



File #: 1564
Date: February 27, 2019

Mr. David Hurst, C.E.T.
Town of Caledon
6311 Old Church Road
Caledon, Ontario, L7C 1J6

Dear Mr. Hurst:

Re: Stormwater Management Compliance Letter
Digram Developments Caledon – Block 132
Town of Caledon, Regional Municipality of Peel

We are pleased to provide you with the following stormwater management (SWM) compliance letter in support of the proposed Digram Developments Caledon – Block 132 residential site plan development in the Town of Caledon (refer to **Figure 1**).

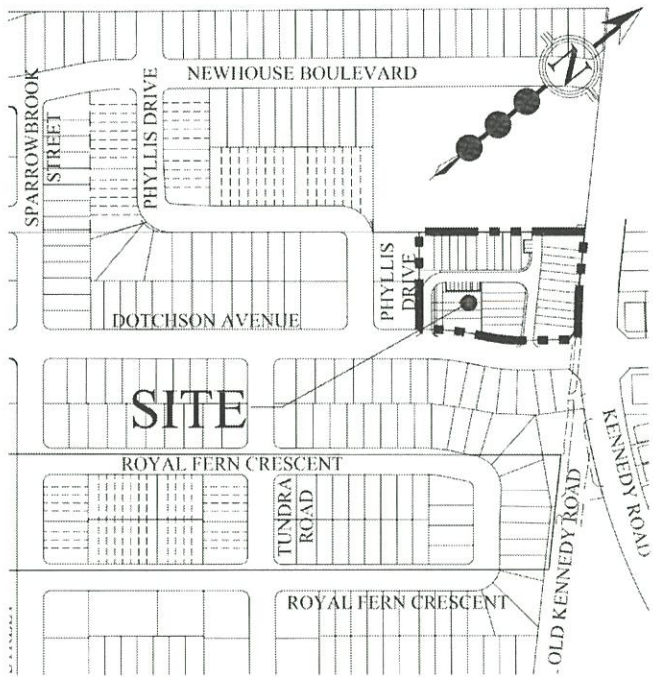


Figure 1: Site Location Map

SCS Consulting Group Ltd. has been retained by Digram Developments Caledon Inc. to prepare this SWM compliance letter in support of the proposed development within the West ‘A’ Residential Neighbourhood of the Mayfield West Community Secondary Plan, located in the Town of Caledon. The study area is approximately 0.79 ha in size and is bound by:

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Digram Developments Caledon – Block 132
Town of Caledon, Regional Municipality of Peel

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- ➔ Kennedy Trails Development Ltd. (21T-12003C) to the north,
- ➔ Kennedy Road to the east,
- ➔ Phyllis Drive to the west,
- ➔ Dotchson Avenue to the south.

The proposed storm drainage within the subject development, per the previously approved design of the existing SWM Pond E2, was to be conveyed via the existing minor and major system infrastructure at an imperviousness of 79%. The existing SWM Pond E2 is located in the existing Argo Caledon Subdivision Phase 1 and the Moscorp VII Developments Inc. subdivisions, respectively. Refer to **Drawing STM-1** for the proposed storm drainage plan.

As shown in **Drawing STM-1**, the proposed site development area and density remains the same as what was contemplated in the design of the existing downstream SWM Facilities. Therefore, it can be concluded that the proposed development satisfies the design criteria for the existing SWM pond and can therefore rely on SWM Pond E2 for quality, quantity and erosion control.

Due to the nature of the servicing design for the subject development, the assumed conveyance method of the 10 year flow in the storm sewer (minor system) with the remaining flows conveyed via the municipal ROW (major system) cannot be achieved. This is due to a number of inlets that cannot be equipped with inlet control devices (ICDs). While ICDs have been included where feasible and/or appropriate within the laneway, the design results in four rear lot catchbasins without ICDs, one laneway catchbasin without an ICD and three catchbasin manholes without ICDs. To ensure that the downstream minor and major system conveyance infrastructure is adequately sized, an update to the existing modelling was completed by JF Sabourin on behalf of the subject development (**Attachment A**). The memo concludes that there are no negative impacts downstream.

Since the development of the Comprehensive Environmental Impact Statement and Management Plan for the Mayfield West Community Development Plan Area in 2007; new requirements for water balance have been implemented requiring the first 5 mm of rainfall to be detained on-site.

To select appropriate controls for the subject development, the following site characteristics were taken into consideration:

- ➔ Subsurface soils and groundwater information within the proposed development area were evaluated by Canadian Engineering Services Inc. Excerpts can be found in **Attachment B**. Underlying fills or reworked tills, the native subsoil mainly consist of silty sand and glacial tills ranging in gradation from clayey silt till to silty sand till;
- ➔ Water table depth is typically 0.7 m to 1.9 m below the existing grade, acknowledging this is subject to seasonal fluctuation.

Note that based on the shallow groundwater table (0.7 m to 1.9 m) and the requirements set out within the TRCA/CVC LID guidelines, infiltration measures will not be effective as they require at least 1.0 m of separation from the groundwater table which is not present. As such, soakaway pits, pervious pipe and catchbasin infiltration systems are not expected to be effective.

The following stormwater management measures are proposed to provide best efforts:



Re: Stormwater Management Compliance Letter
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Reduced Lot Grading

Reducing lot grading from a maximum of 5% to a minimum of 2% is suggested wherever possible to maximize lot level initial abstraction and infiltration. As shown on the engineering drawing set, provided under separate cover, lot grading on site ranges from 2%-5%.

Increased Topsoil Depth

An increase in the proposed topsoil depth on lots will also be used to promote lot level infiltration (up to 0.3 m depth).

Roof Leaders to Grassed Areas

Roof leaders and sump pumps (if required) will be discharged to grassed areas to promote lot level infiltration where feasible.

Rear Lot Catchbasin Infiltration System

Due to high groundwater elevations and the proximity of potential RLCB infiltration systems to the proposed buildings, it is not feasible to propose this solution on the subject development.

In conclusion, the proposed Caledon Block 132 private residential development is in compliance with the existing downstream SWM Pond E2, the storm conveyance infrastructure as well as with the overall Comprehensive Environmental Impact Statement and Management Plan for the Mayfield West Community Development Plan Area.

Please contact the undersigned if you have any questions or require any additional information.

Sincerely,

SCS Consulting Group Ltd.



John Priamo, P. Eng.
jpriamo@scsconsultinggroup.com

Attachments: **Drawing STM-1** – Storm Drainage Plan
Attachment A – JF Sabourin SWM Memo
Attachment B – Relevant Report Excerpts

c. Mr. A. Memon, Digram Developments Caledon Inc. (Letter only via email)

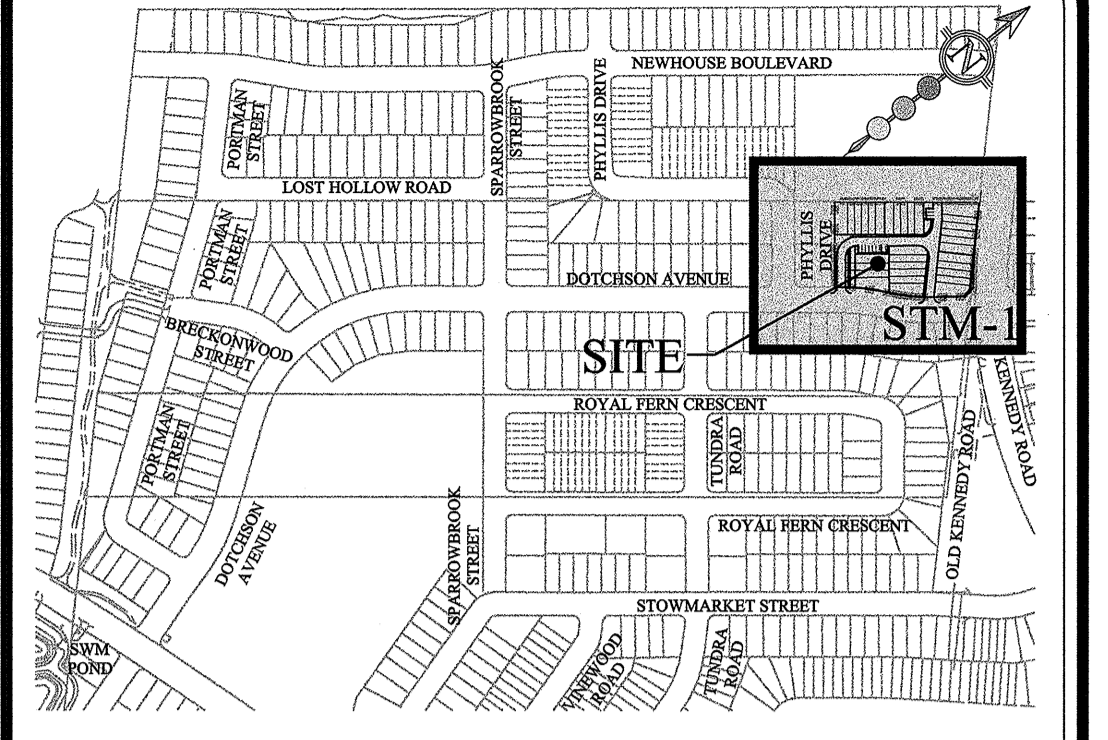
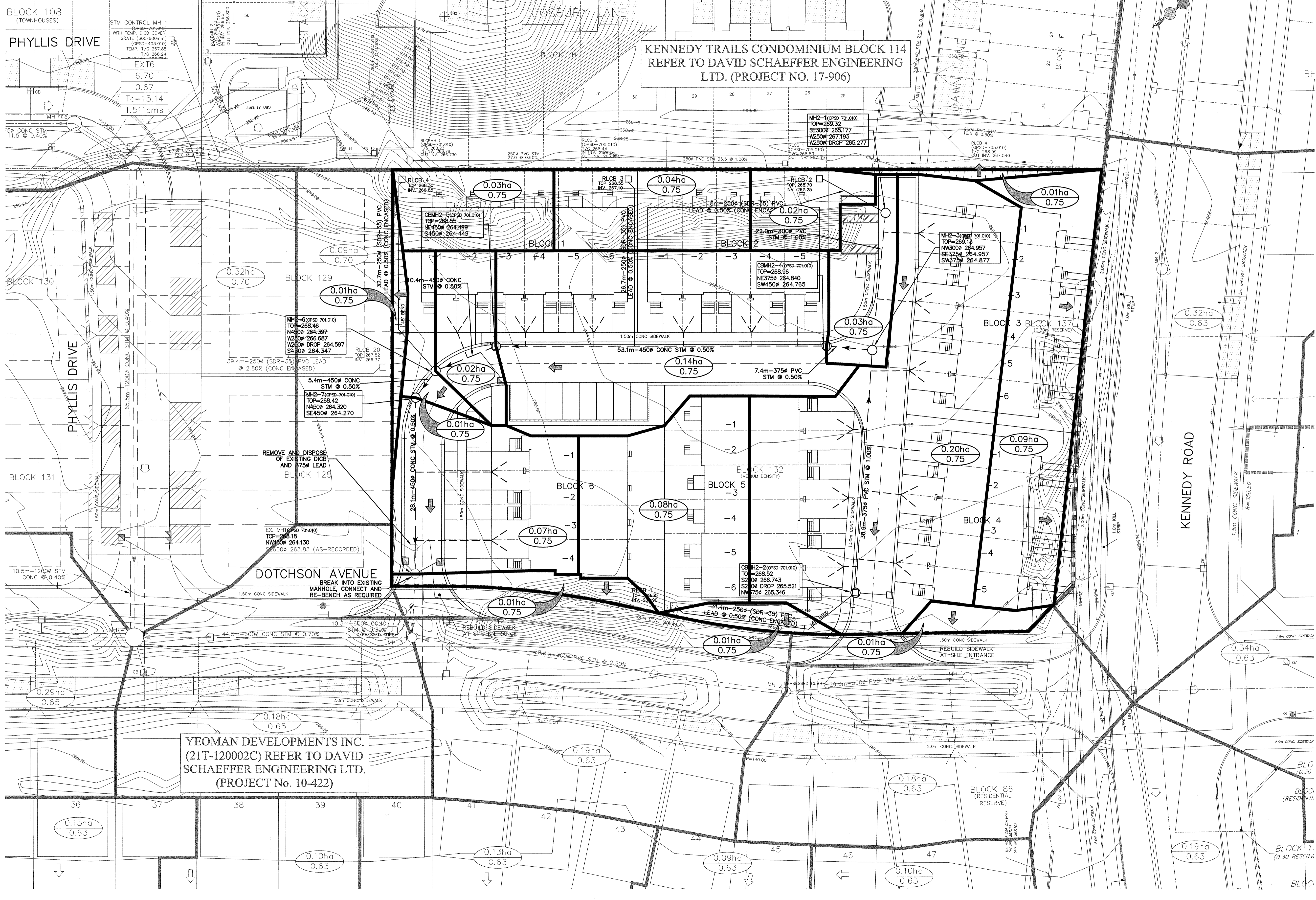
P:\1564 Digram-Caledon\Correspondence\Letters\Caledon-jmp-Block 132 Detailed Design SWM Compliance Letter-26feb19.doc



KENNEDY TRAILS DEVELOPMENT LTD. (21T-12003C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 17-906)

KENNEDY TRAILS CONDOMINIUM BLOCK 114 REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 17-906)

YEOMAN DEVELOPMENTS INC. (21T-120002C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT No. 10-422)



KEY PLAN
N.T.S.

BENCHMARK: ELEV. 259.419
PEEL BENCHMARK NO. 56.

- LEGEND:**
- STORM MANHOLE
 - EXISTING/FUTURE STORM MANHOLE
 - EXISTING/FUTURE SINGLE CATCHBASIN
 - EXISTING/FUTURE DOUBLE CATCHBASIN
 - SINGLE CATCHBASIN WITH NO ICD
 - SINGLE CATCHBASIN WITH 19.8 L/S (TYPE 'A') INLET CONTROL DEVICE (ICD)
 - DOUBLE CATCHBASIN WITH NO ICD
 - SINGLE SERVICE CONNECTION
 - DUAL SERVICE CONNECTIONS
 - 100 LOT NUMBER
 - 9 EXISTING/FUTURE LOT NUMBER
 - LIMIT OF PROPERTY
 - DRAINAGE BOUNDARY
 - EXISTING/FUTURE DRAINAGE BOUNDARY
 - 0.51ha DRAINAGE AREA (HECTARES)
 - 0.50 RUNOFF COEFFICIENT
 - 2.29ha EXISTING DRAINAGE AREA (HECTARES)
 - 0.75 RUNOFF COEFFICIENT
 - OVERLAND FLOW ROUTE
 - EXISTING/FUTURE OVERLAND FLOW ROUTE
 - EXISTING CONTOUR
 - 100 YEAR FLOW INFORMATION FROM STORMWATER MANAGEMENT REPORT FOR KENNEDY TRAILS DEVELOPMENT LTD. PREPARED BY J.F. SABOURIN AND ASSOCIATES INC., DATED FEBRUARY 2016. EXTERNAL DRAINAGE AREA, EQUIVALENT RUNOFF AND TIME OF CONCENTRATION INFORMATION FROM 10 YEAR DESIGN SHEET FOR KENNEDY TRAILS DEVELOPMENT LTD., DATED FEBRUARY 2016. YEOMAN DEVELOPMENTS INC., DATED APRIL 2015. MOSCORP DEVELOPMENT INC., DATED OCTOBER 2013. PREPARED BY DAVID SCHAEFFER ENGINEERING LIMITED.
 - 1.51 EXTERNAL DRAINAGE AREA (ha)
 - 0.63 EQUIVALENT RUNOFF COEFFICIENT
 - Tc=11.60 TIME OF CONCENTRATION (MIN.)
 - 0.527cms 100-YEAR FLOW

TOPOGRAPHIC SURVEY PROVIDED BY HOLDING JONES VANDERVEEN INC., JANUARY 2016

REVISIONS				
No.	DESCRIPTION	DATE	BY	APPROVED

SCS consulting group ltd
30 CENTURIAN DRIVE, SUITE 100
MARKHAM, ONTARIO L3R 8B8
TEL: (905) 475-1900
FAX: (905) 475-8335

TOWN OF CALEDON
6311 OLD CHURCH ROAD
CALEDON, ONTARIO L7C 1J6
TEL: (905) 584-2272
FAX: (905) 584-4325

DIGRAM DEVELOPMENTS CALEDON INC.
CALEDON - CONDO BLOCK 132
21T-14002C (T-14002Ca)



STORM DRAINAGE PLAN

DATE: MARCH 2019 DESIGNED BY: D.W.C.L.C. CHECKED BY: M.R.C.
SCALE: 1:300 DRAWN BY: J.Y.L. CHECKED BY: J.Y.L.

APPROVED FOR CONSTRUCTION
THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CERTIFY DIMENSIONAL ACCURACY.

THIS APPROVAL IS SUBJECT TO THE FURTHER CERTIFICATION OF THE "AS-CONSTRUCTED" WORKS BY A REGISTERED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO

DATE: _____ APPROVED BY: _____ PROJECT No: **1564**
DRAWING No: **STM-1**



ATTACHMENT A
JF SABOURIN SWM MEMO



Feb 16, 2022



J.F. Sabourin and Associates Inc.

WATER RESOURCES AND ENVIRONMENTAL
CONSULTANTS

52 Springbrook Drive
Ottawa (Stittsville), ON K2S 1B9
TEL: (613) 836-3884
FAX: (613) 836-0332
WEB: www.jfsa.com

July 4, 2018

SCS Consulting Group Limited

30 Centurian Drive, Suite 100
Markham, Ontario L3R 8B8

Attention: Mr. Douglas Woo, P.Eng.

Subject: Digram Developments Caledon Inc. /
Downstream Impacts of Condo Block 132 SWM Design

our file: 552-05

As requested by your office and based on the available information as described below, we have evaluated the impact of the latest design for Condo Block 132 by SCS Consulting Group Limited on the performance of downstream storm sewer system (existing and proposed) and existing Stormwater Management (SWM) Facility E2.

The proposed condo block is located within the Diagram Developments Caledon Inc. Subdivision, and north of the Yeoman Developments Inc. subdivision, within the Town of Caledon. The downstream storm sewer system and pond were designed to service a 0.79 ha area at 79% imperviousness from the condo block site, as per the August 2011 *Design Brief for Stormwater Management Pond E2 for the Mayfield West Community 3rd Registration Area* by David Schaeffer Engineering Limited and J.F. Sabourin and Associates Inc., and the more recent update to the overall model in the December 2017 *Stormwater Management Report for Kennedy Trails Development Condo Block* by J.F. Sabourin and Associates Inc. Per the December 2017 *SWM Report*, minor system capture from the condo block to Yeoman MH 3 was limited to the 10-year Rational Method flow of 221 L/s, assuming no surface storage on-site and with excess major system flows draining south to Dotchson Avenue just east of the intersection with Phyllis Drive.

The detailed design of the condo block by SCS Consulting Group Limited proposes that the 0.79 ha block be developed at an average imperviousness of 64%. Refer to Attachment C for the grading plan, servicing plan and storm drainage plan as provided by SCS Consulting Group Limited in July 2018. As the proposed imperviousness is lower than that estimated in the preliminary design (79%), it may be reasonably expected that the detailed design of the condo block will not have a negative impact on the performance of SWM Facility E2.

The SWM design by SCS Consulting Group Limited proposes an internal storm sewer system servicing 0.65 ha of the site. Excess major system flows from the 0.65 ha area will drain overland to Dotchson Avenue at two access roads to the condo block (east and west). Major system flows from the west access road drain to Dotchson Avenue just east of the intersection with Phyllis Drive, consistent with the preliminary design. However, the east access road is located at a high point on Dotchson Avenue, such that major system flows on the west side of the access road will drain west along Dotchson Avenue towards its intersection with Phyllis Avenue, and major system flows from the east side of the access road will drain east along Dotchson Avenue towards its intersection with Kennedy Road. This is not consistent with the preliminary design and may impact the performance of the major and minor systems on this segment of Dotchson Avenue and downstream Kennedy Road. Furthermore, the remaining 0.14 ha of the condo block cannot be serviced by the internal storm sewer due to grading restrictions, and sheet flows overland to Dotchson Avenue and Kennedy Road, with a small portion (0.01 ha) draining into external rearyards along the boundary of the site.

The most recent DDSWMM / XPSWMM models of the overall drainage area to SWM Facility E2, as submitted with the December 2017 *SWM Report*, were updated to reflect the detailed design of Condo Block 132 as provided by SCS Consulting Group Limited. The proposed minor and major system drainage routes for Condo Block 132

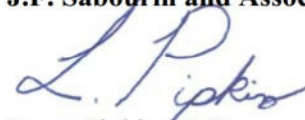
are shown in plan view in Figure 1. Refer to the December 2017 *SWM Report* for details of the external areas included in the model. The internal storm sewer within the condo block has four (4) rearyard catchbasins without inlet control devices (ICDs), three single catchbasins with Type A ICDs, one single catchbasin without an ICD, and three catchbasin manholes without ICDs.

Within the downstream development, ICDs were used to limit minor system capture to the 10-year flow. As a result of the 0.13 ha of the condo block draining overland to Kennedy Road and Dotchson Avenue, the ICDs in the two existing catchbasins on Dotchson Avenue at its intersection with Kennedy Road (97.5 L/s combined capacity) are now undersized to fully capture the 10-year flows (112 L/s based on the updated modelling). However, the existing storm sewer is not sufficiently sized to allow for larger ICDs to be installed at this location without a negative impact on the performance of the storm sewer system. Major system flows from the condo block to this location was minimized where possible, most notably by the inclusion of a 12 cm deep static ponding area and catchbasins without ICDs on the east Condo Block access road, to ensure the capture the full 10-year flows on the east access road. Redirecting roof drainage into the internal Condo Block system from the portion of the site fronting Kennedy Road was considered, but we understand is not possible due to the peaking of the roofs. It should be noted that the full 5-year flows (87 L/s) are captured at the two existing catchbasins on Dotchson Avenue, and as such standing water at this location will not be frequent. Furthermore, all ICDs and existing catchbasins on Kennedy Road downstream of this intersection are sufficiently sized to capture the 10-year flows, so the 14.5 L/s of flow spilling during the 10-year storm is a localized issue only.

Within the downstream development, the depth of water at the gutter will be retained within the right-of-way and will not exceed the maximum allowable value of 30 cm during the 100-year Chicago Storm (refer to Calculation Sheet B-1 of Attachment B, where the calculated maximum was 24.8 cm). Furthermore, it was determined that, for the 100-year event and for all major system segments, the product of the depth of water (m) at the gutter multiplied by the velocity of flow (m/s) will not exceed the maximum allowable 0.65 m²/s (refer to Calculation Sheet B-1 of Attachment B, where the calculated maximum was 0.371 m²/s). Furthermore, the two 15.5 m wide overland flow routes provided north of Pond E2 still have sufficient capacity to safely convey the 100-year major system flows from Dougall Avenue to the pond (refer to Calculation Sheet B-2 of Attachment B).

Tables A-1 and A-2 of Attachment A summarizes the pipe data and hydraulic simulation results based on the updated modelling for the 100-year 4-hour Chicago design storm under free outfall conditions for the Pond E2 system under ultimate and interim conditions, respectively (for the school block to the north; refer to the December 2017 *SWM Report* for further details). Note that the flowing full pipe velocities are not less than 0.75 m/s and no greater than 6.0 m/s for all proposed pipes. The 100-year flow will surcharge most parts of the minor system; however, a freeboard of 0.3 m between the hydraulic grade line and the underside of footings has been provided throughout developments tributary to Pond E2. Note that lots marked "FUT" indicate a location where the freeboard is an estimate only, and the actual freeboard within the Digram Developments Caledon Inc. subdivision and Condo Block 132 is to be verified by SCS Consulting Group Limited.

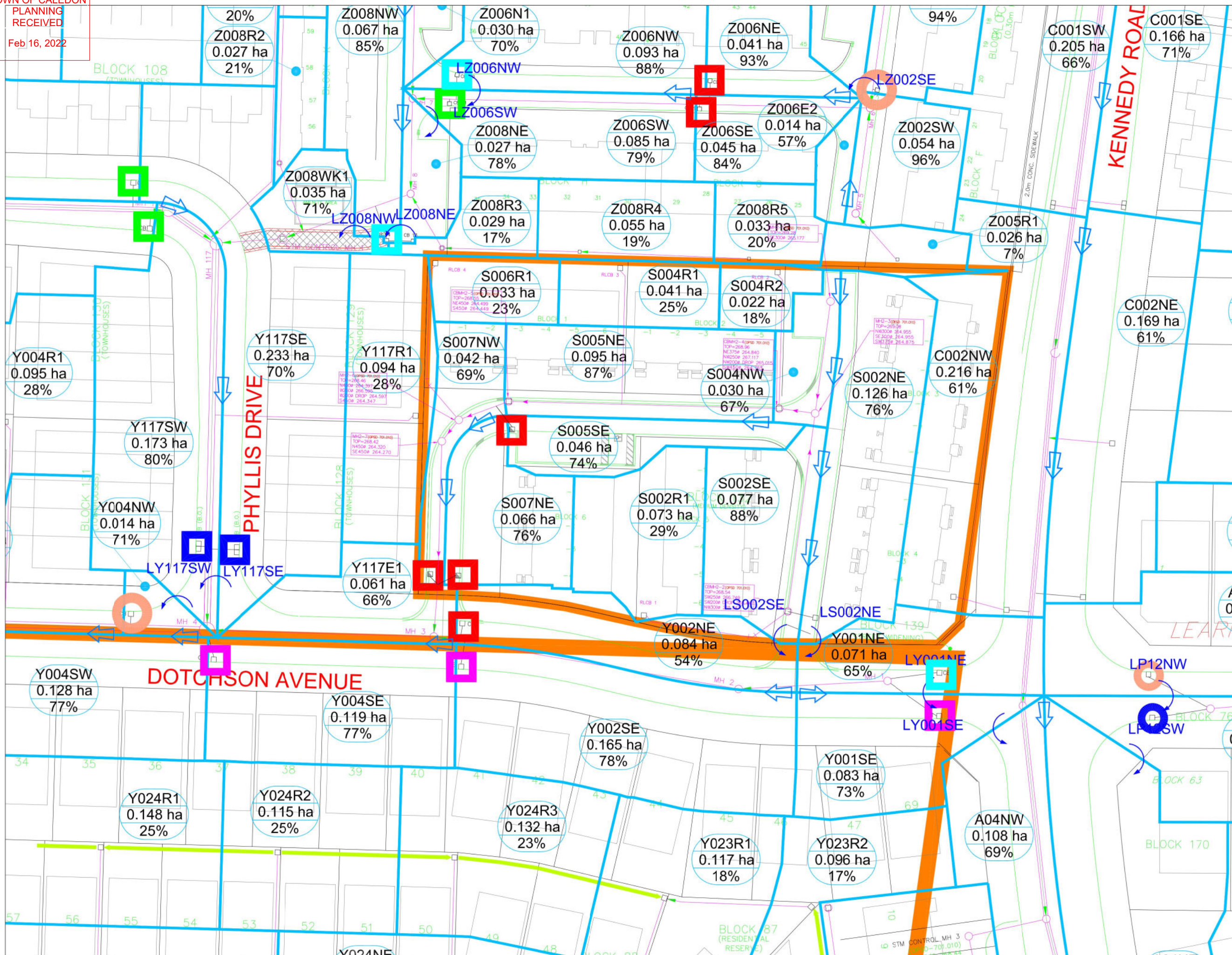
Yours truly,
J.F. Sabourin and Associates Inc.



Laura Pipkins, P.Eng.

cc: J.F. Sabourin, M.Eng, P.Eng.
Director of Water Resources Projects

- Figure 1: Proposed Minor and Major System
- Attachment A: Pipe Data and Hydraulic Gradeline Results; XPSWMM Model Schematic
- Attachment B: Calculation Sheets
- Attachment C: Drawings GR-1, S-1 and STM-1 (July 2018, SCS Consulting Group Limited)



LEGEND :

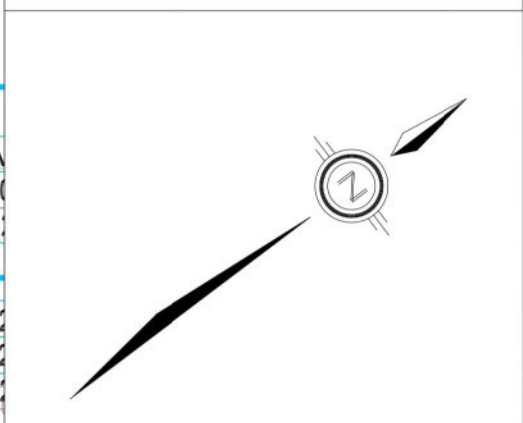
- LIMITS OF SUBDIVISION
- MAJOR SYSTEM SUBCATCHMENT BOUNDARY TO LOW POINTS AND OTHER AREAS
- ⇨ MAJOR SYSTEM FLOW DIRECTION
- ↻ FIRST DIRECTION OF EXCESS MAJOR SYSTEM FLOW AT LOW POINT
- LZ003NE LOW POINT
- Z003NE SUB-CATCHMENT ID
- 0.114 ha SUB-CATCHMENT AREA
- 93% TOTAL IMPERVIOUSNESS

Inlet Control Device (ICD) Type :

- ◻ Tempest A (19.9L/s)/Ex. IPEX A (19.7L/s)
- ◻ Tempest B (28.4L/s)/Ex. IPEX B (28.0L/s)
- ◻ Tempest C (35.5L/s)/Ex. IPEX C (36.5L/s)
- ◻ Tempest D (50.1L/s)/Ex. IPEX D (54.6L/s)
- ◻ Tempest E (69.1L/s)/Ex. IPEX F (77.2L/s)
- Custom Type G (100 L/s)
- Custom Type H (136 L/s)
- Custom Type I (160 L/s)
- Custom Type J (185 L/s)

SCALE :

0 10 20 30 40m



J.F. Sabourin & Associates Inc.
 WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS
 OTTAWA (613) 836-3884
 GATINEAU (819) 243-6858

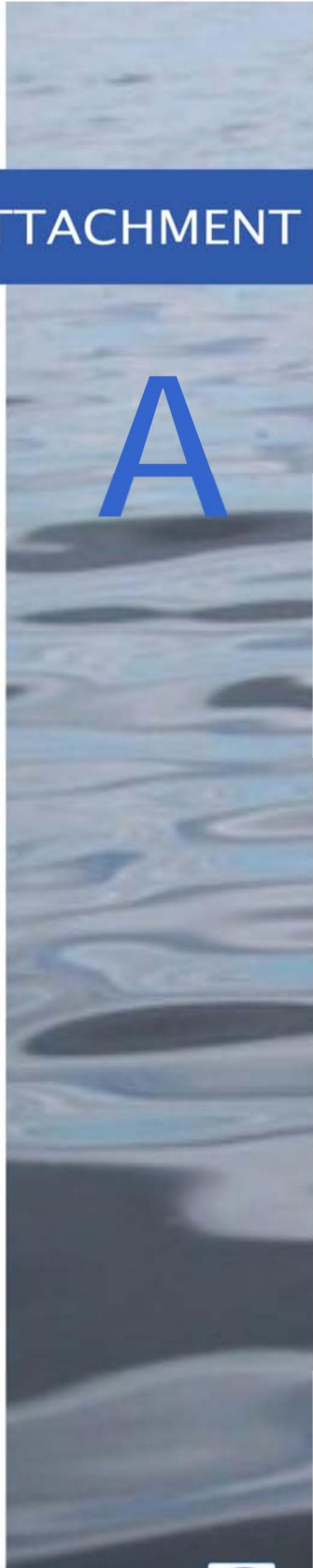
PROJECT :
 DIGRAM DEVELOPMENTS CALEDON INC.
 CONDO BLOCK 132

BY	DATE	DESCRIPTION	BY

PROPOSED MINOR AND MAJOR SYSTEM

FIGURE 1	DESIGNED:	
	DRAWN: LP	
	VERIFIED: JFS	
	APPROVED: JFS	
DRAWING REF.	DATE	PROJECT No.
552-05\201806 Block 132 Subm1\Design\CAD\JFSA Figures.dwg	Jul/18	552-05

ATTACHMENT



Pipe Data and Hydraulic Gradeline Results

XPSWMM Model Schematic

Table A-1: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2)

U/S MH	D/S MH	U/S Invert (m)	D/S Invert (m)	Pipe Dia. / Height (mm)	Pipe Width (mm)	Pipe Length (m)	Pipe Slope (%)	n	U/S MH Cover Elev. (m)	D/S MH Cover Elev. (m)	Design Vel. (m/s)	Design Flow (m³/s)	Peak Pipe Flow (m³/s)	Peak / Design Flow	Surcharge U/S ⁽¹⁾ (m)	Time to Peak (h)	Max. U/S HGL (m)	Max. D/S HGL (m)	Lot Number	BF (m)	Freeboard (m)	Interpolated HGL			
																						Length HGL (m)	Dist. From D/S MH (m)	HGL (m)	
T6	T7	269.774	269.644	300	N/A	13.0	1.0	0.013	272.613	272.453	1.368	0.097	0.079	0.8	0.006	1.367	270.080	270.003	72	271.49	1.180				
T6	T9	269.661	269.269	525	N/A	87.0	0.5	0.013	272.613	272.520	1.333	0.288	0.202	0.7	-0.106	1.367	270.080	269.951	74	271.18	0.870				
T7	T8	269.494	268.474	450	N/A	120.0	0.9	0.013	272.453	271.766	1.653	0.263	0.187	0.7	0.059	1.367	270.003	269.594	B128	270.36	0.127				
																				B128	270.36	0.531	120.2	1.4	269.599
																				63	270.47	0.641	120.2	1.4	269.599
																				64	270.55	0.629	120.2	28.4	269.691
																				65	270.55	0.629	120.2	28.4	269.691
																				66	270.65	0.640	120.2	54.6	269.780
																				67	270.70	0.690	120.2	54.6	269.780
																				68	270.70	0.604	120.2	80.0	269.866
																				69	270.84	0.744	120.2	80.0	269.866
																				70	270.84	0.654	120.2	106.3	269.956
																				71	270.89	0.704	120.2	106.3	269.956
																				98	271.09	0.926	120.2	99.8	269.934
																				99	271.09	0.926	120.2	99.8	269.934
																				100	270.99	0.916	120.2	73.6	269.844
																				101	270.82	0.746	120.2	73.6	269.844
																				102	270.72	0.735	120.2	47.4	269.755
																				103	270.64	0.655	120.2	47.4	269.755
																				104	270.58	0.683	120.2	21.5	269.667
																				105	270.40	0.503	120.2	21.5	269.667
T8	T12	268.244	267.881	600	N/A	33.0	1.1	0.013	271.766	271.539	2.278	0.644	0.395	0.6	0.750	1.333	269.594	269.387	Part4	270.20	0.376				
T9	T10	269.194	269.150	600	N/A	14.5	0.3	0.013	272.520	272.353	1.189	0.336	0.238	0.7	0.157	1.367	269.951	269.927	82	271.14	0.959				
T10	T11	269.100	268.125	600	N/A	97.5	1.0	0.013	272.353	271.833	2.172	0.614	0.415	0.7	0.227	1.367	269.927	269.521	B133	270.20	0.043				
																				B133	270.20	0.444	97.3	1.3	269.526
																				90	270.25	0.432	97.3	16.1	269.588
																				89	270.31	0.492	97.3	16.1	269.588
																				88	270.37	0.445	97.3	41.8	269.695
																				87	270.44	0.515	97.3	41.8	269.695
																				86	270.50	0.465	97.3	68.0	269.805
																				85	270.57	0.535	97.3	68.0	269.805
																				84	270.63	0.486	97.3	94.2	269.914
																				83	270.63	0.486	97.3	94.2	269.914
																				97	271.13	1.023	97.3	85.2	269.877
																				96	271.13	1.023	97.3	85.2	269.877
																				95	270.76	0.762	97.3	59.1	269.768
																				94	270.76	0.762	97.3	59.1	269.768
																				93	270.59	0.702	97.3	32.9	269.658
																				92	270.59	0.702	97.3	32.9	269.658
																				91	270.40	0.576	97.3	17.4	269.594
T11	T12	267.967	267.811	750	N/A	39.0	0.4	0.013	271.833	271.539	1.594	0.704	0.510	0.7	0.804	1.483	269.521	269.387	Part3	270.15	0.399				
T12	52	267.731	267.595	750	N/A	19.5	0.7	0.013	271.539	271.337	2.108	0.931	0.853	0.9	0.906	1.483	269.387	269.273	N/A	N/A	N/A				
T13	T14	269.222	268.800	600	N/A	105.5	0.4	0.013	272.254	272.129	1.373	0.388	0.227	0.6	-0.263	1.367	269.559	269.385	B123W	270.50	0.711				
T14	T15	268.750	268.442	600	N/A	44.0	0.7	0.013	272.129	271.890	1.817	0.514	0.333	0.6	0.035	1.350	269.385	269.278	Part14	270.22	0.605				
T15	T21	268.367	268.121	675	N/A	41.0	0.6	0.013	271.890	271.662	1.820	0.651	0.494	0.8	0.236	1.367	269.278	269.155	Part15W	270.06	0.552				
T16	T17	269.547	269.251	375	N/A	74.0	0.4	0.013	272.395	272.209	1.004	0.111	0.040	0.4	-0.215	1.300	269.707	269.406	112	271.11	1.173				
T17	T18	269.176	268.884	450	N/A	58.5	0.5	0.013	272.209	271.888	1.268	0.202	0.097	0.5	-0.220	1.350	269.406	269.294	Part41	270.50	0.864				
T18	T19	268.834	268.738	450	N/A	12.0	0.8	0.013	271.888	271.741	1.603	0.255	0.102	0.4	0.010	1.483	269.294	269.280	Part42	270.30	0.776				
T19	T20	268.663	268.465	525	N/A	36.0	0.6	0.013	271.741	271.527	1.473	0.319	0.141	0.4	0.092	1.483	269.280	269.255	Part46	270.21	0.700				
T20	T21	268.265	268.121	675	N/A	24.0	0.6	0.013	271.527	271.662	1.820	0.651	0.351	0.5	0.315	1.317	269.255	269.155	Part22	270.46	0.975				
T21	39	267.971	267.755	825	N/A	36.0	0.6	0.013	271.662	271.464	2.080	1.112	0.819	0.7	0.359	1.500	269.155	269.036	Part16W	269.91	0.525				
CMH3	4	264.692	264.635	375	N/A	11.5	0.5	0.013	268.180	268.203	1.123	0.124	0.049	0.4	-0.051	1.433	265.016	264.998	N/A	N/A	N/A				
Y1	Y2	265.512	265.222	300	N/A	29.0	1.0	0.013	268.294	268.509	1.368	0.097	0.098	1.0	-0.047	1.283	265.765	265.465	69	266.80	0.805				

Table A-1: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2)

U/S MH	D/S MH	U/S Invert (m)	D/S Invert (m)	Pipe Dia. / Height (mm)	Pipe Width (mm)	Pipe Length (m)	Pipe Slope (%)	n	U/S MH Cover Elev. (m)	D/S MH Cover Elev. (m)	Design Vel. (m/s)	Design Flow (m ³ /s)	Peak Pipe Flow (m ³ /s)	Peak / Design Flow	Surcharge U/S (1) (m)	Time to Peak (h)	Max. U/S HGL (m)	Max. D/S HGL (m)	Lot Number	BF (m)	Freeboard (m)	Interpolated HGL				
																						Length HGL (m)	Dist. From D/S MH (m)	HGL (m)		
																					BHL30	267.42	0.530	89.9	40.3	266.660
																					BGL29	267.45	0.511	89.9	53.5	266.709
																					BGL25	267.59	0.575	89.9	73.6	266.785
																					BJL45	267.71	0.698	89.9	73.0	266.782
																					BJL41	267.78	0.848	89.9	51.7	266.702
																					BIL40	267.78	0.909	89.9	35.4	266.641
																					BIL36	267.64	0.849	89.9	14.1	266.561
Z7	Z8	265.952	265.841	600	N/A	18.5	0.6	0.013	269.236	268.839	1.682	0.476	0.421	0.9	-0.044	1.333	266.508	266.266	BKL56	267.13	0.392					
																					BKL56	267.13	0.444	18.7	14.7	266.456
Z8	ZCMH1	265.761	265.443	600	N/A	26.5	1.2	0.013	268.839	268.463	2.379	0.673	0.555	0.8	-0.171	1.350	266.190	265.859	N/A	N/A	N/A					
ZCMH1	Y117	263.915	263.850	675	N/A	13.0	0.5	0.013	268.463	269.228	1.661	0.594	0.552	0.9	0.133	1.350	264.723	264.557	N/A	N/A	N/A					
MH2-1	MH2-3	265.117	264.955	300	N/A	22.2	1.0	0.013	269.280	269.080	1.368	0.097	-0.004	0.0	-0.248	1.267	265.169	265.167	FUT	267.71	2.311					
MH2-2	MH2-3	265.347	264.955	300	N/A	39.3	1.0	0.013	268.540	269.080	1.368	0.097	0.133	1.4	0.545	1.283	266.192	265.228	FUT	266.97	0.548					
MH2-3	MH2-4	264.875	264.840	375	N/A	7.1	0.5	0.013	269.080	268.960	1.123	0.124	0.133	1.1	-0.083	1.283	265.167	265.109	FUT	267.51	2.113					
MH2-4	MH2-5	264.765	264.499	450	N/A	53.1	0.5	0.013	268.960	268.550	1.268	0.202	0.157	0.8	-0.145	1.350	265.070	264.810	FUT	267.39	2.090					
MH2-5	MH2-6	264.449	264.397	450	N/A	10.4	0.5	0.013	268.550	268.460	1.268	0.202	0.186	0.9	-0.089	1.350	264.810	264.732	FUT	266.98	1.940					
MH2-6	MH2-7	264.347	264.320	450	N/A	5.4	0.5	0.013	268.460	268.420	1.268	0.202	0.198	1.0	-0.065	1.367	264.732	264.672	FUT	266.89	1.928					
MH2-7	MH1	264.270	264.130	450	N/A	28.1	0.5	0.013	268.420	268.180	1.268	0.202	0.234	1.2	-0.048	1.350	264.672	264.471	FUT	266.85	1.948					
MH1	Y3	263.830	263.779	600	N/A	10.3	0.5	0.013	268.180	268.054	1.536	0.434	0.233	0.5	-0.065	1.350	264.365	264.303	FUT	266.61	2.015					

Note: ⁽¹⁾ A negative surcharge implies that the pipe is not flowing full

266.886 Interpolated HGL elevation

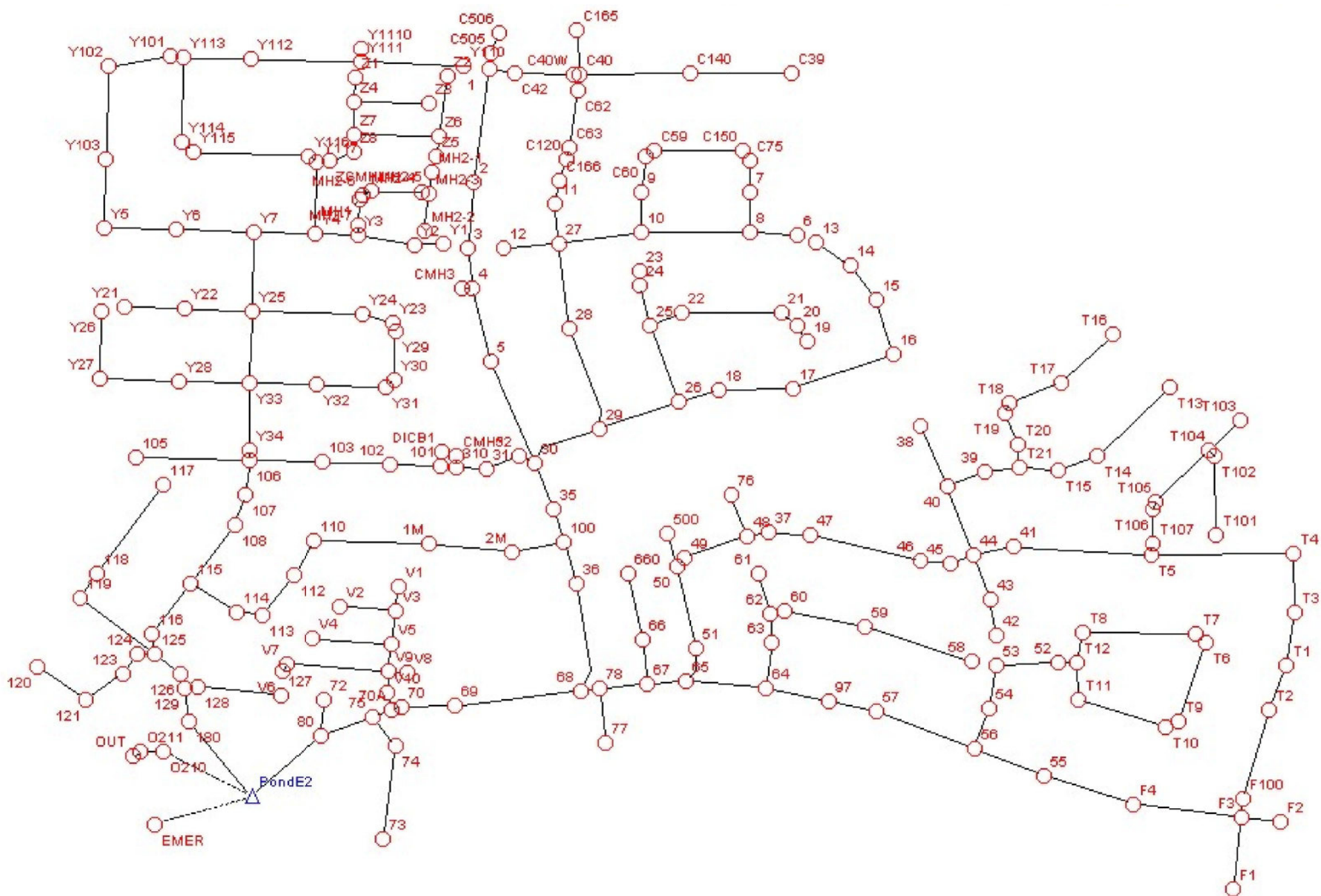
Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

U/S MH	D/S MH	U/S Invert (m)	D/S Invert (m)	Pipe Dia. / Height (mm)	Pipe Width (mm)	Pipe Length (m)	Pipe Slope (%)	n	U/S MH Cover Elev. (m)	D/S MH Cover Elev. (m)	Design Vel. (m/s)	Design Flow (m ³ /s)	Peak Pipe Flow (m ³ /s)	Peak / Design Flow	Surcharge U/S (1) (m)	Time to Peak (h)	Max. U/S HGL (m)	Max. D/S HGL (m)	Lot Number	BF (m)	Freeboard (m)	Interpolated HGL				
																						Length HGL (m)	Dist. From D/S MH (m)	HGL (m)		
																					BHL30	267.42	0.530	89.9	40.3	266.660
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Z7	Z8	265.952	265.841	600	N/A	18.5	0.6	0.013	269.236	268.839	1.682	0.476	0.421	0.9	-0.044	1.333	266.508	266.266	BKL56	267.13	0.392					
																					BKL56	267.13	0.444	18.7	14.7	266.456
Z8	ZCMH1	265.761	265.443	600	N/A	26.5	1.2	0.013	268.839	268.463	2.379	0.673	0.555	0.8	-0.171	1.350	266.190	265.859	N/A	N/A	N/A					
ZCMH1	Y117	263.915	263.850	675	N/A	13.0	0.5	0.013	268.463	269.228	1.661	0.594	0.555	0.9	0.017	1.350	264.607	264.341	N/A	N/A	N/A					
MH2-1	MH2-3	265.117	264.955	300	N/A	22.2	1.0	0.013	269.280	269.080	1.368	0.097	-0.004	0.0	-0.248	1.267	265.169	265.167	FUT	267.71	2.311					
MH2-2	MH2-3	265.347	264.955	300	N/A	39.3	1.0	0.013	268.540	269.080	1.368	0.097	0.133	1.4	0.545	1.283	266.192	265.228	FUT	266.97	0.548					
MH2-3	MH2-4	264.875	264.840	375	N/A	7.1	0.5	0.013	269.080	268.960	1.123	0.124	0.133	1.1	-0.083	1.283	265.167	265.109	FUT	267.51	2.113					
MH2-4	MH2-5	264.765	264.499	450	N/A	53.1	0.5	0.013	268.960	268.550	1.268	0.202	0.157	0.8	-0.145	1.350	265.070	264.810	FUT	267.39	2.090					
MH2-5	MH2-6	264.449	264.397	450	N/A	10.4	0.5	0.013	268.550	268.460	1.268	0.202	0.186	0.9	-0.089	1.350	264.810	264.732	FUT	266.98	1.940					
MH2-6	MH2-7	264.347	264.320	450	N/A	5.4	0.5	0.013	268.460	268.420	1.268	0.202	0.198	1.0	-0.065	1.367	264.732	264.672	FUT	266.89	1.928					
MH2-7	MH1	264.270	264.130	450	N/A	28.1	0.5	0.013	268.420	268.180	1.268	0.202	0.234	1.2	-0.048	1.350	264.672	264.471	FUT	266.85	1.948					
MH1	Y3	263.830	263.779	600	N/A	10.3	0.5	0.013	268.180	268.054	1.536	0.434	0.234	0.5	-0.142	1.367	264.288	264.183	FUT	266.61	2.092					

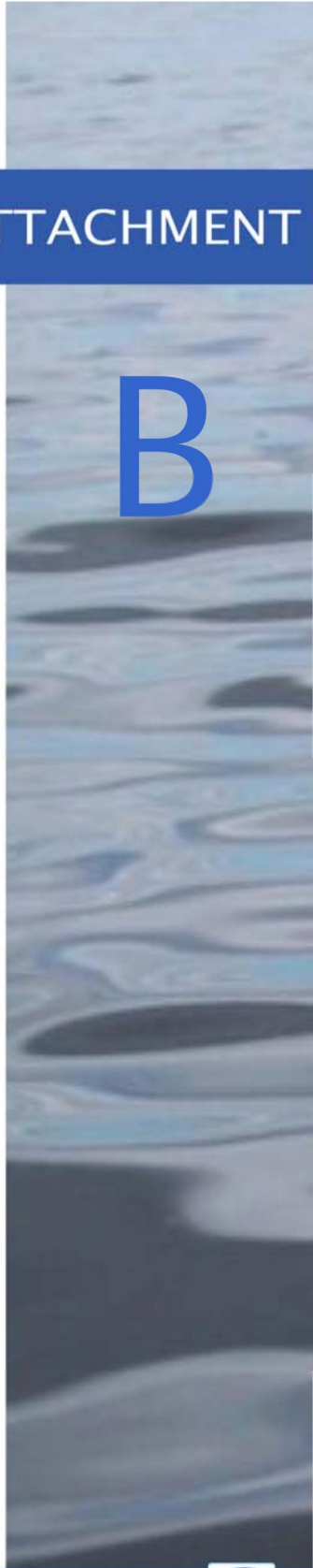
Note: ⁽¹⁾ A negative surcharge implies that the pipe is not flowing full

266.886 Interpolated HGL elevation

Figure A-1: XPSWMM MODEL SCHEMATIC



ATTACHMENT



B

Calculation Sheets

Feb 16, 2022

Calculation Sheet B-1: Flow Depth and Spread at Location with Highest Peak Flow on a Typical Street

Sub-catchment(s)		M075NE and M075SE	Ultimate Conditions
Location		Dougall Avenue	12.5 m wide road
Q _{M075NE}		2.593	
Q _{M075SE}		0.764	
Q _{combined} ⁽²⁾		3.357	for 100-Year Storm
Tr	(m)	6.25	
So	(m/m)	0.005	
W	(m)	0.000	
Sw	(m/m)	0.000	
T	(m)	8.257	
Sx	(m/m)	0.03	
n _{road}		0.013	
dc	(m)	0.15	
Se	(m/m)	0.03	
n _{shoulder}		0.025	
dw	(m)	0.000	
Ts	(m)	8.257	
ds	(m)	0.248	
d	(m)	0.248	
d _{crown}	(m)	0.187	
dd	(m)	0.060	dd < 0.15 m, the max. depth over road crown of an arterial road
de	(m)	0.098	
Te	(m)	3.257	Flow is contained within ROW
Q _{area(A+B)}	(m ³ /s)	0.000	
Q _{area(B)}	(m ³ /s)	0.000	
Q _{area(A)}	(m ³ /s)	0.000	
Q _{area(B+C+D)}	(m ³ /s)	1.645	
Q _{area(D)}	(m ³ /s)	0.038	
Q _{area(B+C)}	(m ³ /s)	1.607	
Q _{area(E)}	(m ³ /s)	0.072	
Q _{area(A+B+C+E)}	(m ³ /s)	1.678	
Q _{two sides}	(m ³ /s)	3.357	
d _{Flow} ⁽³⁾	(m)	0.248	d _{flow} < 0.30 m, the maximum allowable depth of flow
A _{flow two sides}	(m ²)	2.243	
v	(m/s)	1.497	
vxd	(m ² /s)	0.371	vxd < 0.65 m ² /s

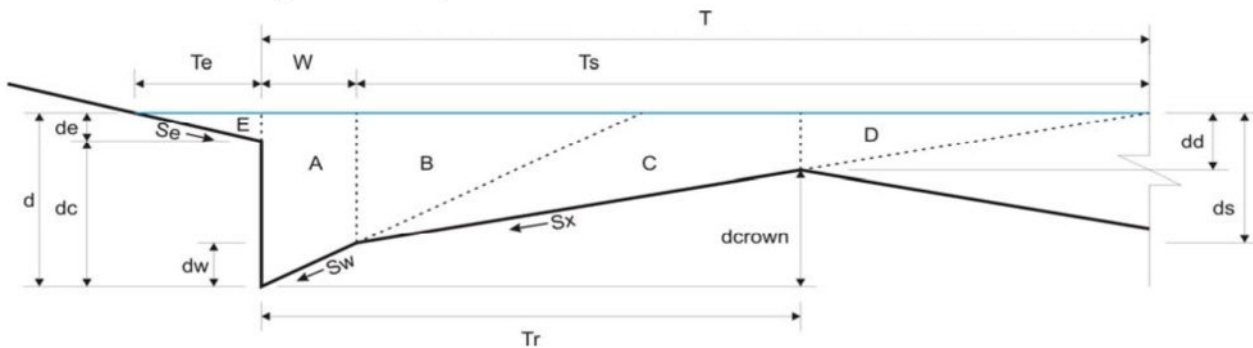
Notes:

(1) 100-year flow from DDSWMM model (Chicago storm).

(2) The computations assume that the total incoming flow is equally divided on both sides on the road.

(3) Computations based on methodology described in MTO Drainage Management Manual, 1997, Ch.4, pp. 59-60.

So is the longitudinal road slope



Equations:

$$Q_{\text{area(A+B)}} = 0.375 \times So^{0.5} \times d^{2.667} / (n_{\text{road}} \times Sw)$$

$$Q_{\text{area(B)}} = 0.375 \times So^{0.5} \times (ds)^{2.667} / (n_{\text{road}} \times Sw)$$

$$Q_{\text{area(B+C+D)}} = 0.375 \times So^{0.5} \times (ds)^{2.667} / (n_{\text{road}} \times Sx)$$

$$Q_{\text{area(D)}} = 0.375 \times So^{0.5} \times (dd)^{2.667} / (n_{\text{road}} \times Sx)$$

$$Q_{\text{area(E)}} = 0.375 \times So^{0.5} \times (de)^{2.667} / (n_{\text{shoulder}} \times Se)$$

CALCULATION SHEET B-2A : POND E2 EAST OVERLAND FLOW ROUTE

EAST OVERLAND FLOW ROUTE AT DOUGALL AVENUE (NORTH SIDE OF POND) - CURB CUT WEIR

Approaching flow =	2.960 m ³ /s	for 100-yr event (on MAJ road segment)
Curb cut width =	15.5 m	as per DSEL grading plan
Curb cut height =	0.050 m	as per DSEL
Maximum flow depth at gutter =	0.293 m	(0.15 m +0.03×4.75 m = 0.293 m for flow contained within RW)
Average head of water over curb cut =	0.243 m	(0.15 m +0.03×4.75 m - 0.050 m = 0.243 m)
Curb cut weir coefficient =	1.84	
Maximum flow through cub cut =	3.406 m ³ /s	for 100-yr event

Therefore the capacity of the curb cut (3.406 m³/s) is higher than the computed overland flow (2.960 m³/s)

EAST OVERLAND FLOW ROUTE DOWNSTREAM OF CURB CUT (NORTH SIDE OF POND)

$Q = 1/n \times AR^{2/3} S^{1/2}$

	Min. Slope		Max. Slope
normal depth =	0.224	m	0.074
n =	0.03		0.03
Channel width =	15.5	m	15.5
A (area of flow) =	3.471	m ²	1.144
wetted perimeter =	15.948	m	15.648
R (hydraulic radius) =	0.218	m	0.073
S (slope) =	0.005	m/m	0.197
Q (flow) =	2.960	m ³ /s	2.960
velocity =	0.85	m/s	2.59

CALCULATION SHEET B-2B : POND E2 WEST OVERLAND FLOW ROUTE

WEST OVERLAND FLOW ROUTE AT DOUGALL AVENUE (NORTH SIDE OF POND) - CURB CUT WEIR

Approaching flow =	1.805 m ³ /s	for 100-yr event (on MAJ road segment)
Curb cut width =	15.5 m	as per DSEL grading plan
Curb cut height =	0.050 m	as per DSEL
Maximum flow depth at gutter =	0.293 m	(0.15 m +0.03×4.75 m = 0.293 m for flow contained within RW)
Average head of water over curb cut =	0.243 m	(0.15 m +0.03×4.75 m - 0.050 m = 0.243 m)
Curb cut weir coefficient =	1.84	
Maximum flow through cub cut =	3.406 m ³ /s	for 100-yr event

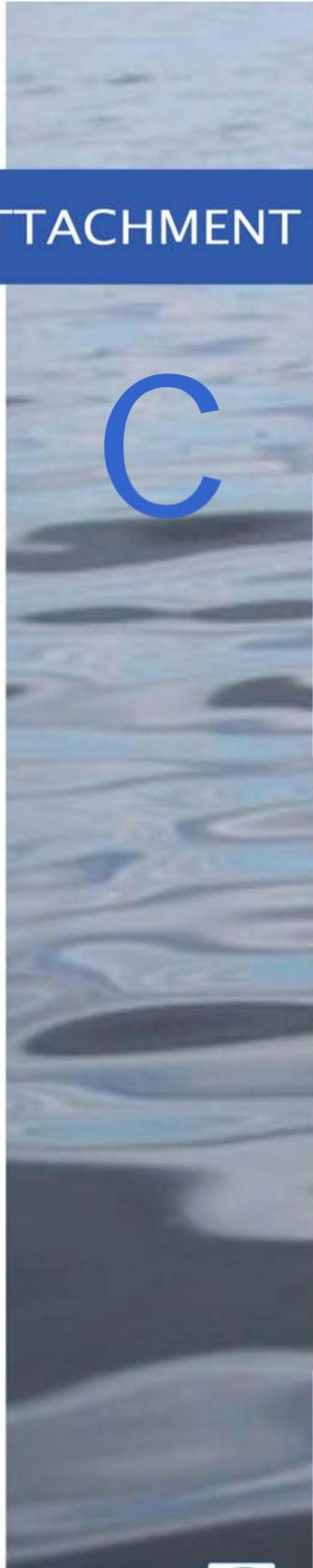
Therefore the capacity of the curb cut (3.406 m³/s) is higher than the computed overland flow (1.808 m³/s)

WEST OVERLAND FLOW ROUTE DOWNSTREAM OF CURB CUT (NORTH SIDE OF POND)

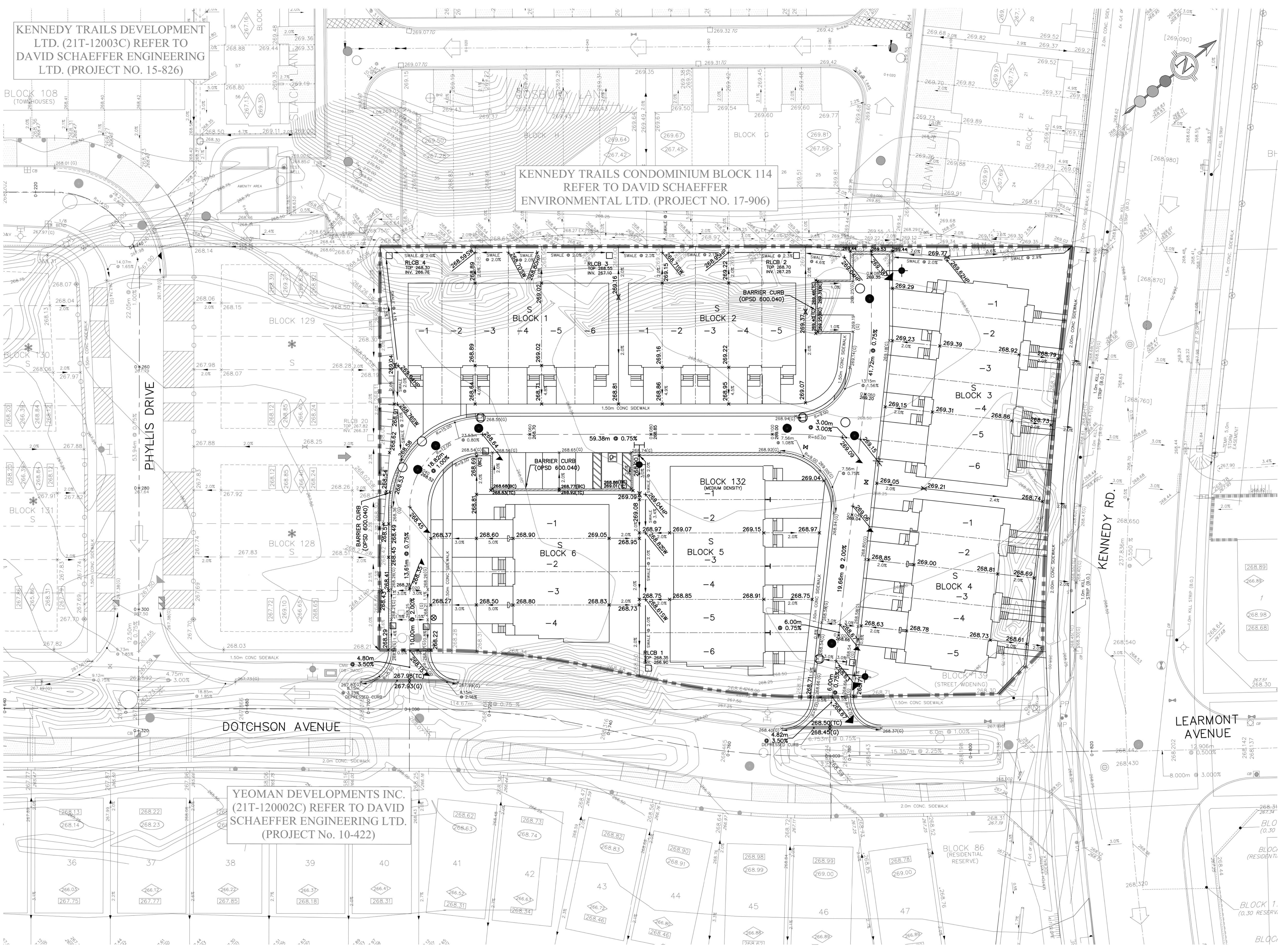
$Q = 1/n \times AR^{2/3} S^{1/2}$

	Min. Slope		Max. Slope
normal depth =	0.166	m	0.056
n =	0.03		0.03
Channel width =	15.5	m	15.5
A (area of flow) =	2.572	m ²	0.867
wetted perimeter =	15.832	m	15.612
R (hydraulic radius) =	0.162	m	0.056
S (slope) =	0.005	m/m	0.184
Q (flow) =	1.805	m ³ /s	1.805
velocity =	0.70	m/s	2.08

ATTACHMENT



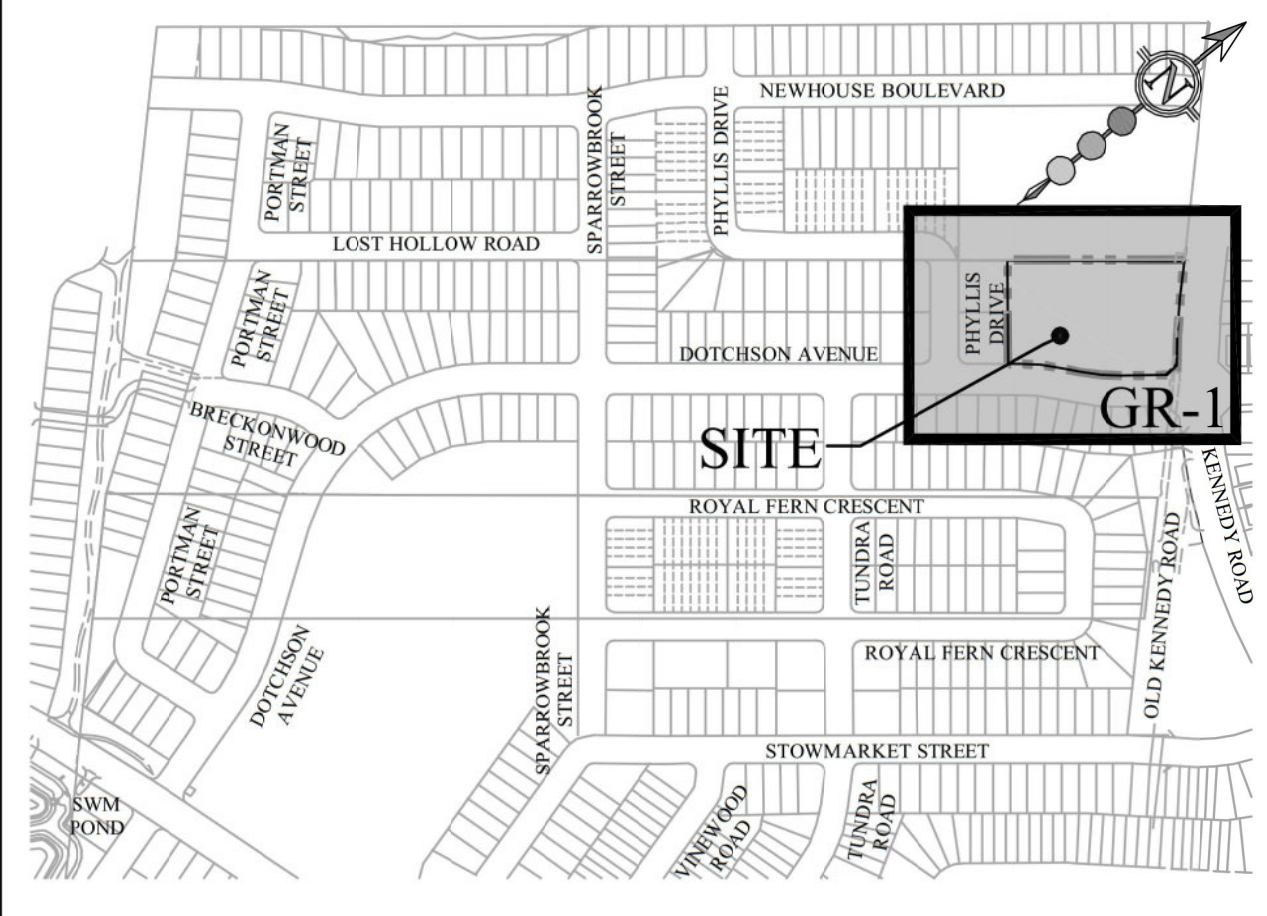
Drawings GR-1, S-1 and STM-1
(July 2018, SCS Consulting Group Limited)



KENNEDY TRAILS DEVELOPMENT LTD. (21T-12003C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 15-826)

KENNEDY TRAILS CONDOMINIUM BLOCK 114 REFER TO DAVID SCHAEFFER ENVIRONMENTAL LTD. (PROJECT NO. 17-906)

YEOMAN DEVELOPMENTS INC. (21T-12002C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT No. 10-422)



KEY PLAN
N.T.S.

BENCHMARK: ELEV. 259.419

LEGEND:

- LIMIT OF PROPERTY
- SANITARY MANHOLE
- STORM MANHOLE
- EXISTING/FUTURE SANITARY MANHOLE
- EXISTING/FUTURE STORM MANHOLE
- SINGLE CATCHBASIN WITH NO ICD
- SINGLE CATCHBASIN WITH 19'S L/S (TYPE 'A') INLET CONTROL DEVICE (ICD)
- DOUBLE CATCHBASIN WITH NO ICD
- EXISTING SINGLE CATCHBASIN
- EXISTING DOUBLE CATCHBASIN
- FUTURE SINGLE CATCHBASIN
- FUTURE DOUBLE CATCHBASIN
- HYDRANT AND VALVE
- EXISTING/FUTURE HYDRANT AND VALVE
- WATERMAIN VALVE BOX
- 25mm WATER SERVICE BOX LOCATION (WATER SERVICE AT 90° TO WATERMAIN UNLESS OTHERWISE SHOWN)
- WATERMAIN VALVE CHAMBER
- LOT/BLOCK NUMBER
- FUTURE / EXISTING LOT NUMBER
- EXISTING CONTOUR AND ELEVATION
- FUTURE ELEVATION
- EXTERNAL DESIGN ELEVATION
- EXISTING ELEVATION
- PROPOSED ELEVATION
- PROPOSED GUTTER ELEVATION
- PROPOSED SWALE ELEVATION
- FRONT DRAINING LOT
- SPLIT DRAINAGE LOT
- TRANSITION TYPE LOT
- ENGINEERED FILL LOTS
- MINIMUM BASEMENT ELEVATION
- OVERLAND FLOW
- 100-YEAR EMERGENCY OUTLET
- PROPOSED EMBANKMENT (MAX. 4:1 UNLESS OTHERWISE NOTED)
- BARRIER CURB (AS PER OPSD 600.040)
- EXISTING/FUTURE DRIVEWAY LOCATION
- COMMUNITY MAILBOX PAD, SEE DETAILS ON 900 SERIES DRAWINGS
- CANADA POST SPEC. ELEV-ENG-##
- NUMBER OF MODULES
- FRONT/REAR HOUSE ELEVATION
- FINISHED FLOOR ELEVATION
- BASEMENT SLAB ELEVATION

TOPOGRAPHIC SURVEY PROVIDED BY HOLDING JONES VANDERVEEN INC., JANUARY 2016

REVISIONS			
No.	DESCRIPTION	DATE	BY / APPROVED

SGS consulting group Ltd
30 CENTURIAN DRIVE, SUITE 100
MARKHAM, ONTARIO L3R 8B8
TEL: (905) 475-1900
FAX: (905) 475-8335

TOWN OF CALEDON
6311 OLD CHURCH ROAD
CALEDON, ONTARIO L7C 1J6
TEL: (905) 884-2272
FAX: (905) 884-4325

DIGRAM DEVELOPMENTS
CALEDON INC.
CALEDON - CONDO BLOCK 132
21T-14002C (T-14002Ca)

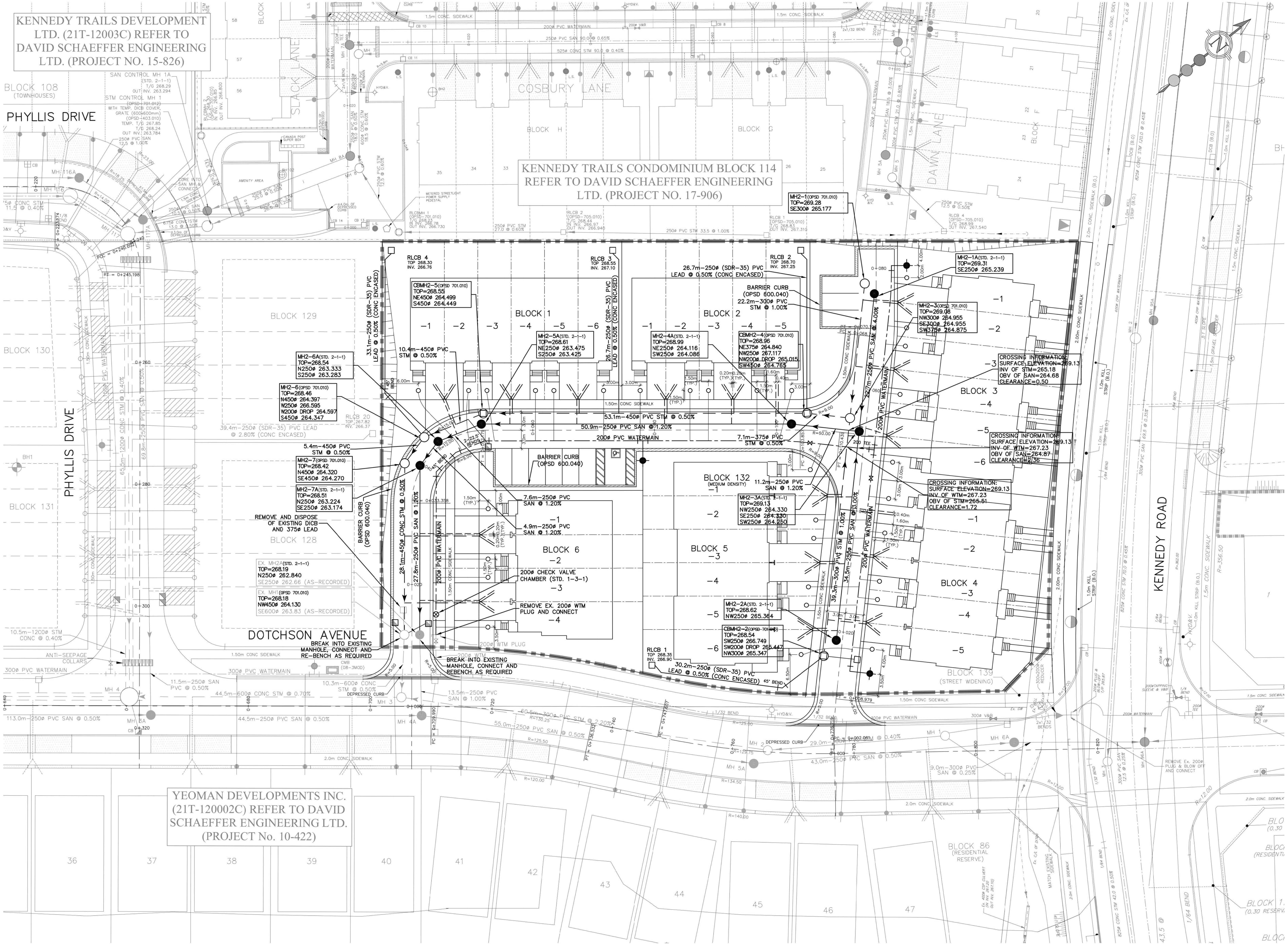
GRADING PLAN

DATE: JULY 2018	DESIGNED BY: D.W./C.J.C.	CHECKED BY: M.R.C./S.M.S.
SCALE: 1:300	DRAWN BY: J.Y.L.	CHECKED BY: P.A.S.

APPROVED FOR CONSTRUCTION
THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CERTIFY DIMENSIONAL ACCURACY.

THIS APPROVAL IS SUBJECT TO THE FURTHER CERTIFICATION OF THE "AS-CONSTRUCTED" WORKS BY A REGISTERED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO

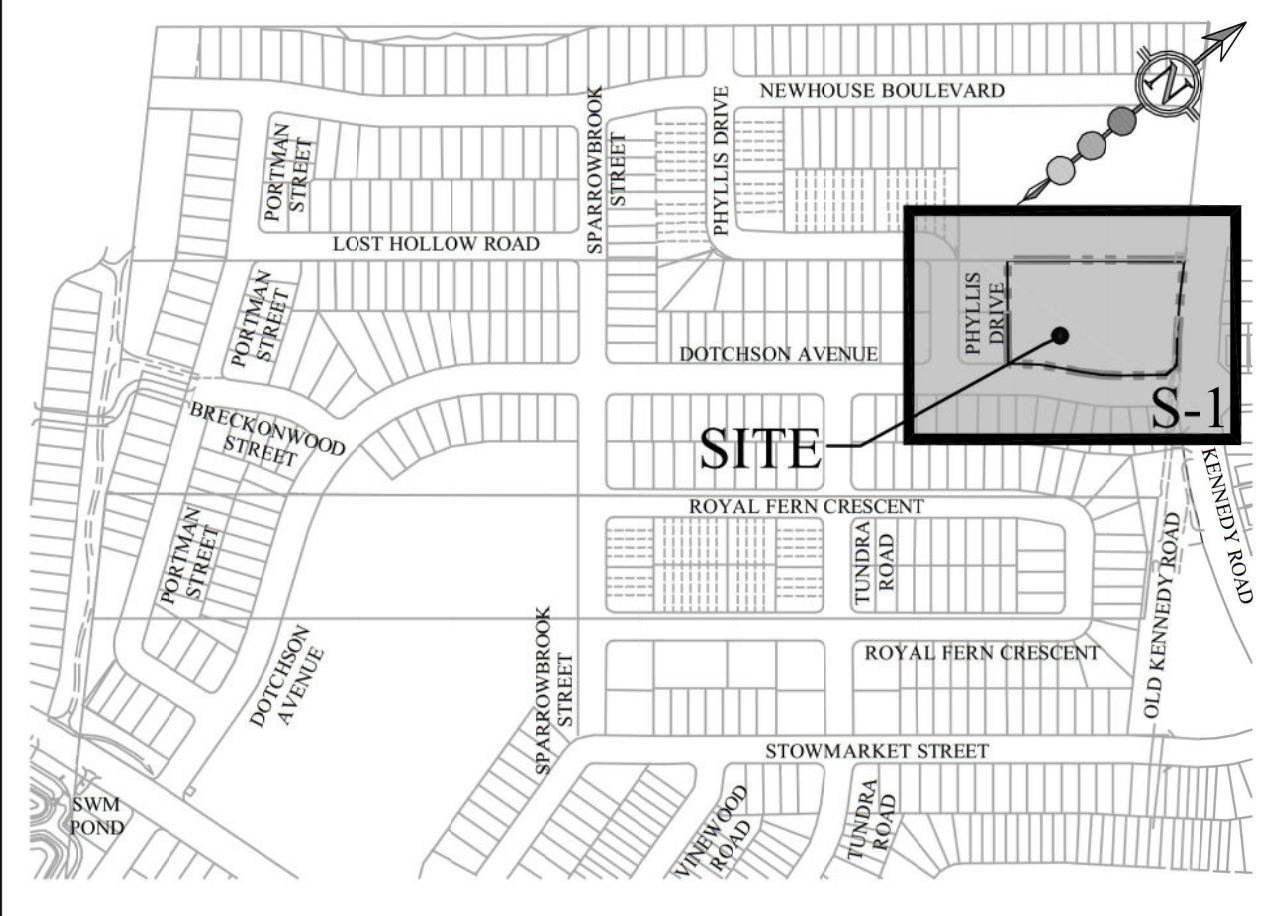
DATE: _____ APPROVED BY: _____ PROJECT NO: **1564**
DRAWING NO: **GR-1**



KENNEDY TRAILS DEVELOPMENT LTD. (21T-12003C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 15-826)

KENNEDY TRAILS CONDOMINIUM BLOCK 114 REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 17-906)

YEOMAN DEVELOPMENTS INC. (21T-120002C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT No. 10-422)



BENCHMARK:
PEEL BENCHMARK NO. 56. **ELEV. 259.419**

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 - STORM SEWER/MANHOLE
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 - EXISTING SINGLE CATCHBASIN
 - EXISTING DOUBLE CATCHBASIN
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 - FUTURE DOUBLE CATCHBASIN
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 - WATERMAIN VALVE BOX
 - EXISTING/FUTURE VALVE AND BOX
 - CHECK VALVE CHAMBER (STD. 1-34)
 - EXISTING VALVE CHAMBER
 - EXISTING HYDRO POLE/LAMP STANDARDS
 - 93 LOT/BLOCK NUMBER
 - 65 EXISTING/FUTURE LOT NUMBER
 - EXISTING/FUTURE WATER SERVICE BOX LOCATION
 - 25mm WATER SERVICE BOX LOCATION (WATER SERVICE AT 90° TO WATERMAIN UNLESS OTHERWISE SHOWN)
 - TYPICAL DUAL SERVICE CONNECTION INCLUDES ONE DOUBLE STORM CONNECTION AND ONE DOUBLE SANITARY CONNECTIONS AT 90° TO SEWERS AS PER REGION OF PEEL STANDARD DWG 2-4-3 (UNLESS OTHERWISE SHOWN)
 - SINGLE STORM AND SINGLE SANITARY CONNECTIONS
 - COMMUNITY MAILBOX. PAD, SEE DETAILS ON 900 SERIES DRAWINGS
 - CANADA POST SPEC. #10X-ENG-# NUMBER OF MODULES
 - BARRIER CURB (AS PER OPSD 600.040)
 - EXISTING/FUTURE DRIVEWAY LOCATION
 - BOREHOLE LOCATION

TOPOGRAPHIC SURVEY PROVIDED BY HOLDING JONES VANDERVEEN INC., JANUARY 2016

REVISIONS			
No.	DESCRIPTION	DATE	BY / APPROVED

SGS consulting group Ltd
30 CENTURIAN DRIVE, SUITE 100
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FAX: (905) 475-8335

TOWN OF CALEDON
6311 OLD CHURCH ROAD
CALEDON, ONTARIO L7C 1P6
TEL: (905) 584-2272
FAX: (905) 584-4325

DIGRAM DEVELOPMENTS
CALEDON INC.
CALEDON - CONDO BLOCK 132
21T-14002C (T-14002Ca)

SERVICING PLAN

DATE: JULY 2018	DESIGNED BY: D.W./C.J.C.	CHECKED BY: M.R.C./S.M.S.
SCALE: 1:300	DRAWN BY: J.Y.L.	CHECKED BY: P.A.S.

APPROVED FOR CONSTRUCTION:
THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CERTIFY DIMENSIONAL ACCURACY.

PROJECT No: **1564**

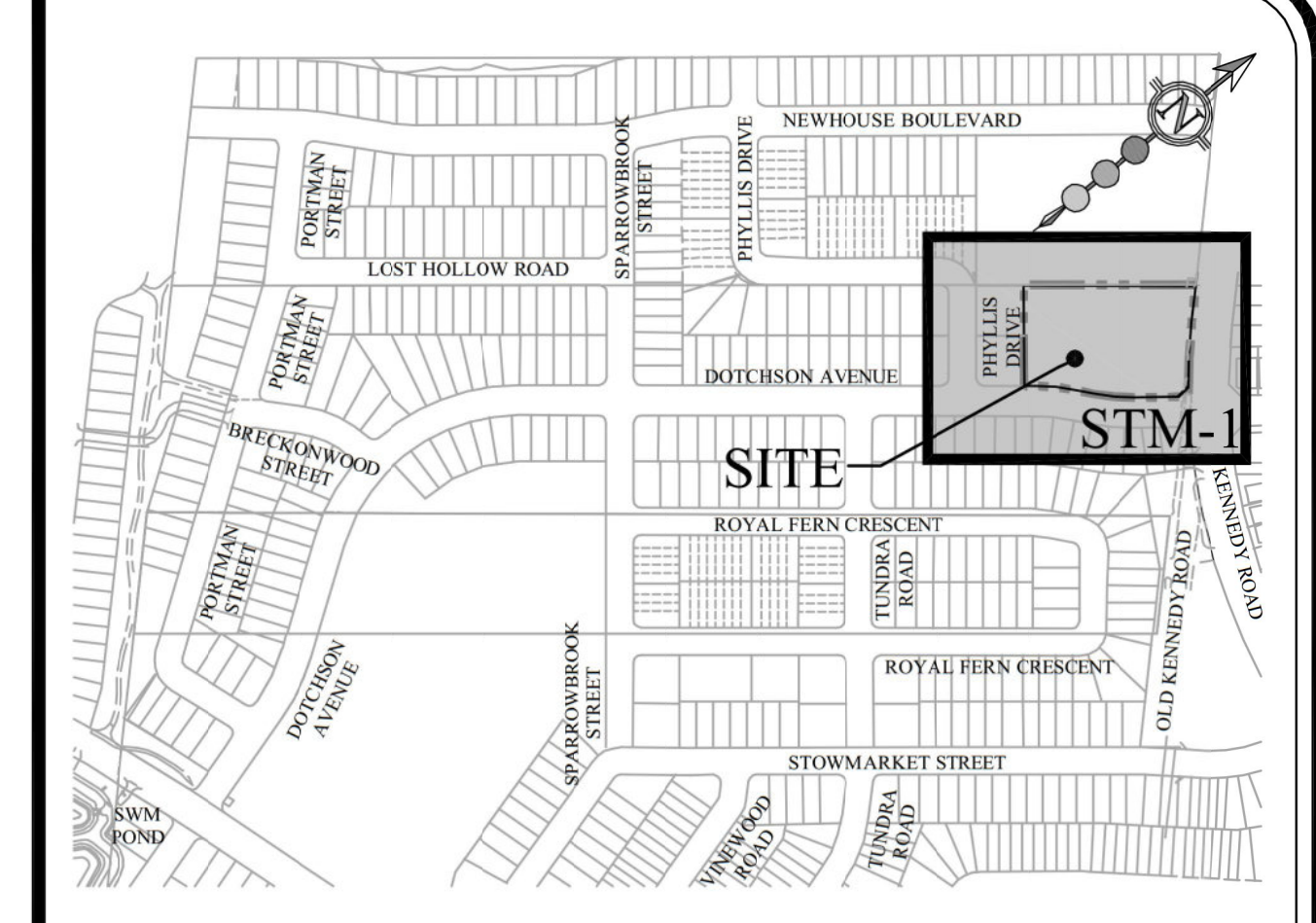
DRAWING No: **S-1**

DATE: _____ APPROVED BY: _____

KENNEDY TRAILS DEVELOPMENT LTD. (21T-12003C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 17-906)

KENNEDY TRAILS CONDOMINIUM BLOCK 114 REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT NO. 17-906)

YEOMAN DEVELOPMENTS INC. (21T-120002C) REFER TO DAVID SCHAEFFER ENGINEERING LTD. (PROJECT No. 10-422)



KEY PLAN
N.T.S.

BENCHMARK: PEEL BENCHMARK NO. 56. **ELEV. 259.419**

- LEGEND:**
- STORM MANHOLE
 - EXISTING/FUTURE STORM MANHOLE
 - EXISTING/FUTURE SINGLE CATCHBASIN
 - EXISTING/FUTURE DOUBLE CATCHBASIN
 - SINGLE CATCHBASIN WITH NO ICD
 - SINGLE CATCHBASIN WITH 19.8 L/S (TYPE 'A') INLET CONTROL DEVICE (ICD)
 - DOUBLE CATCHBASIN WITH NO ICD
 - SINGLE SERVICE CONNECTION
 - DUAL SERVICE CONNECTIONS
 - LOT NUMBER
 - EXISTING/FUTURE LOT NUMBER
 - LIMIT OF PROPERTY
 - DRAINAGE BOUNDARY
 - EXISTING/FUTURE DRAINAGE BOUNDARY
 - DRAINAGE AREA (HECTARES)
 - RUNOFF COEFFICIENT
 - EXISTING DRAINAGE AREA (HECTARES)
 - RUNOFF COEFFICIENT
 - OVERLAND FLOW ROUTE
 - EXISTING/FUTURE OVERLAND FLOW ROUTE
 - EXISTING CONTOUR
 - 100 YEAR FLOW INFORMATION FROM STORMWATER MANAGEMENT REPORT FOR KENNEDY TRAILS DEVELOPMENT LTD. PREPARED BY J.F. SABOURIN AND ASSOCIATES INC., DATED FEBRUARY 2016. EXTERNAL DRAINAGE AREA, EQUIVALENT RUNOFF AND TIME OF CONCENTRATION INFORMATION FROM 10 YEAR DESIGN SHEET FOR KENNEDY TRAILS DEVELOPMENT LTD., DATED FEBRUARY 2016. YEOMAN DEVELOPMENTS INC., DATED APRIL 2015. MISCORP DEVELOPMENT INC., DATED OCTOBER 2013. PREPARED BY DAVID SCHAEFFER ENGINEERING LIMITED.
 - EXTERNAL DRAINAGE AREA (ha)
 - EQUIVALENT RUNOFF COEFFICIENT
 - TIME OF CONCENTRATION (MIN.)
 - 100-YEAR FLOW

TOPOGRAPHIC SURVEY PROVIDED BY HOLDING JONES VANDERVEEN INC., JANUARY 2016

REVISIONS			
No.	DESCRIPTION	DATE	BY / APPROVED

SGS consulting group Ltd
30 CENTURIAN DRIVE, SUITE 100
MARKHAM, ONTARIO L3R 8B8
TEL: (905) 475-1900
FAX: (905) 475-8335

TOWN OF CALEDON
6311 OLD CHURCH ROAD
CALEDON, ONTARIO L7C 1P6
TEL: (905) 584-2272
FAX: (905) 584-4325

DIGRAM DEVELOPMENTS
CALEDON INC.
CALEDON - CONDO BLOCK 132
21T-14002C (T-14002Ca)

STORM DRAINAGE PLAN

DATE: JULY 2018	DESIGNED BY: D.W./C.J.C.	CHECKED BY: M.R.C./S.M.S.
SCALE: 1:300	DRAWN BY: J.Y.L.	CHECKED BY: P.A.S.

APPROVED FOR CONSTRUCTION
THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CERTIFY DIMENSIONAL ACCURACY.

PROJECT No: **1564**

THIS APPROVAL IS SUBJECT TO THE FURTHER CERTIFICATION OF THE "AS-CONSTRUCTED" WORKS BY A REGISTERED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO

DRAWING No: **STM-1**

DATE: _____ APPROVED BY: _____

ATTACHMENT B

RELEVANT REPORT EXCERPTS



February 18, 2014

Report No. 130177-H4

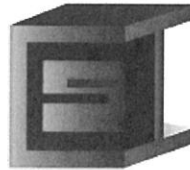
North Star Investments
1862 Albion Road
Rexdale, Ontario
M9W 5T2

Attention: Mr. Tony Ferrara

**HYDROGEOLOGIC STUDY FOR
PROPOSED RESIDENTIAL SUBDIVISION
12654, 12728 AND 12738 KENNEDY ROAD
TOWN OF CALEDON, ONTARIO**

Prepared for:

North Star Investments



CANADA ENGINEERING SERVICES INC.

39 Davisbrook Blvd., Scarborough

Toronto, Ontario M1T 2H6

Phone 416 492 4000

Fax 416 492 4001

Email cesi@cesi.ca

Hydrogeologic Study
12654, 12728 and 12738 Kennedy Road,
Town of Caledon, Ontario

February 18, 2014

130177-H4

Underlying the till unit are silty fine sands which have been encountered at depths of 3.0 m to 4.9 m below grade and extended down to the bases of all boreholes at a depth of 6.6 m. These silty fine sand deposits are very dense and are confined under hydrostatic pressure with water levels when allowed to slowly rise, reaching the existing ground surface.

2.2 TOPOGRAPHY

The site is located at 12654, 12728 and 12738 Kennedy Road, Town of Caledon, Ontario, in a predominantly residential area with recently built residential houses and new residential subdivision sites surrounding it.

For convenience Kennedy Road is taken as running in a North-South direction for this report. The site is bounded by Kennedy Road on the east side, new residential subdivision sites on the north, south and west sides. Further east, beyond the Kennedy Road are newly built residential houses and further north beyond the residential subdivision site are vacant farm lands.

The subject lands are gently sloping toward the rear of the subject property in a southwest direction. The site is currently a vacant field with forested lands in the middle portion of 12728 Kennedy Road which at the time of the investigation was almost fully covered with snow. Generally surface water flow is expected to flow naturally toward the south, draining into tributaries of the Etobicoke Creek and is then carried to Lake Ontario, via Etobicoke Creek. Groundwater level in this surficial layer generally parallels the ground surface at depths varying from 0.6 m to 1.5 m in all the boreholes. These groundwater levels more or less parallel the contour of the land.

The project area is relatively flat with a gradual slope to the southwest of the site. The elevation of the site ranges from a maximum of approximately 267 m to the northeast to approximately 261 m to the southwest. Site topography is somewhat influenced by two man-made ponds on the subject property. Based on the aerial photographs, it has been established that these ponds did not exist on the site prior to 1988. Surface water drainage from the site is primarily from ground surface to Etobicoke Creek to the south of the site, and then to Lake Ontario.

Due to the fair permeability surficial silty sand at the site, surface water infiltration will tend to be significantly high following rainfall events. The coefficient of permeability of this surficial silty sand at the site is expected to be around 10^{-3} cm/second.

3.0 HYDROGEOLOGIC PROCEDURE

The field work for the boreholes was carried out with two track-mounted drill rigs with continuous flight solid stem auger equipment on December 23, 2013 and on January 2, 2014. It was supervised by an engineer from our office. A total of eight boreholes were put down over the entire site.

Hydrogeologic Study
12654, 12728 and 12738 Kennedy Road,
Town of Caledon, Ontario

February 18, 2014

130177-H4

Each of the eight boreholes was put down at the site to a depth of 6.6 m and one piezometer was placed in each borehole. From the boreholes, soil samples were taken at 500 mm intervals between the ground surface and a depth of 3.0 m and thereafter at depths of 1.5 m to the termination of the boreholes.

The samples were taken by means of a split-spoon sampler, in accordance with the requirements of the Standard Penetration Test, (CSA test specifications A119.1).

All the samples taken were brought back to our laboratory where moisture content tests, three grain size analysis tests and further visual observations were carried out. Our field and laboratory findings are plotted on the borehole log numbers 1 to 8.

The locations and elevations of the boreholes were established by the property owner's surveyor, Calder Engineering Limited and are shown on drawing numbers 1 and 2. The geotechnical terms and symbols used in this report are shown in Appendix "A".

3.1 HYDROGEOLOGY

Regional ground water flow at the site is directed to the south, toward Lake Ontario, through the Etobicoke Creek. There are essentially three ground water flow systems present across the site. A shallow unconfined flow system within the medium permeability soils near the surface and a confined slow moving system consisting of silty fine sand, trapped by the dense sand till above. A deeper aquifer system below the silty fine sand within the bedrock at a depth of around 20 m to 30 m used for private well water supplies. Our boreholes do not extend to this aquifer system, but this data was obtained from MOE well records and this final flow system is not expected to be affected by the development of the subdivision. Some groundwater recharge is also expected to occur upstream from higher lands.

All boreholes encountered water in the surficial silty sand layer which was close to the ground surface. No water or silt or sand seams were found within the silty sand till, though they may be present between soil samples.

All boreholes were terminated within the very dense silty fine sand. Piezometers installed in this stratum and sealed in the relatively impermeable silty sand till rose slowly in the piezometers to close to ground surface, indicating that this is a confined aquifer.

Water infiltration and recharge rates for the site and surrounding areas are well covered in a report prepared by Shaheen and Peaker Limited for the site and surrounding areas, referred to as the Mayfield Community. They estimated a recharge rate for the area on the order of 150 mm/a.

Hydrogeologic Study
 12654, 12728 and 12738 Kennedy Road,
 Town of Caledon, Ontario

February 18, 2014

130177-H4

Detailed soil descriptions are shown on the borehole log numbers 1 to 8. Grain size analysis curves of the different soil types found at the site were plotted and are shown in Figure No. 1.

3.3 STRATIGRAPHY

The site consists mainly forested and grass lands with residential houses in front. A defined topsoil layer was found at the surface of all the boreholes, This was followed by a layer of silty sand, which in some places was reworked or relocated at the site.

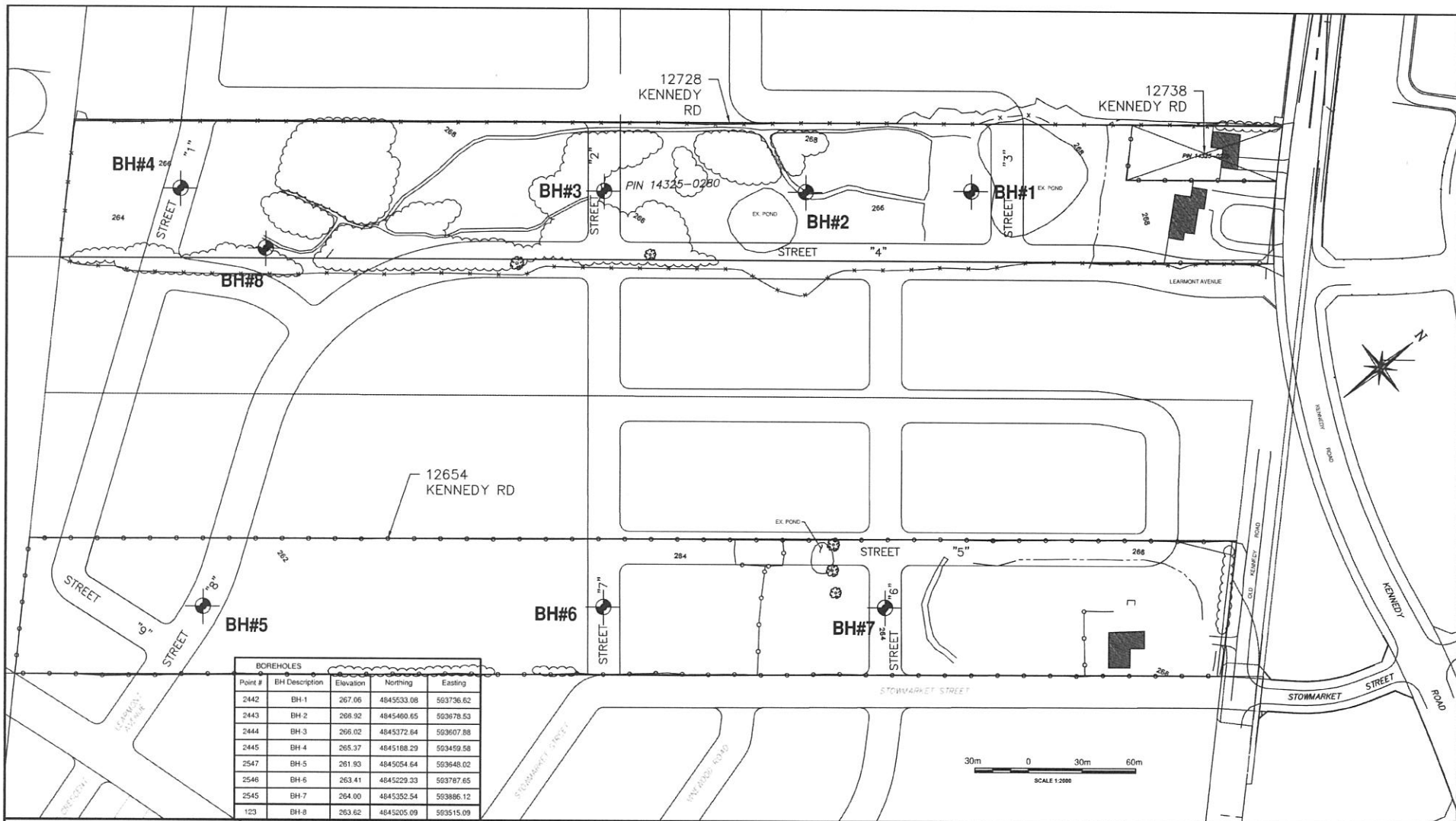
Below this was a very dense silty sand till. This in turn was underlain by a very dense silty fine sand, confined aquifer where all the boreholes were terminated at a depth of 6.6 m.

3.4 GROUND AND SURFACE WATER FLOW

One piezometer was installed in each of the boreholes and was extended down close to the base of the borehole. The piezometers were sealed in the dense silty sand till and at the surface and therefore the water levels obtained from these piezometers were of the confined very dense silty fine sand below the silty sand till. Water levels in borehole numbers 1, 2, 3, 4, 5 and 6 were at depths of 1.8 m (elev 282.28 m), 1.6 m (elev 282.16 m), 1.5 m (281.84 m), 1.5 m (elev 280.06 m), 2.7 m (elev 277.54 m) and 2.3 m (elev 280.02 m) respectively. The soils in borehole numbers 1 to 6 caved at depths of 3.2 m, 5.2 m, 4.1 m, 3.1 m, 3.1 m and 5.2 m respectively. The water levels taken in these piezometers represent the confined aquifer water levels and are summarized in the table below:

Date Taken	Borehole Number (Depth/Elevation: (m))							
	1	2	3	4	5	6	7	8
Upon Completion date: Dec 23, 2013	5.50/ 261.56	5.5/ 261.42	N/A	N/A	4.0/ 257.93	4.60/ 258.81	4.60/ 259.4	N/A
Upon Completion date: Jan 2, 2014	N/A	N/A	0.60/ 265.42	4.60/ 260.77	N/A	N/A	N/A	0.30/ 263.32
January 2, 2014	1.13/ 265.9	1.90/ 265.02	N/A	N/A	1.3/ 260.63	0.60/ 262.81	1.7/ 262.3	N/A
January 9, 2014	1.14/ 265.91	1.90/ 265.02	1.40/ 266.22	2.40/ 262.97	1.60/ 260.93	0.70/ 262.91	1.90/ 262.5	0.00/ 263.62

De-watering is recommended prior to excavation to maintain stable soil footings and to assist in maintaining stable soil slopes for open cut excavation.



BOREHOLES				
Point #	BH Description	Elevation	Northing	Easting
2442	BH-1	267.06	4845533.08	593736.62
2443	BH-2	268.92	4845460.65	593678.53
2444	BH-3	266.02	4845372.64	593607.88
2445	BH-4	265.37	4845188.29	593459.58
2547	BH-5	261.93	4845054.64	593648.02
2548	BH-6	263.41	4845229.33	593787.65
2545	BH-7	264.00	4845352.54	593886.12
123	BH-8	263.62	4845205.09	593515.09

CLIENT:
NORTH STAR INVESTMENTS
 12654,12728 & 12738 KENNEDY RD,
 CALEDON, ONTARIO

BOREHOLE SITE LOCATION PLAN
 12654,12728 & 12738 KENNEDY RD,
 CALEDON, ONTARIO

NOTE:
 -DRAWINGS WAS OBTAINED FROM CALDER TOPOGRAPHIC SURVEY DONE IN DECEMBER 2013

SCALE: 1:2000	DATE: JAN/14
DRAWING NO: 2	PROJECT NO: 130177

CANADA ENGINEERING SERVICES INC.
 39 DAVISBROOK BOULEVARD
 SCARBOROUGH, ONTARIO M1T 2H6
 Ph: 416 492 4000 Fax: 416 492 4001
 E-mail address: cesi@cesi.ca