



TOWN OF CALEDON
PLANNING
RECEIVED
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FERROENG GROUP INC.

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September 23, 2024

Our File 2478

Raymond Nicolini
InSite Construction Management Inc.
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Re: 8.5m H. Proposed Retaining Wall Feasibility Review
14 Station Road, Bolton, ON

In respect of the question as to whether an 8.5m H. Retaining Wall is feasible we offer the following comments.

We have reviewed the following drawings by GM BluePlan dated November 2023, Project No. 423091:

- a. Site Grading & Servicing Plan, Drawing No. 2 of 5.
- b. Retaining Wall Plan, Retaining Wall Section A-A, Drawing No. 5 of 5.

We have completed a Feasibility Review of the proposed 8.5 m H. Retaining Wall. On the basis of our Review, the proposed 8.5 m H. Retaining Wall is indeed feasible, in our opinion. The proposed 8.5 m H. Retaining Wall requires no loss of the quantity of parking spaces currently proposed, and does not require 'tie-backs'. Please see Feasibility Design Report, attached, for preliminary retaining wall design and associated design assumptions.

Please let us know if you require any other information.

Respectfully,
FERROENG GROUP INC.



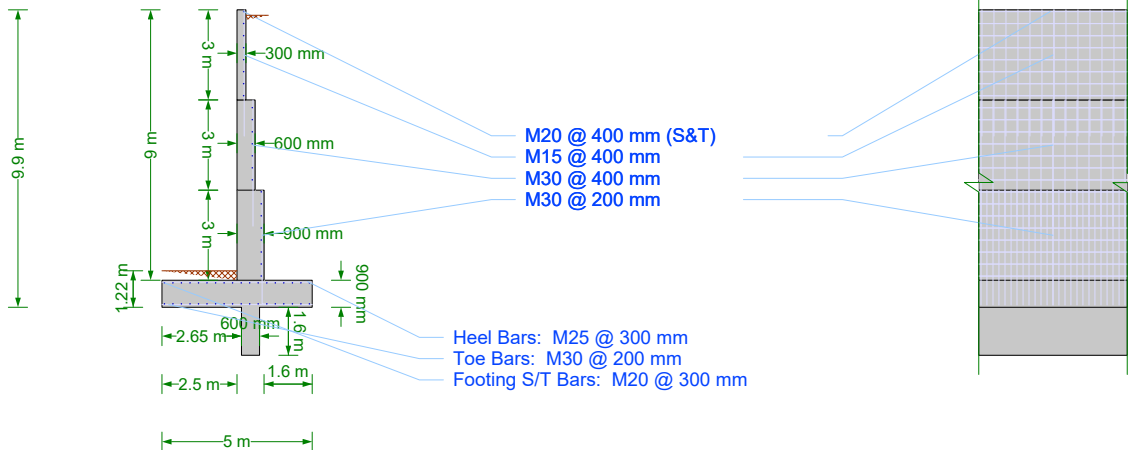
Per: Mike Tancredi, P. Eng., PE, MSCE

Attach.: Feasibility Design Report



Design Detail

Concrete $f_c = 35 \text{ MPa}$
Rebar $F_y = 413.7 \text{ MPa}$
Unit Weight = 23.56 kN/m^3



Check Summary

Criteria

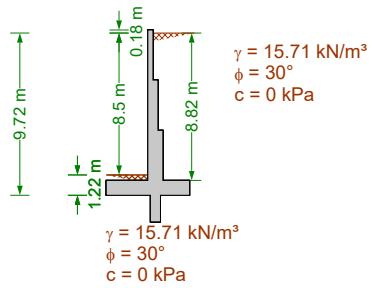
| Ratio | Check | Provided | Required | Combination |
|-------------------------------------|----------------------|--------------------------|-----------------------|-------------|
| ----- Stability Checks ----- | | | | |
| ✓ 0.728 | Overturning | 2.06 | 1.50 | 1.0D + 1.0H |
| ✓ 0.994 | Sliding | 1.51 | 1.50 | 1.0D + 1.0H |
| ✓ 0.853 | Bearing Pressure | 225 kPa | 191.8 kPa | 1.0D + 1.0H |
| ✓ 0.804 | Bearing Eccentricity | 0.67 m | 0.83 m | 1.0D + 1.0H |
| ----- Toe Checks ----- | | | | |
| ✓ 0.869 | Shear | 372.7 kN/m | 323.8 kN/m | 1.4D + 1.5H |
| ✓ 0.626 | Moment | 957.9 kN·m/m | 599.9 kN·m/m | 1.4D + 1.5H |
| ✓ 0.000 | Min Steel | 89.31 mm ² | 0 mm ² | 1.4D + 1.5H |
| ✓ 0.311 | Development | 242.4 cm | 75.47 cm | 1.4D + 1.5H |
| ✓ 0.600 | S&T Max Spacing | 300 mm | 500 mm | 1.4D + 1.5H |
| ✓ 0.890 | S&T Min Steel | 20.22 cm ² /m | 18 cm ² /m | 1.4D + 1.5H |
| ----- Heel Checks ----- | | | | |
| ✓ 0.942 | Shear | 380 kN/m | 357.8 kN/m | 1.4D + 1.5H |
| ✓ 0.741 | Moment | 386.2 kN·m/m | 286.3 kN·m/m | 1.4D + 1.5H |
| ✓ 0.000 | Min Steel | 42.61 mm ² | 0 mm ² | 1.4D + 1.5H |
| ✓ 0.250 | Development | 334.9 cm | 83.68 cm | 1.4D + 1.5H |
| ✓ 0.600 | S&T Max Spacing | 300 mm | 500 mm | 1.4D + 1.5H |
| ✓ 0.890 | S&T Min Steel | 20.22 cm ² /m | 18 cm ² /m | 1.4D + 1.5H |
| ----- Stem Checks ----- | | | | |
| ✓ 0.908 | Moment | 989.3 kN·m/m | 898 kN·m/m | 1.4D + 1.5H |
| ✓ 0.617 | c/d Limit | 0.3877 | 0.6285 | 1.4D + 1.5H |
| ✓ 0.805 | Shear | 379.4 kN/m | 305.5 kN/m | 1.4D + 1.5H |
| ✓ 0.683 | Min Steel | 17.58 cm ² /m | 12 cm ² /m | 1.4D + 1.5H |
| ✓ 0.404 | Base Development | 82.38 cm | 33.29 cm | 1.4D + 1.5H |
| ✓ 0.931 | Lap Splice Length | 120 cm | 111.8 cm | 1.4D + 1.5H |
| ✓ 0.000 | Lap Splice Spacing | 0 mm | 0 mm | 1.4D + 1.5H |
| ✓ 0.791 | Horz Min Steel | 7.58 cm ² /m | 6 cm ² /m | 1.4D + 1.5H |
| ✓ 0.800 | Horz Bar Spacing | 400 mm | 500 mm | 1.4D + 1.5H |

| | |
|--|--------------------------|
| Use basic criteria from common proje... | No |
| Building Code | CSA A23.3-04 |
| Concrete Load Combs | Canadian CBC-05 |
| Masonry Load Combs | ASCE 7-16 (ASD) |
| Stability Load Combs | IBC Retaining Wall St... |
| Apply Sds Factor to Seismic Combin... | No |
| Restrained Against Sliding | No |
| Neglect Bearing At Heel | Yes |
| Use Vert. Comp. for OT | No |
| Use Vert. Comp. for Sliding | No |
| Use Vert. Comp. for Bearing | Yes |
| Use Surcharge for Sliding & OT | Yes |
| Use Surcharge for Bearing | Yes |
| Neglect Soil Over Toe | No |
| Neglect Backfill Wt. for Coulomb | No |
| Factor Soil Weight As Dead | Yes |
| Use Passive Force for OT | Yes |
| Assume Pressure To Top | Yes |
| Extend Backfill Pressure To Key Bott... | No |
| Use Toe Passive Pressure for Bearing | No |
| Required F.S. for OT | 1.50 |
| Required F.S. for Sliding | 1.50 |
| Has Different Safety Factors for Seis... | No |
| Allowable Bearing Pressure | 225 kPa |
| Req'd Bearing Location | Middle third |
| Wall Friction Angle | 25° |
| Friction Coefficient | 0.35 |
| Soil Reaction Modulus | 27146 kN/m ³ |

Loads

Loading Options/Assumptions

→ Passive pressure neglects top 0 m of soil.



Load Combinations

Canadian CBC-05

- 1.4D + 1.5H
- 0.9D + 1.5H
- 1.25D + 1.5H
- 1.0D + 1.5H
- 1.4D
- 0.9D
- 1.25D
- 1.0D