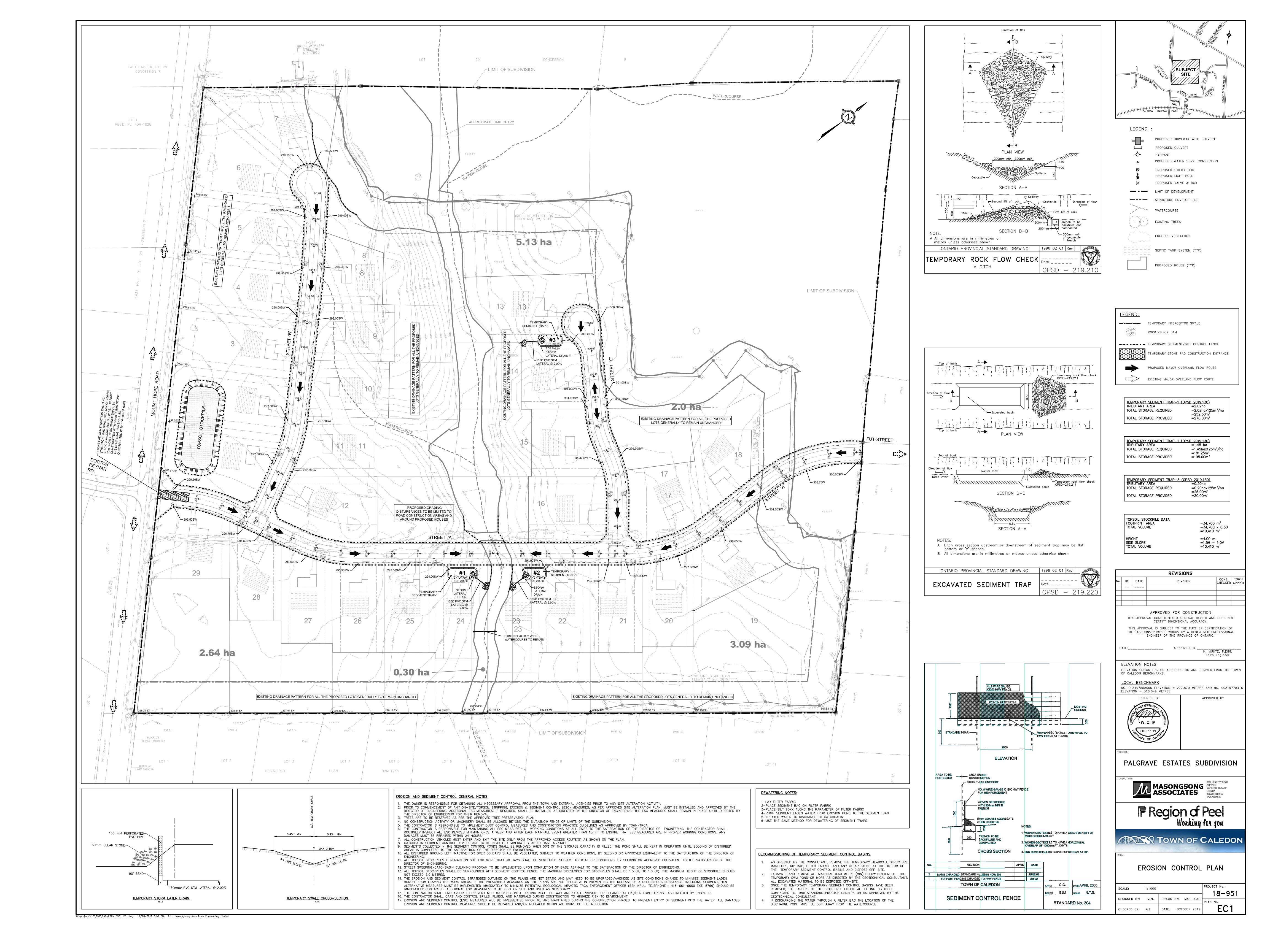
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AND NOT LESS THAN 355 kg/m3 OF CEMENT.(PER OPSS 315 AND 353)











Municipal and Development Engineering



Water Resources Engineering



Planning



Project Manag

MASONGSONG ASSOCIATES ENGINEERING LIMITED Consulting Engineers • Planners • Project M

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