File #: 1564

Date: February 27, 2019

Mr. David Hurst, C.E.T. Town of Caledon 6311 Old Church Road Caledon, Ontario, L7C 1J6

Dear Mr. Hurst:

Re:

Stormwater Management Compliance Letter Digram Developments Caledon – Block 132

Town of Caledon, Regional Municipality of Peel

We are pleased to provide you with the following stormwater management (SWM) compliance letter in support of the proposed Digram Developments Caledon – Block 132 residential site plan development in the Town of Caledon (refer to **Figure 1**).

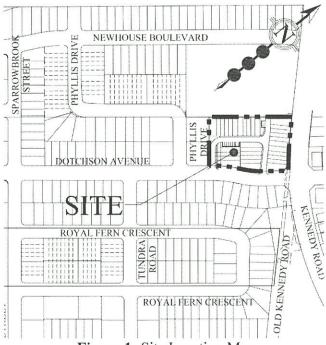


Figure 1: Site Location Map

SCS Consulting Group Ltd. has been retained by Digram Developments Caledon Inc. to prepare this SWM compliance letter in support of the proposed development within the West 'A' Residential Neighbourhood of the Mayfield West Community Secondary Plan, located in the Town of Caledon. The study area is approximately 0.79 ha in size and is bound by:

Feb 16, 2022

Stormwater Management Compliance Letter Digram Developments Caledon – Block 132 Town of Caledon, Regional Municipality of Peel File #: 1564 February 27, 2019 Page 2 of 3

- Kennedy Trails Development Ltd. (21T-12003C) to the north,
- Kennedy Road to the east,
- Phyllis Drive to the west,
- Dotchson Avenue to the south.

The proposed storm drainage within the subject development, per the previously approved design of the existing SWM Pond E2, was to be conveyed via the existing minor and major system infrastructure at an imperviousness of 79%. The existing SWM Pond E2 is located in the existing Argo Caledon Subdivision Phase 1 and the Moscorp VII Developments Inc. subdivisions, respectively. Refer to **Drawing STM-1** for the proposed storm drainage plan.

As shown in **Drawing STM-1**, the proposed site development area and density remains the same as what was contemplated in the design of the existing downstream SWM Facilities. Therefore, it can be concluded that the proposed development satisfies the design criteria for the existing SWM pond and can therefore rely on SWM Pond E2 for quality, quantity and erosion control.

Due to the nature of the servicing design for the subject development, the assumed conveyance method of the 10 year flow in the storm sewer (minor system) with the remaining flows conveyed via the municipal ROW (major system) cannot be achieved. This is due to a number of inlets that cannot be equipped with inlet control devices (ICDs). While ICDs have been included where feasible and/or appropriate within the laneway, the design results in four rear lot catchbasins without ICDs, one laneway catchbasin without an ICD and three catchbasin manholes without ICDs. To ensure that the downstream minor and major system conveyance infrastructure is adequately sized, an update to the existing modelling was completed by JF Sabourin on behalf of the subject development (Attachment A). The memo concludes that there are no negative impacts downstream.

Since the development of the Comprehensive Environmental Impact Statement and Management Plan for the Mayfield West Community Development Plan Area in 2007; new requirements for water balance have been implemented requiring the first 5 mm of rainfall to be detained on-site.

To select appropriate controls for the subject development, the following site characteristics were taken into consideration:

- Subsurface soils and groundwater information within the proposed development area were evaluated by Canadian Engineering Services Inc. Excerpts can be found in **Attachment B**. Underlying fills or reworked tills, the native subsoil mainly consist of silty sand and glacial tills ranging in gradation from clayey silt till to silty sand till;
- Water table depth is typically 0.7 m to 1.9 m below the existing grade, acknowledging this is subject to seasonal fluctuation.

Note that based on the shallow groundwater table (0.7 m to 1.9 m) and the requirements set out within the TRCA/CVC LID guidelines, infiltration measures will not be effective as they require at least 1.0 m of separation from the groundwater table which is not present. As such, soakaway pits, pervious pipe and catchbasin infiltration systems are not expected to be effective.

The following stormwater management measures are proposed to provide best efforts:

Re:

Stormwater Management Compliance Letter Digram Developments Caledon – Block 132 Town of Caledon, Regional Municipality of Peel File #: 1564 February 27, 2019 Page 3 of 3

Reduced Lot Grading

Reducing lot grading from a maximum of 5% to a minimum of 2% is suggested wherever possible to maximize lot level initial abstraction and infiltration. As shown on the engineering drawing set, provided under separate cover, lot grading on site ranges from 2%-5%.

Increased Topsoil Depth

An increase in the proposed topsoil depth on lots will also be used to promote lot level infiltration (up to 0.3 m depth).

Roof Leaders to Grassed Areas

Roof leaders and sump pumps (if required) will be discharged to grassed areas to promote lot level infiltration where feasible.

Rear Lot Catchbasin Infiltration System

Due to high groundwater elevations and the proximity of potential RLCB infiltration systems to the proposed buildings, it is not feasible to propose this solution on the subject development.

In conclusion, the proposed Caledon Block 132 private residential development is in compliance with the existing downstream SWM Pond E2, the storm conveyance infrastruture as well as with the overall Comprehensive Environmental Impact Statement and Management Plan for the Mayfield West Community Development Plan Area.

Please contact the undersigned if you have any questions or require any additional information.

Sincerely,

SCS Consulting Group Ltd.



John Priamo, P. Eng. jpriamo@scsconsultinggroup.com

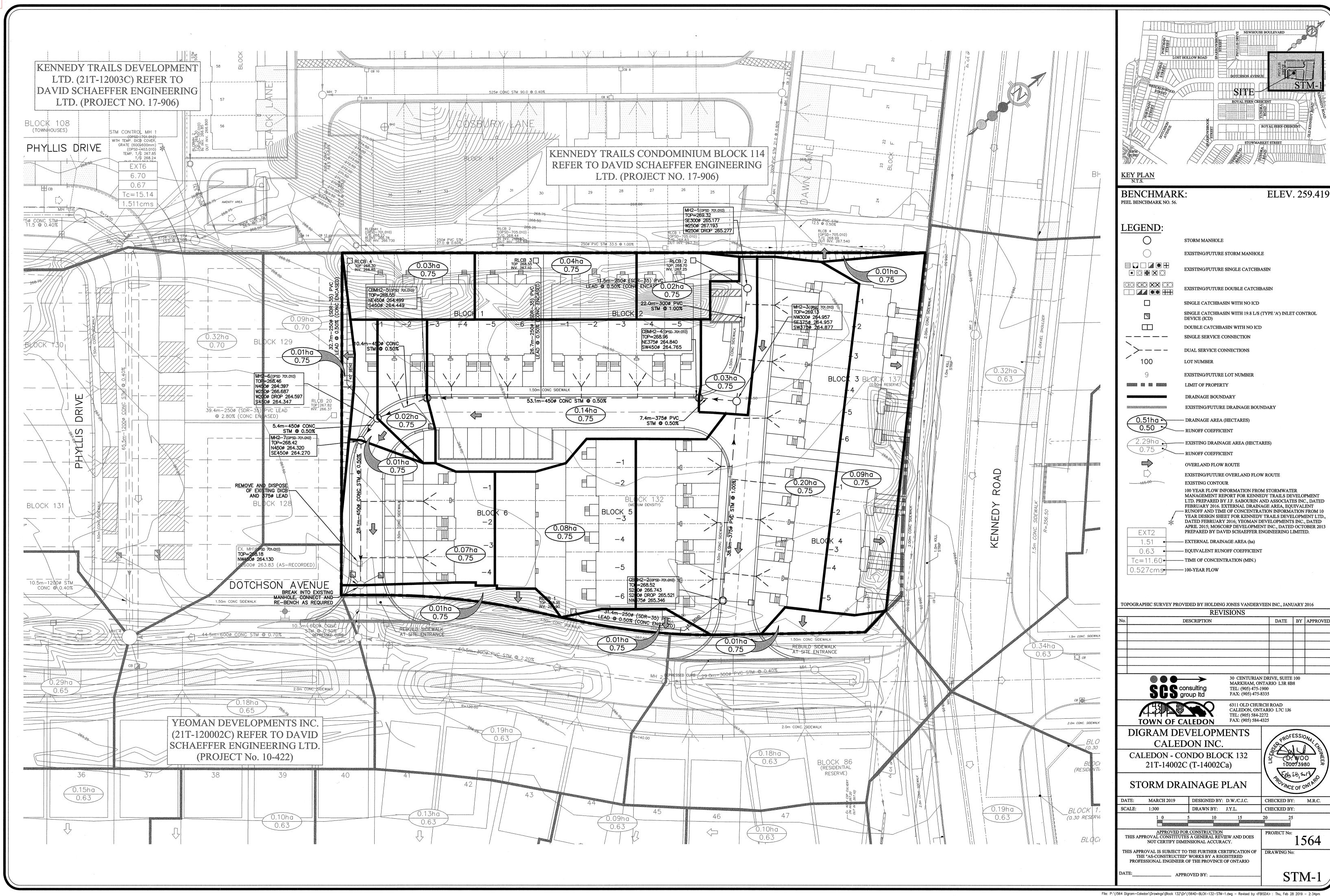
Attachments: **Drawing STM-1** – Storm Drainage Plan

Attachment A – JF Sabourin SWM Memo Attachment B – Relevant Report Excerpts

c. Mr. A. Memon, Digram Developments Caledon Inc. (Letter only via email)

P:\1564 Digram-Caledon\Correspondence\Letters\Caledon-jmp-Block 132 Detailed Design SWM Compliance Letter-26feb19.doc





ATTACHMENT A JF SABOURIN SWM MEMO





J.F. Sabourin and Associates Inc.

WATER RESOURCES AND ENVIRONMENTAL CONSULTANTS

52 Springbrook Drive

Ottawa (Stittsville), ON K2S 1B9

TEL: (613) 836-3884 FAX: (613) 836-0332 WEB: www.jfsa.com

July 4, 2018

SCS Consulting Group Limited

30 Centurian Drive, Suite 100 Markham, Ontario L3R 8B8

Attention: Mr. Douglas Woo, P.Eng.

Subject: Digram Developments Caledon Inc. /

Downstream Impacts of Condo Block 132 SWM Design our file: 552-05

As requested by your office and based on the available information as described below, we have evaluated the impact of the latest design for Condo Block 132 by SCS Consulting Group Limited on the performance of downstream storm sewer system (existing and proposed) and existing Stormwater Management (SWM) Facility E2.

The proposed condo block is located within the Diagram Developments Caledon Inc. Subdivision, and north of the Yeoman Developments Inc. subdivision, within the Town of Caledon. The downstream storm sewer system and pond were designed to service a 0.79 ha area at 79% imperviousness from the condo block site, as per the August 2011 *Design Brief for Stormwater Management Pond E2 for the Mayfield West Community 3rd Registration Area* by David Schaeffer Engineering Limited and J.F. Sabourin and Associates Inc., and the more recent update to the overall model in the December 2017 *Stormwater Management Report for Kennedy Trails Development Condo Block* by J.F. Sabourin and Associates Inc. Per the December 2017 *SWM Report*, minor system capture from the condo block to Yeoman MH 3 was limited to the 10-year Rational Method flow of 221 L/s, assuming no surface storage on-site and with excess major system flows draining south to Dotchson Avenue just east of the intersection with Phyllis Drive.

The detailed design of the condo block by SCS Consulting Group Limited proposes that the 0.79 ha block be developed at an average imperviousness of 64%. Refer to Attachment C for the grading plan, servicing plan and storm drainage plan as provided by SCS Consulting Group Limited in July 2018. As the proposed imperviousness is lower than that estimated in the preliminary design (79%), it may be reasonably expected that the detailed design of the condo block will not have a negative impact on the performance of SWM Facility E2.

The SWM design by SCS Consulting Group Limited proposes an internal storm sewer system servicing 0.65 ha of the site. Excess major system flows from the 0.65 ha area will drain overland to Dotchson Avenue at two access roads to the condo block (east and west). Major system flows from the west access road drain to Dotchson Avenue just east of the intersection with Phyllis Drive, consistent with the preliminary design. However, the east access road is located at a high point on Dotchson Avenue, such that major system flows on the west side of the access road will drain west along Dotchson Avenue towards its intersection with Phyllis Avenue, and major system flows from the east side of the access road will drain east along Dotchson Avenue towards its intersection with Kennedy Road. This is not consistent with the preliminary design and may impact the performance of the major and minor systems on this segment of Dotchson Avenue and downstream Kennedy Road. Furthermore, the remaining 0.14 ha of the condo block cannot be serviced by the internal storm sewer due to grading restrictions, and sheet flows overland to Dotchson Avenue and Kennedy Road, with a small portion (0.01 ha) draining into external rearyards along the boundary of the site.

The most recent DDSWMM / XPSWMM models of the overall drainage area to SWM Facility E2, as submitted with the December 2017 SWM Report, were updated to reflect the detailed design of Condo Block 132 as provided by SCS Consulting Group Limited. The proposed minor and major system drainage routes for Condo Block 132

Client: SCS Consulting Group Limited

are shown in plan view in Figure 1. Refer to the December 2017 SWM Report for details of the external areas included in the model. The internal storm sewer within the condo block has four (4) rearyard catchbasins without inlet control devices (ICDs), three single catchbasins with Type A ICDs, one single catchbasin without an ICD, and three catchbasin manholes without ICDs.

Within the downstream development, ICDs were used to limit minor system capture to the 10-year flow. As a result of the 0.13 ha of the condo block draining overland to Kennedy Road and Dotchson Avenue, the ICDs in the two existing catchbasins on Dotchson Avenue at its intersection with Kennedy Road (97.5 L/s combined capacity) are now undersized to fully capture the 10-year flows (112 L/s based on the updated modelling). However, the existing storm sewer is not sufficiently sized to allow for larger ICDs to be installed at this location without a negative impact on the performance of the storm sewer system. Major system flows from the condo block to this location was minimized where possible, most notably by the inclusion of a 12 cm deep static ponding area and catchbasins without ICDs on the east Condo Block access road, to ensure the capture the full 10-year flows on the east access road. Redirecting roof drainage into the internal Condo Block system from the portion of the site fronting Kennedy Road was considered, but we understand is not possible due to the peaking of the roofs. It should be noted that the full 5-year flows (87 L/s) are captured at the two existing catchbasins on Dotchson Avenue, and as such standing water at this location will not be frequent. Furthermore, all ICDs and existing catchbasins on Kennedy Road downstream of this intersection are sufficiently sized to capture the 10-year flows, so the 14.5 L/s of flow spilling during the 10-year storm is a localized issue only.

Within the downstream development, the depth of water at the gutter will be retained within the right-of-way and will not exceed the maximum allowable value of 30 cm during the 100-year Chicago Storm (refer to Calculation Sheet B-1 of Attachment B, where the calculated maximum was 24.8 cm). Furthermore, it was determined that, for the 100-year event and for all major system segments, the product of the depth of water (m) at the gutter multiplied by the velocity of flow (m/s) will not exceed the maximum allowable 0.65 m²/s (refer to Calculation Sheet B-1 of Attachment B, where the calculated maximum was 0.371 m²/s). Furthermore, the two 15.5 m wide overland flow routes provided north of Pond E2 still have sufficient capacity to safely convey the 100-year major system flows from Dougall Avenue to the pond (refer to Calculation Sheet B-2 of Attachment B).

Tables A-1 and A-2 of Attachment A summarizes the pipe data and hydraulic simulation results based on the updated modelling for the 100-year 4-hour Chicago design storm under free outfall conditions for the Pond E2 system under ultimate and interim conditions, respectively (for the school block to the north; refer to the December 2017 *SWM Report* for further details). Note that the flowing full pipe velocities are not less than 0.75 m/s and no greater than 6.0 m/s for all proposed pipes. The 100-year flow will surcharge most parts of the minor system; however, a freeboard of 0.3 m between the hydraulic grade line and the underside of footings has been provided throughout developments tributary to Pond E2. Note that lots marked "FUT" indicate a location where the freeboard is an estimate only, and the actual freeboard within the Digram Developments Caledon Inc. subdivision and Condo Block 132 is to be verified by SCS Consulting Group Limited.

Yours truly,

J.F. Sabourin and Associates Inc.

Laura Pipkins, P.Eng.

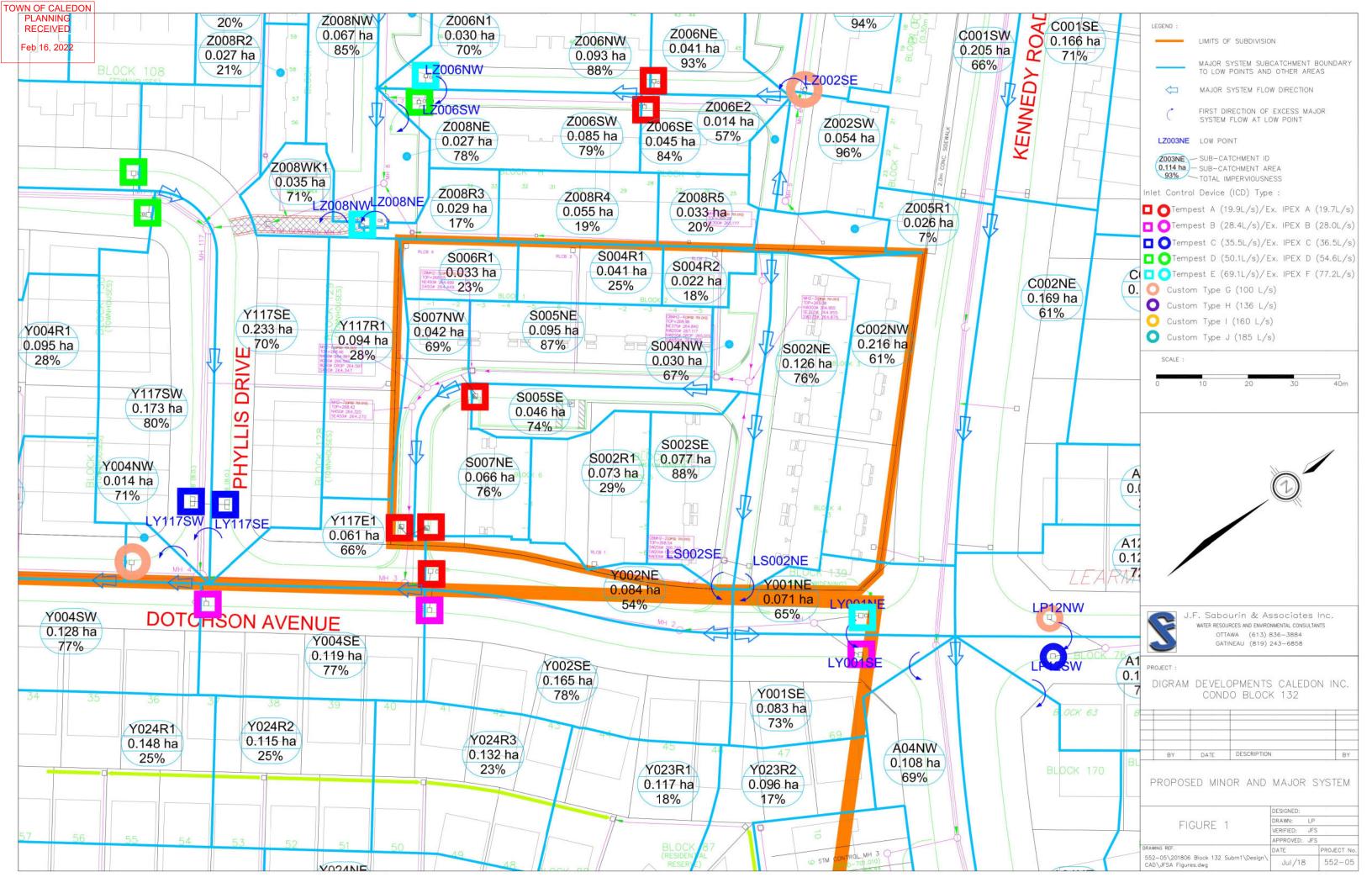
cc: J.F. Sabourin, M.Eng, P.Eng. Director of Water Resources Projects

Figure 1: Proposed Minor and Major System

Attachment A: Pipe Data and Hydraulic Gradeline Results; XPSWMM Model Schematic

Attachment B: Calculation Sheets

Attachment C: Drawings GR-1, S-1 and STM-1 (July 2018, SCS Consulting Group Limited)



TOWN OF CALEDON **PLANNING**

ATTACHMENT

Pipe Data and Hydraulic Gradeline Results

XPSWMM Model Schematic





Water Resources and **Environmental Consultants**



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									Elev.	Elev.			Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																			22	266.25	0.762	99.9	89.6	265.25
																			23	266.24	0.752	99.9	89.6	265.25
																			24	266.10	0.735	99.9	64.9	265.13
																			25	266.21	0.845	99.9	64.9	265.13
																			26	266.21	0.955	99.9	42.6	265.02
29	30	262.824	262.429	1500	N/A	79.0	0.5	0.013	267.163	267.122	2.829	4.998	5.859	1.2	0.489	1.417	264.813	263.865	27 120	266.00 265.55	0.745	99.9	42.6	265.0
30	35		261.885	1500	N/A	53.0	0.5	5656306605106	267.122		3.688	VICEDERA SECOND	7.355	1.1	0.489	1.433	263.865	263.277	N/A	N/A	0.507 N/A			
31	32		262.909	600	N/A	37.5	0.9	0.013			1.373		0.369	1.0	0.451	1.333	264.110	263.984	Part12	265.30	0.960			
32	30	262.859	262.785	600	N/A	18.5	0.4	0.013		267.122	1.373	0.388	0.368	0.9	0.525	1.333	263.984	263.865	N/A	N/A	0.900 N/A			
35	100		261.679	1500	2400	29.5	0.5	0.013	and the second second	266.641	3.345	12.040	7.520	0.6	-0.058	1.433	263.277	263.237	B122S	264.97	1.463			
36	68	The second second	and the second	1500	2400	120.0	0.5	0.013	266.363		3.345	12.040	8.113	0.7	-0.330	1.433	262.568	262.030	B95S	264.27	1.472			
38	40		267.896	375	N/A	70.0	0.8	0.013		271.439	1.420	0.157	0.065	0.4	-0.025	1.350	268.806	268.742	57	270.08	1.044			
39	40	267.623		900	N/A	43.0	0.4	0.013	271.464		1.800	1.145	1.056	0.9	0.513	1.367	269.036	268.742	N/A	N/A	N/A			
40	44		267.061	900	N/A	77.5	0.4	0.013	271.439		1.800	1.145	1.135	1.0	0.471	1.367	268.742	268.272	62	269.77	0.798			
41	44	The second secon	267.061	900	N/A	43.0	0.4	0.013	270.975		1.800	1.145	1.340	1.2	0.369	1.400	268.502	268.272	N/A	N/A	N/A			
42	43	268.296	267.911	300	N/A	38.5	1.0	0.013	271.446		1.368	0.097	-0.004	0.0	-0.272	1.433	268.324	268.322	13	269.79	1.236			
43	44		267.511	450	N/A	50.0	0.5	0.013	271.302		1.268	0.202	0.086	0.4	0.111	1.350	268.322	268.272	67	269.64	1.088			
44	45		266.736	1050	N/A	25.0	0.7	0.013	271.006		2.639	2.285	2.516	1.1	0.311	1.400	268.272	268.011	N/A	N/A	N/A			
45	46		266.462	1050	N/A	32.0	0.7	0.013		270.449	2.639	2.285	2.506	1.1	0.275	1.400	268.011	267.743	B145	269.15	0.909			
46	47		265.452	1050	N/A	120.0	0.8	0.013		269.754	2.821	2.442	2.662	1.1	0.281	1.400	267.743	266.647	77	268.30	0.327			
47	37	265.402	265.050	1050	N/A	44.0	0.8	0.013			2.821	2.442	2.783	1.1	0.195	1.500	266.647	266.221	81	267.95	1.073			
48	49	264.712	264.007	1200	N/A	70.5	1.0	0.013	269.299	268.030	3.447	3.899	3.367	0.9	0.061	1.483	265.973	265.417	87	266.71	0.507			
49	50	264.007	263.887	1200	N/A	12.0	1.0	0.013	268.030	268.145	3.447	3.899	3.384	0.9	0.210	1.483	265.417	265.154	N/A	N/A	N/A			
50	51	263.837	262.437	1200	N/A	87.5	1.6	0.013	268.145	266.532	4.360	4.932	3.513	0.7	0.117	1.483	265.154	264.486	B94N	265.45	0.066			
																			B94N	265.45	0.611	87.4	16.1	264.6
																			B93S	265.60	0.642	87.4	31.7	264.7
																			B93N	266.29	1.148	87.4	55.7	264.9
																			B125S	266.32	1.061	87.4	71.1	265.0
																			B129	266.55	1.471	87.4	47.5	264.8
																			1	266.22	1.262	87.4	31.7	264.7
					_														2	265.61	0.776	87.4	15.4	264.6
51	65	262.287	262.011	1350	N/A	39.5	0.7	0.013	266.532	266.418	3.120	4.466	3.830	0.9	0.849	1.483	264.486	264.035	B94S	265.25	0.534			
52	53		267.124	825	N/A	65.5	0.6			271.276	2.080	1.112	1.118	1.0	0.931	1.350	269.273	268.655	N/A	N/A	N/A			
53	54		266.765	825	N/A	46.5	0.6	and the second	271.276		2.080	1.112	1.121	1.0	0.786	1.450	268.655	268.374	42	269.59	0.705			
54	56	The second secon	266.375	900	N/A	45.0	0.7		270.994		2.381	1.515	1.192	0.8	0.784	1.450	268.374	267.973	40	269.40	0.796			
55	56		266.452	825	N/A	78.5	0.7		270.864		2.247	1.201	1.291	1.1	0.665	1.350	268.510		51	269.55	0.810			
56	57	266.295	264.859	975	N/A	110.5	1.3	0.013	270.726	269.282	3.422	2.555	2.563	1.0	0.703	1.500	267.973	266.558	34	268.07	-0.133	440.5		000 5
																			34	268.07	1.264	110.5	1.4	266.5
																			35	268.30	1.494	110.5	1.4	266.5
																			36 37	268.30	1.305	110.5	16.2	266.7
																			37	268.53	1.252	110.5	38.3	267.0
																			38	268.84	1.368	110.5	53.4	267.2
57	97	264 704	264.062	1050	N/A	E1 E	1.4	0.042	260 202	260 605	2 724	2 224	2 770	0.0	0.724	1 522	266 550	266.046	39	268.84	1.368	110.5	53.4	267.2
58	97 59		264.063	525	N/A N/A	51.5	1.4	0.013		268.695		3.231	2.770	0.9	0.724	1.533	266.558	266.046	31	267.57	0.782			
59	60	267.423 266.413		525	N/A N/A	120.0	0.8	0.013	270.764		1.777	0.385	0.183	0.5	-0.268	1.367	267.680	266.985	20 B131	268.32	0.410 0.085			
JJ	00	200.413	205.138	1 325	IN/A	85.0	1.5	0.013	269.720	208.586	2.433	0.527	0.437	0.8	0.047	1.350	266.985	266.200	B131	267.30		94.0	10	266
																			B131	267.30	0.853	84.8	1.8	266.
																			B132	267.42	0.973	84.8	1.8	266.
																			B133	267.70	1.052	84.8	23.6	266.4
																			B134	267.70	1.052	84.8	23.6	266.4
																			B135	267.89	1.031	84.8	46.3	266.6 266.6
																			B136	268.23	1.371	84.8	46.3	200.

U/S	D/S	U/S	D/S	raulic Si Pipe Dia.		Pipe	Pipe	n		D/S MH		Design	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	In	terpolated H	GL
МН	МН	Invert	Invert	/ Height	and the same	Long IV and	100		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number				Dist. From	
100,780,000,00		Southern							Elev.	Elev.		2000000	Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m^3/s)	100-000000	(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																			B137	268.29	1.225	84.8	68.6	266.835
																			B138	268.29	1.225	84.8	68.6	266.835
																			21	268.32	1.168	84.8	78.0	266.922
																			22	268.05	0.898	84.8	78.0	266.922
																			23	267.87	0.973	84.8	50.4	266.667
s		7																	24	267.81	0.913	84.8	50.4	266.667
60	62	A STATE OF THE STA	264.538	675	N/A	15.0	3.0	0.013	34-60-60-60-60-60-60-60-60-60-60-60-60-60-	268.734	4.069	1.456	0.708	0.5	0.537	1.333		265.870	N/A	N/A	N/A			
61	62		265.000	300	N/A	43.5	2.1	0.013	268.976		1.982	0.140	0.126	0.9	0.372	1.350		265.870	B91	267.28	0.464			
62	63		264.038	675	N/A	30.0	1.4	0.013		268.239	2.779	0.995	0.819	0.8	0.737	1.417		265.579	7	267.01	0.910			
63	64		263.306	825	N/A	48.5	1.2	0.013		267.701	2.942	1.572	1.032	0.7	0.866	1.433		265.119	5	266.38	0.571			
64	65	and the second second	262.161	1200	N/A	85.5	0.9	0.013	Section Contraction	266.418	3.270	3.699	4.312	1.2	0.988	1.400	265.119	264.035	N/A	N/A	N/A			
65	67 67		261.497	1500 525	N/A	40.5	0.9	0.013		265.810	3.795	6.706	8.137	1.2	0.674	1.433	COLAR MC III SOCIODOLINI	263.479	N/A	N/A	N/A			
66 67	78		262.422	1500	N/A N/A	46.5	0.4	0.013		265.810	1.256	0.272	0.266	1.0	0.601	1.367	263.734	263.479	N/A N/A	N/A	N/A			
68	69	Total Control of the	260.891 259.281	1500	3000	50.5 133.0	1.1	0.013	and the second	265.678 264.722	4.195 4.846	7.414 21.806	9.630 17.991	1.3 0.8	0.532 -0.081	1.433	263.479 262.030	262.523 261.143	N/A	N/A N/A	N/A N/A			
69	70	100000000000000000000000000000000000000	258.706	1500	3000	57.5	1.0	0.013	The Court of the C	264.089	4.846	21.806	19.109	0.9	0.361	1.433		260.623	N/A	N/A	N/A N/A			
70	70A		258.610	1500	3000	10.2	0.5	0.013		263.870	3.251	14.628	19.117	1.3	0.466	1.433		260.527	N/A	N/A	N/A			
73	74		259.664	525	N/A	99.0	0.8	0.013		264.262	1.777	0.385	0.147	0.4	-0.272	1.483		260.423	N/A	N/A	N/A			
74	75	259.589		600	N/A	43.5	0.4			263.400	1.373	0.388	0.243	0.6	0.234	1.483	F-1000 SENSON	260.333	N/A	N/A	N/A			
75	80	258.512		1500	3000	57.5	0.5	0.013		261.870	3.251	14.628	20.067	1.4	0.320	1.433		260.273	N/A	N/A	N/A			
76	48		265.162	750	N/A	46.0	0.4	0.013	269.631	269.299	1.594	0.704	0.306	0.4	-0.069	1.517		265.973	N/A	N/A	N/A			
77	78	263.019		375	N/A	57.5	0.5	0.013		265.678	1.123	0.124	0.060	0.5	-0.168	1.300		262.909	N/A	N/A	N/A			
78	68		260.641	1500	N/A	20.0	1.1	0.013		265.468	4.195	7.414	9.669	1.3	0.162	1.433	101000000000000000000000000000000000000	262.141	N/A	N/A	N/A			
80	PondE2		258.117	1500	3000	21.5	0.4	0.013		260.700	3.065	13.792	20.083	1.5	0.570	1.417		260.265	N/A	N/A	N/A			
97	64	264.033	263.081	1050	N/A	68.0	1.4	0.013	268.695	267.701	3.731	3.231	3.116	1.0	0.963	1.533	266.046	265.119	29	267.35	1.074			
310	31	263.237	263.109	600	N/A	32.0	0.4	0.013	266.131	266.448	1.373	0.388	0.371	1.0	0.383	1.333	264.220	264.110	Part1	264.92	0.470			
500	50	265.097	264.737	300	N/A	36.0	1.0	0.013	268.339	268.145	1.368	0.097	0.005	0.1	-0.230	1.450	265.167	265.154	B125N	266.60	1.203			
660	66	263.109	262.683	450	N/A	71.0	0.6	0.013	266.585	265.876	1.389	0.221	0.093	0.4	0.229	1.283	263.788	263.734	N/A	N/A	N/A			
PondE2	O210	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	4.782	N/A	N/A	1.933	N/A	N/A	N/A	N/A	N/A			
PondE2	EMER	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	1.648	N/A	N/A	1.933	N/A	N/A	N/A	N/A	N/A			
O210	0211	256.300	255.994	1800	N/A	25.5	1.2	0.013	260.000	260.000	4.948	12.592	4.782	0.4	-0.893	1.933	257.207	256.779	N/A	N/A	N/A			
37	48	265.050	264.862	1050	N/A	23.5	8.0	0.013	269.459	269.299	2.821	2.442	2.803	1.1	0.121	1.500	266.221	265.973	89	268.03	1.579			
72	80	o x succession out out out of	259.216	750	N/A	28.0	0.5	0.013		261.870	1.782	0.787	-0.031	0.0	0.178	1.383		260.273	N/A	N/A	N/A			
0211	OUT		255.892	1800	N/A	8.5	1.2	000000000000000000000000000000000000000		258.292	4.948	12.592	4.782	0.4	-1.015	1.933		256.661	N/A	N/A	N/A			
C120	C166	264.490	CONTRACTOR OF THE PARTY OF THE	1050	N/A	19.3	0.4	0.013		268.110	1.995	1.727	1.496	0.9	0.512	1.483		265.989	Cole Eng.	266.72	0.438			
C140	C40	265.810	Second services	675	N/A	118.3	0.4	C. C	Total Control	268.640	1.486	0.532	0.522	1.0	0.351	1.367	Action to the control of the	266.310	Cole Eng.	267.94	0.874			
C150	C59	266.750		450	N/A	94.9	1.0		270.310	43.500 MODEL (CO.)	1.793	0.285	0.191	0.7	-0.172	1.350	Salara Calara Calara	266.519	Cole Eng.	268.47	1.212			
C165 C166	C40 11	265.500		900 1050	N/A	47.3	0.4			268.640	1.800	1.145	0.830	0.7	0.004	1.367	266.404	266.310	Cole Eng.	N/A	N/A			
C39	C140	264.370	00000000000000000000000000000000000000	450	N/A	28.9	0.4	400000000000000000000000000000000000000	CONTRACTOR CONTRACTOR	267.991	1.995	1.727	1.555	0.9	0.569	1.483	265.989	265.883	Cole Eng.	266.55	0.331			
C40	C62	267.940 264.960		1050	N/A N/A	105.2 16.7	1.0 0.4	0.013		269.620 268.640	1.793 1.995	0.285 1.727	0.305	1.1 0.8	-0.041 0.300	1.367	268.349 266.310	267.274 266.267	Cole Eng. Cole Eng.	268.89 N/A	0.311 N/A			
C42	1	266.170		450	N/A	36.8	0.4	0.013	269.170	269.380	1.268	0.202	0.072	0.6	-0.232	1.300		266.283	Cole Eng.	N/A	N/A N/A			
C505	1		265.830	825	N/A	17.2	0.5	0.013	269.380	269.380	1.899	1.015	0.364	0.4	-0.232	1.317		266.283	Cole Eng.	N/A	N/A N/A			
C506	C505	266.220	transfer out the second second	750	N/A	23.1	1.0	0.013	269.380	269.380	2.520	1.113	0.365	0.3	-0.421	1.317	TO DESCRIPTION OF THE PERSONS OF THE	266.314	Cole Eng.	N/A	N/A			
C59	C60	265.730		450	N/A	9.7	1.0			269.170	1.793	0.285	0.174	0.6	0.339	1.333		266.464	Cole Eng.	267.63	0.881			
C60	9	265.550		450	N/A	37.7	0.8	0.013		268.665	1.552	0.247	0.170	0.7	0.464	1.500		266.344	Cole Eng.	267.58	0.886			
C62	C63		264.610	1050	N/A	60.5	0.4	0.013		268.300	1.995	1.727	1.422	0.8	0.367	1.433		266.094	Cole Eng.	267.21	0.713			
C63	C120	264.580	hospitation communications	1050	N/A	13.7	0.4	0.013	200000000000000000000000000000000000000	268.210	1.995	1.727	1.463	0.8	0.464	1.483	SCHOOL SECTION OF THE PARTY OF	266.052	Cole Eng.	266.91	0.586			
C75	7	267.270		300	N/A	32.8	1.0			270.096	1.368	0.097	0.000	0.0	-0.458	0.000		267.112	Cole Eng.	268.88	1.538			
C75	C150	267.070	porposition and control	300	N/A	12.7	1.0			270.310	1.368	0.097	0.000	0.0	-0.342	0.000		267.028	Cole Eng.	269.88	2.622			
C40W	C42	266.490		450	N/A	57.8	0.5		268.640		1.268	0.202	0.073	0.4	-0.263	1.283		266.388	Cole Eng.	267.52	0.613			
F1	F3	268.818		450	N/A	76.5	0.4	000000000000000000000000000000000000000	272.312		1.134	0.180	0.172	1.0	0.124	1.367	269.392	269.153	14	270.59	0.968			
F2	F3	269.772	269.150	300	N/A	41.5	1.5	0.013	272.721	271.881	1.675	0.118	0.045	0.4	-0.171	1.367	269.901	269.279	N/A	N/A	N/A			

				raulic Si											Constrains	T:	Man	Mari	1	DE.	Farabasad	l-s		01
U/S MH	D/S MH	U/S	D/S	Pipe Dia.		Pipe	Pipe	n	U/S MH	122	Design	Design	Peak	Peak /	Surcharge	Time	Max. U/S	Max.	Lot Number	BF	Freeboard		erpolated H	HGL
IVIT	IVIH	Invert	Invert	/ Height	vviatn	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design Flow	U/S (1)	to	HGL	D/S HGL	Number			HGL	Dist. From D/S MH	HGL
		(m)	(m)	(mm)	(mm)	(m)	(0/_)		Elev. (m)	Elev.	(m/s)	(m ³ /s)	Flow (m ³ /s)	FIOW		Peak (b)		00000000000		(m)	(m)		1/10/2/11/03/11/2/03/11	(m)
F3	F4	(m) 268.212	(m) 267.640	(mm) 750	(mm) N/A	(m) 114.5	(%) 0.5	0.013		(m) 271.359	1.782	0.787	0.670	0.9	(m) 0.191	(h) 1.367	(m) 269.153	(m) 268.832	9	(m) 270.05	0.667	(m)	(m)	(m)
F4	55		267.070	825	N/A	99.0	0.5			270.864	1.899	1.015	0.931	0.9	0.442	1.350	268.832	268.510	3	269.67	0.608			
F100	F3		268.362	600	N/A	19.5	0.4	0.013	271.710		1.373	0.388	0.349	0.9	0.235	1.367	269.275	269.153	N/A	N/A	N/A			
101	102	NO. 10. 10. 10. 10. 10. 10. 10. 10. 10. 10	262.663	525	N/A	52.5	0.6	0.013	266.056	265.856	1.539	0.333	0.248	0.5	-0.185	1.367	263.318	263.000	53	264.55	1.002			
102	103		261.790	750	N/A	72.0	0.9	0.013		265.495	2.391	1.056	0.406	0.4	-0.409	1.367	262.779	262.382	47	264.37	1.361			
103	106	THE RESERVE AND ADDRESS OF THE PERSON NAMED IN	260.842	750	N/A	76.5	1.2	0.013		265.315	2.760	1.220	0.855	0.7	-0.128	1.367	262.382	261.789	41	263.91	1.298			
Y34	106		260.102	1500	N/A	10.8	0.5	0.013		265.315	2.683	4.742	5.346	1.1	0.211	1.417	261.861	261.789	FUT	263.89	1.795			
105	106	and the second control of the second	261.142	450	N/A	120.0	1.0	5656306605106	ALCOHOLOGICA AND ACCOUNT	265.315	Section Control Control	0.285	0.379	1.3	1.376	1.367	264.168	261.789	29	263.74	-0.658			
		202.042	201.142	100	1,44.	120.0	1.0	0.010	200.000	200.010	1.700	0.200	0.075	1.0	1.070	1.007	204.100	201.700	29	263.74	1.184	120.0	27.1	262.32
																			28	263.96	1.146	120.0	40.1	262.58
																			27	264.06	1.246	120.0	40.1	262.58
																			26	264.22	0.970	120.0	62.1	263.02
																			25	264.22	0.970	120.0	62.1	263.02
106	107	260.092	259.808	1500	N/A	35.5	0.8	0.013	265.315	265 195	3.578	6.323	6.528	1.0	0.197	1.433	261.789	261.457	N/A	N/A	N/A	120.0	02.1	200.02
107	108	The second secon	259.486	1500	N/A	34.0	0.8	0.013	265.195	and the second second	3.578	6.323	6.854	1.1	0.197	1.433	261.457	261.104	70	263.62	1.933			
108	115	Charles and Charles	258.824	1500	N/A	76.5	0.8	0.013	The Court of the C	264.761	3.578	6.323	7.009	1.1	0.168	1.433	261.104	260.382	B108S	263.02	1.876			
100	36	261.679		1500	N/A	53.0	0.5	0.013		266.363	2.912	5.146	7.810	1.5	0.058	1.433	263.237	262.568	B122S	264.99	1.523			
110	112		261.790	525	N/A	42.5	0.8	0.013		265.342	1.777	0.385	0.285	0.7	0.522	1.433	263.177	262.996	67	263.98	0.573			
112	113	A CONTRACTOR CONTRACTOR CONTRACTOR	261.790	600	N/A	54.0	0.5	0.013		265.038	1.536	0.434	0.354	0.7	0.681	1.400	262.996	262.772	B104S	263.55	0.324			
113	114		261.203	600	N/A	27.0	0.6		265.038		1.682	0.476	0.451	0.9	0.807	1.400		262.609	84	263.21	0.208			
110		201.505	201.203	000	14// 1	27.0	0.0	0.013	200.000	204.003	1.002	0.470	0.451	0.5	0.007	1.400	202.112	202.003	84	263.21	0.361	27.1	1.6	262.61
																			85	263.29	0.358	27.1	15.4	262.70
																			86	263.44	0.446	27.1	25.7	262.76
114	115	261.153	260.754	600	N/A	57.0	0.7	0.013	264.865	264 761	1.817	0.514	0.761	1.5	0.856	1.367	262.609	261.302	81	263.13	0.291	27.1	20.7	202.70
		201.100	200.704			07.0	0.7	0.010	204.000	204.701	1.017	0.014	0.701	1.0	0.000	1.007	202.000	201.002	81	263.13	1.210	57.2	17.0	261.690
																			82	263.43	1.167	57.2	32.0	262.033
																			83	263.43	0.833	57.2	46.6	262.367
115	116	258.794	258.590	1500	2400	68.0	0.3	0.013	264.761	264.341	2.516	9.059	8.037	0.9	0.088	1.433	260.382	260.299	B106S	263.53	2.918	01.2	10.0	202.00
116	125	A CONTRACTOR OF THE PARTY OF TH	258.527	1500	2400	21.0	0.3	-55000000000000000000000000000000000000	500000000000000000000000000000000000000	264.463	2.516	9.059	8.039	0.9	0.209	1.433	260.299	260.290	N/A	N/A	N/A			
117	118	261.674	260.972	450	N/A	117.0	0.6	0.013		264.950	1.389	0.221	0.095	0.4	-0.232	1.350	261.892	261.521	B109S	263.40	1.278			
118	119	260.922		450	N/A	31.5	0.7		264.950		1.500	0.239	0.263	1.1	0.149	1.350		261.062	N/A	N/A	N/A			
119	125		259.452	525	N/A	97.5	1.2			264.463		(Sec. 1997)	0.356	0.8	-0.181	1.350			16	263.24	2.044			
120	121	261.339	Section Contraction of the Contr	375	N/A	61.0	0.8	0.004.000000000000000000000000000000000	265.068	Market State Control	1.420	0.157	0.064	0.4	-0.203	1.367	261.511	261.140	7	263.08	1.339			
121	123		260.339	450	N/A	48.0	0.9			264.403	1.701	0.270	0.152	0.6	-0.081	1.400		261.022	3	262.83	1.460			
123	124		260.009	450	N/A	25.5	1.1		264.403		1.880	0.299	0.378	1.3	0.283	1.367	TO STATISTICS - SANSANANA	260.422	1	262.64	1.388			
124	125	control of the second second	259.527	450	N/A	17.5	2.3	and a second	and the second	264.463	2.719	0.432	0.377	0.9	-0.070	1.367		260.290	N/A	N/A	N/A			
125	126	Committee of the Commit	258.371	1500	2400	37.5	0.3	0.120	264.463	Control of the second	2.516	9.059	8.766	1.0	0.313	1.433		260.279	80	262.79	2.270			
126	129		258.325	1500	2400	15.5	0.3		264.186		2.516	9.059	8.769	1.0	0.408	1.433		260.276	77	262.83	2.321			
127	128		259.506	450	N/A	88.5	0.9	The second second second		264.067	1.701	0.270	0.380	1.4	1.238	1.367		260.435	72	262.58	0.360			
128	129	providence and another	259.375	450	N/A	13.5	0.6	0.0000000000000000000000000000000000000	00000000000000000	264.080	1.389	0.221	0.380	1.7	0.529	1.367	a contract to the second	260.276	N/A	N/A	N/A			
129	180		258.220	1500	2400	35.0	0.3		264.080		2.516	9.059	9.137	1.0	0.451	1.433		260.267	N/A	N/A	N/A			
180	PondE2	258.135		1500	2400	11.5	0.3			260.700	2.516	9.059	9.119	1.0	0.632	1.433		260.265	N/A	N/A	N/A			
1M	110		262.210	450	N/A	120.0	0.7	a trace trace and trace	266.242		1.500	0.239	0.140	0.6	-0.052	1.467	263.448	263.177	64	264.32	0.642			
1M	2M	263.004	DOMESTIC SECTION OF THE SECTION OF T	525	N/A	88.0	0.4	10700000000000000	266.242	200000000000000000000000000000000000000	1.256	0.272	0.191	0.7	-0.081	1.367	263.448	263.344	Part29	264.67	0.992			
2M	100		262.306	675	N/A	56.0	0.4	0.013		266.641	1.486	0.532	0.330	0.6	0.145	1.367		263.237	Part47	264.78	1.206			
CMH5	310		263.388	450	N/A	12.5	0.5	0.013		266.131	1.268	0.202	0.138	0.7	0.399	1.433		264.220	N/A	N/A	N/A			
DICB1	101		263.129	375	N/A	15.5	1.5			266.056	1.944	0.215	0.076	0.4	-0.179	1.317		263.318	N/A	N/A	N/A			
T1	T2	promotoscovoraneas.	269.547	375	N/A	50.5	0.8	NAME OF TAXABLE PARTY.	272.822	The Control of the Co	1.420	0.157	0.026	0.4	-0.173	1.367	ACRES NO SERVICE	269.651	17	271.20	0.914			
T1	T3		269.553	375	N/A	56.0	0.7		272.822		1.280	0.141	0.020	0.2	-0.236	1.367		269.691	41	271.14	0.854			
T2	F100	SOMEON BUTCHES OF THE	268.515	525	N/A	98.0	0.9		272.450		1.885	0.408	0.152	0.3	-0.298	1.367	24,000-31,000(3000000000)	269.275	25	270.49	0.636			
T3	T4		268.993	525	N/A	63.0	0.9		272.430		1.602	0.408	0.132	0.4	-0.403	1.367		269.273	2	270.49	1.025			
T4	T5	No. or a construction of the construction of t	268.247	675	N/A	149.0	1021102	000000000000000000000000000000000000000	272.160	WATER CONTRACTOR OF THE PARTY O	na seranan	0.532	0.248	0.1	-0.403	100.00000000000000000000000000000000000	THE RESIDENCE OF THE PARTY OF T	268.916	54	270.78	1.025			
T5	41			No. of the last of	N/A		0.4				1.486			200		1.367					0.554			
10	41	200.022	267.290	300	IN/A	146.5	0.5	0.013	211.111	270.975	2.012	1.280	1.078	0.8	-0.006	1.367	268.916	268.502	Part7	269.70	0.554			

U/S	D/S	U/S	D/S	Pipe Dia.		Pipe	Pipe	n	U/S MH	D/S MH		_	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	Int	erpolated H	GI
MH	MH	Invert	Invert	/ Height	and the same	The second second	Local VE	"	Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number	Di	riceboard	0.000	Dist. From	
14111		mvort	mvort	, rioigitt	Widai	Longui	Ciopo		Elev.	Elev.	V 01.	1 1011	Flow	Flow	(1)	Peak	HGL	HGL	rumber			HGL	D/S MH	1102
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)	11011	(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
T6	T7	269.774	269.644	300	N/A	13.0	1.0	0.013	272.613		1.368	0.097	0.079	0.8	0.006	1.367	270.080	270.003	72	271.49	1.180	()	()	()
Т6	Т9	269.661	269.269	525	N/A	87.0	0.5			272.520	1.333	0.288	0.202	0.7	-0.106	1.367	270.080	269.951	74	271.18	0.870			
T7	T8	269.494		450	N/A	120.0	0.9			271.766	100000000000000000000000000000000000000	0.263	0.187	0.7	0.059	1.367		269.594	B128	270.36	0.127			
							Value of the last												B128	270.36	0.531	120.2	1.4	269.599
																			63	270.47	0.641	120.2	1.4	269.599
																			64	270.55	0.629	120.2	28.4	269.691
																			65	270.55	0.629	120.2	28.4	269.691
																			66	270.65	0.640	120.2	54.6	269.780
																			67	270.70	0.690	120.2	54.6	269.780
																			68	270.70	0.604	120.2	80.0	269.866
																			69	270.84	0.744	120.2	80.0	269.866
																			70	270.84	0.654	120.2	106.3	269.956
																			71	270.89	0.704	120.2	106.3	269.956
																			98	271.09	0.926	120.2	99.8	269.934
																			99	271.09	0.926	120.2	99.8	269.934
																			100	270.99	0.916	120.2	73.6	269.844
																			101	270.82	0.746	120.2	73.6	269.844
																			102	270.72	0.735	120.2	47.4	269.755
																			103	270.64	0.655	120.2	47.4	269.755
																			104	270.58	0.683	120.2	21.5	269.667
то	T12	000 044	007.004	600	NI/A	20.0	4.4	0.040	074 700	074 500	0.070	0.044	0.005	0.0	0.750	4 000	200 504	000 007	105	270.40	0.503	120.2	21.5	269.667
T8 T9	T12 T10		267.881	600 600	N/A	33.0	1.1		271.766		2.278	0.644	0.395	0.6	0.750	1.333	269.594	269.387	Part4	270.20	0.376			
T10	T11	269.194 269.100	269.150	600	N/A N/A	14.5	0.3	500000000000000000000000000000000000000	1000	272.353 271.833		0.336	0.238	0.7	0.157	1.367	269.951 269.927	269.927	82 B133	271.14 270.20	0.959 0.043			
110	111	269.100	200.125	000	IN/A	97.5	1.0	0.013	212.353	2/1.033	2.172	0.614	0.415	0.7	0.227	1.367	209.921	269.521	B133	270.20	0.444	97.3	1.3	269.526
																			90	270.25	0.432	97.3	16.1	269.588
																			89	270.23	0.492	97.3	16.1	269.588
																			88	270.37	0.445	97.3	41.8	269.695
																			87	270.44	0.515	97.3	41.8	269.695
																			86	270.50	0.465	97.3	68.0	269.805
																			85	270.57	0.535	97.3	68.0	269.805
																			84	270.63	0.486	97.3	94.2	269.914
																			83	270.63	0.486	97.3	94.2	269.914
																			97	271.13	1.023	97.3	85.2	269.877
																			96	271.13	1.023	97.3	85.2	269.877
																			95	270.76	0.762	97.3	59.1	269.768
																			94	270.76	0.762	97.3	59.1	269.768
																			93	270.59	0.702	97.3	32.9	269.658
																			92	270.59	0.702	97.3	32.9	269.658
																			91	270.40	0.576	97.3	17.4	269.594
T11	T12	267.967	267.811	750	N/A	39.0	0.4	0.013	271.833	271.539	1.594	0.704	0.510	0.7	0.804	1.483	269.521	269.387	Part3	270.15	0.399			
T12	52	267.731	267.595	750	N/A	19.5	0.7	0.013	271.539	271.337	2.108	0.931	0.853	0.9	0.906	1.483	269.387	269.273	N/A	N/A	N/A			
T13	T14	269.222	268.800	600	N/A	105.5	0.4	0.013	272.254	272.129	1.373	0.388	0.227	0.6	-0.263	1.367	269.559	269.385	B123W	270.50	0.711			
T14	T15	268.750	268.442	600	N/A	44.0	0.7	0.013	272.129	271.890	1.817	0.514	0.333	0.6	0.035	1.350	269.385	269.278	Part14	270.22	0.605			
T15	T21	268.367	268.121	675	N/A	41.0	0.6	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	271.890		1.820	0.651	0.494	8.0	0.236	1.367	269.278	269.155	Part15W	270.06	0.552			
T16	T17	269.547	269.251	375	N/A	74.0	0.4		272.395		1.004	0.111	0.040	0.4	-0.215	1.300	269.707	269.406	112	271.11	1.173			
T17	T18		Anna Carlo C	450	N/A	58.5	0.5		272.209		1.268	0.202	0.097	0.5	-0.220	1.350	269.406	269.294	Part41	270.50	0.864			
T18	T19	268.834	268.738	450	N/A	12.0	0.8		271.888		1.603	0.255	0.102	0.4	0.010	1.483	269.294	269.280	Part42	270.30	0.776			
T19	T20	ARRONAL CONTRACTOR		525	N/A	36.0	0.6	2,100,000,000,000	271.741		1.473	0.319	0.141	0.4	0.092	1.483	269.280	269.255	Part46	270.21	0.700			
T20	T21			675	N/A	24.0	0.6		271.527		1.820	0.651	0.351	0.5	0.315	1.317	269.255	269.155	Part22	270.46	0.975			
T21	39	267.971	267.755	825	N/A	36.0	0.6	resonance reconstruction	271.662		2.080	1.112	0.819	0.7	0.359	1.500		269.036	Part16W	269.91	0.525			
CMH3	4			375	N/A	11.5	0.5		Control of the Contro	268.203	1.123	0.124	0.049	0.4	-0.051	1.433	265.016	264.998	N/A	N/A	N/A			
Y1	Y2	265.512	265.222	300	N/A	29.0	1.0	0.013	268.294	268.509	1.368	0.097	0.098	1.0	-0.047	1.283	265.765	265.465	69	266.80	0.805			

U/S	D/S	U/S	D/S	Pipe Dia.	Pipe	Pipe	Pipe	n	U/S MH	D/S MH	Design	Design	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	1000 E-1000	erpolated H	
MH	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			Length	Dist. From	HGL
		///			7	<i>t</i> ×	(0()		Elev.	Elev.	/ · / · ·	(203/2)	Flow	Flow	(1)	Peak	HGL	HGL		<i>t</i> =1		HGL	D/S MH	
Y2	Y3	(m) 265.172	(m) 264.022	(mm) 300	(mm) N/A	(m) 60.5	(%) 1.9	0.013	(m) 268.509	(m) 268.054	(m/s) 1.886	(m ³ /s) 0.133	(m ³ /s) 0.142	1.1	(m) -0.025	(h) 1.317	(m) 265.447	(m) 264.303	41	(m) 266.52	(m) 0.843	(m)	(m)	(m)
Y3	Y4	263.722	263.410	600	N/A	44.5	0.7	3.5527-003-003-003-0	268.054	267.689	1.817	0.133	0.142	0.7	-0.023	1.350	264.303	264.173	37	266.12	1.587			
Y4	Y7	262.810	262.550	1200	N/A	65.0	0.4		267.689	267.220	2.180	2.466	2.567	1.0	0.163	1.400	264.173	263.433	33	265.85	1.447			
Y5	Y6	264.608	264.230	675	N/A	75.5	0.5	0.013	268.358	267.827	1.661	0.594	0.590	1.0	0.039	1.383	265.322	264.972	26	266.26	0.708			
Y6	Y7	264.155	263.743	750	N/A	82.5	0.5	0.013	267.827	267.220	1.782	0.787	0.776	1.0	0.067	1.383	264.972	264.289	32	265.75	0.548			
Y7	Y25	261.367	261.034	1350	N/A	83.2	0.4	0.013	267.220	266.477	2.358	3.376	3.917	1.2	0.228	1.400	262.945	262.537	N/A	N/A	N/A			
Y21	Y22	263.484	262.846	375	N/A	63.8	1.0	0.013	267.204	266.726	1.587	0.175	0.153	0.9	-0.097	1.350	263.762	263.167	63	265.38	1.388			
Y22	Y25	262.771	261.882	450	N/A	70.9	1.3	0.013	266.726	266.477	2.004	0.319	0.275	0.9	-0.054	1.367	263.167	262.537	59	265.11	1.713			
Y23 Y24	Y24	263.512	263.174	375 525	N/A	33.8	1.0	0.013	267.341	267.201	1.587	0.175	0.065	0.4	-0.208	1.367	263.679	263.465	49	265.94	2.031			
Y25	Y25 Y33	263.024	261.800	1500	N/A N/A	116.5	1.1	0.013	267.201	266.477	2.036	0.441	0.386	0.9	-0.084	1.367	263.465	262.537	56 EUT	265.14	1.445			
Y26	Y27	260.862 263.562	260.558 262.843	375	N/A	76.0 71.9	0.4 1.0	0.013	266.477 267.410	265.820 266.435	2.530 1.587	4.471 0.175	4.565 0.085	1.0 0.5	0.175 -0.172	1.400 1.367	262.537 263.765	262.245 263.028	FUT FUT	264.91 265.84	2.140 1.845			
Y27	Y28	262.693	262.277	525	N/A	83.3	0.5	0.013	266.435	265.950	1.405	0.173	0.163	0.5	-0.172	1.367	263.000	262.892	FUT	264.87	1.635			
Y28	Y33	262.247	261.503	525	N/A	74.4	1.0	0.013	265.950	265.820	1.987	0.430	0.384	0.9	0.120	1.367	262.892	262.245	FUT	264.38	1.258			
Y29	Y30	264.287	263.726	300	N/A	51.0	1.1	maline Process	267.240	266.842	1.435	0.101	0.048	0.5	-0.150	1.367	264.437	263.871	FUT	265.67	1.003			
Y30	Y31	263.181	263.100	375	N/A	11.2	0.7	X180 X180 X180 X180 X180 X180 X180 X180	266.842	Service Control	1.328	0.147	0.073	0.5	-0.169	1.383	263.387	263.285	FUT	265.27	1.655			
Y31	Y32	262.621	262.259	525	N/A	72.2	0.5	0.013	266.715	266.108	1.405	0.304	0.125	0.4	-0.287	1.367	262.859	262.613	FUT	265.15	2.056			
Y32	Y33	262.215	261.503	525	N/A	71.2	1.0	0.013	266.108	265.820	1.987	0.430	0.278	0.6	-0.127	1.367	262.613	262.245	FUT	264.54	1.695			
Y33	Y34	260.528	260.208	1500	N/A	71.2	0.5	0.013	265.820	265.456	2.683	4.742	5.351	1.1	0.217	1.417	262.245	261.861	FUT	264.25	1.775			
T101	T102	269.597	269.263	450	N/A	83.5	0.4		272.551	272.327	1.134	0.180	0.116	0.6	-0.176	1.367	269.871	269.563	B7S	270.63	0.529			
T102	T104	269.213	269.177	450	N/A	9.0	0.4		272.327	272.440	1.134	0.180	0.113	0.6	-0.100	1.383	269.563	269.478	B5N	270.85	1.057			
T103	T104	269.612	269.252	375	N/A	45.0	0.8	CONTRACTOR CONTRACTOR	272.477	272.440	1.420	0.157	0.144	0.9	-0.088	1.367	269.899	269.533	B4E	270.61	0.481			
T104 T105	T105 T106	268.952	268.636	675 675	N/A N/A	79.0	0.4		272.440		1.486	0.532	0.400	0.8	-0.149	1.367	269.478	269.297	B2W	270.60	0.892			
T106	T107	268.586 268.504	268.554 268.360	675	N/A	8.0 36.0	0.4	150000000000000	272.063 271.980	271.980	1.486 1.486	0.532 0.532	0.425 0.616	0.8 1.2	0.036 0.091	1.383	269.297 269.270	269.270 269.110	B1N B1S	270.31 270.15	0.783 0.650			
T107	T5	268.310	268.248	675	N/A	12.5	0.5		271.935	700000111111111111111111111111111111111	1.661	0.594	0.614	1.0	0.125	1.367	269.110	268.916	N/A	N/A	0.030 N/A			
70A	75	258.610	258.512	1500	3000	21.8	0.5	0.013	263.870	263.400	3.251	14.628	19.558	1.3	0.417	1.433	260.527	260.332	N/A	N/A	N/A			
V1	V3	261.668	260.688	300	N/A	24.5	4.0		265.632	264.642	2.736	0.193	0.027	0.1	-0.219	1.350	261.749	261.383	BF27/28	263.31	1.331			
V2	V3	261.213	260.618	375	N/A	59.5	1.0		264.947	264.642	1.587	0.175	0.091	0.5	-0.067	1.333	261.521	261.383	BJ68/69	262.60	0.849			
V3	V5	260.538	260.282	450	N/A	36.5	0.7	0.013	264.642	264.402	1.500	0.239	0.144	0.6	0.395	1.417	261.383	261.292	BE22	262.65	1.037			
V4	V5	261.021	260.357	375	N/A	83.0	8.0	0.013	264.768	264.402	1.420	0.157	0.169	1.1	0.613	1.317	262.009	261.292	BG38/39	262.52	0.281			~
																		ž.	BG38/39	262.52	0.849	83.1	17.3	261.44
																			BG40/41	262.52	0.737	83.1	30.2	261.55
																			BG42/43	262.52	0.626	83.1	43.1	261.66
																			BH49/50	262.60	0.572	83.1	58.7	261.79
																			BH51/52	262.60	0.460	83.1	71.6	261.91
																			BH53 BI54/55	262.60 262.93	0.387 0.914	83.1 83.1	80.1 57.3	261.983 261.786
																			BI56/57	262.93	1.025	83.1	44.4	261.67
																			BJ62/63	262.87	1.100	83.1	28.7	261.540
																			BJ64/65	262.60	0.942	83.1	15.8	261.428
V5	V9	260.207	260.008	525	N/A	28.5	0.7	0.013	264.402	264.136	1.662	0.360	0.287	0.8	0.560	1.383	261.292	261.167	BE18/19	262.38	0.858			
V6	V7	261.300	261.210	300	N/A	9.0	1.0	0.013	264.061	264.167	1.368	0.097	0.019	0.2	-0.194	1.267	261.406	261.343	N/A	N/A	N/A			
V7	V9	260.573		450	N/A	107.0	0.5	100000000000000000000000000000000000000		264.136	1.268	0.202	0.128	0.6	0.320	1.283	261.343	261.167	BC13/14	262.14	0.567			
V8	V9	260.398	260.183	300	N/A	19.5	1.1		264.162		1.435	0.101	0.086	0.8	0.685	1.350	261.383	261.167	BD15/16	262.36	0.747			
V9	V10	259.958	259.508	525	N/A	22.5	2.0		264.136		2.810	0.608	0.536	0.9	0.684	1.367	261.167	260.826	N/A	N/A	N/A			
V101	70A	259.420	259.080	600	N/A	17.0	2.0	0.013	264.077	263.870	3.071	0.868	0.534	0.6	0.806	1.367	260.826	260.538	N/A	N/A	N/A			
Y101	Y102	268.078	266.768	375	N/A	65.5	2.0	0.013	270.964	269.863	2.245	0.248	0.226	0.9	-0.045	1.350	268.408	267.066	32	268.65	0.012	6F 7	24.2	267 500
																			32 31	268.65 268.70	0.858 0.908	65.7 65.7	24.3 24.3	267.562 267.562
																			30	268.91	0.609	65.7	49.2	268.07
																			29	269.11	0.809	65.7	49.2	268.071
																			28	269.33	0.719	65.7	64.4	268.381

Table F								, the										900 (Miles - 100)						
U/S	D/S	U/S	D/S	Pipe Dia.	and the same	Pipe	Pipe	n	U/S MH	D/S MH	Design	1000	Peak	Peak /		Time	Max.	Max.	Lot	BF	Freeboard		erpolated H	
MH	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			1000	Dist. From	HGL
									Elev.	Elev.		, 3, s	Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)	07	(m)	(m)	(m)	(m)	(m)
V400	V402	000 040	005.040	F0E	NI/A	07.0	4.0	0.040	000 000	000 040	4.007	0.400	0.440	4.0	0.077	4.007	007.000	000 000	27	269.55	0.939	65.7	64.4	268.381
Y102	Y103	266.618	265.648	525	N/A	97.0	1.0	0.013	269.863	268.810	1.987	0.430	0.442	1.0	-0.077	1.367	267.066	266.093	91	267.53	0.234	07.0	40.4	000 047
																			91	267.53	1.083	97.2	12.4	266.217
																			92	267.78	1.333	97.2	12.4	266.217
																			93	267.98	1.313	97.2	34.4	266.437
																			94 95	267.98 268.01	1.313 1.122	97.2 97.2	34.4 56.4	266.437 266.658
																			96	268.29	1.402	97.2	56.4	266.658
																			97	268.29	1.184	97.2	78.2	266.876
																			98	268.49	1.384	97.2	78.2	266.876
Y103	Y5	265.491	264.688	600	N/A	73.0	1.1	0.013	268.810	268.358	2.278	0.644	0.536	0.8	-0.001	1.367	266.090	265.322	FUT	267.24	0.920	01.2	7 0.2	200.010
Y110	Y111	266.309		600	N/A	108.0	0.4	Charles College	269.419	***************************************	1.373	2022/10/20	0.397	1.0	-0.081	1.367	266.828	266.424	1	268.09	1.032			
																			1	268.09	1.042	107.9	105.3	266.818
																			2	268.09	1.042	107.9	105.3	266.818
																			3	268.10	1.109	107.9	90.0	266.761
																			4	268.42	1.429	107.9	90.0	266.761
																			5	268.77	1.861	107.9	68.0	266.679
																			6	268.77	1.861	107.9	68.0	266.679
																			7	268.93	2.104	107.9	46.0	266.596
																			8	269.15	2.324	107.9	46.0	266.596
																			9	269.37	2.626	107.9	24.0	266.514
	_														2 9//				10	269.37	2.626	107.9	24.0	266.514
Y111	Y112	265.652	265.184	825	N/A	117.0	0.4	0.013	270.586	272.103	1.698	0.908	0.969	1.1	-0.053	1.383	266.424	265.974	11	269.63	2.976			
Y112	Y113	265.109	264.829	900	N/A	70.0	0.4	0.013	272.103	271.237	1.800	1.145	0.974	0.9	-0.035	1.450	265.974	265.619	26	269.89	3.686			
Y113	Y114	264.749	264.397	900	N/A	88.0	0.4	0.013		270.497	1.800	1.145	1.108	1.0	-0.030	1.450	265.619	265.265	B111S	268.07	2.221			
Y114	Y115	264.347	264.277	900	N/A	17.5	0.4	0.013		270.335	1.800	1.145	1.121	1.0	0.018	1.450	265.265	265.151	B114S	268.06	2.565			
Y115	Y116	264.202	263.720	975	N/A	120.5	0.4	0.013		269.418	1.898	1.417	1.368	1.0	-0.026	1.450	265.151	264.704	B109W	267.25	1.869			
Y116	Y117	PARTITION DOCUMENTS	263.624	975	N/A	11.5	0.4	0.013			1.898	1.417	1.439	1.0	0.059	1.450	264.704	264.557	B109E	267.25	2.316			
Y117	Y4		262.890	1200	N/A	76.0	0.4	0.013		267.689	2.180	2.466	2.003	0.8	0.163	1.400	264.557	264.173	FUT	267.66	2.871			
Y1110	Y111		265.757	750	N/A	14.0	0.4	0.013		270.586	1.594	0.704	0.410	0.6	-0.057	1.483	266.506	266.424	N/A	N/A	N/A			
Z1	Z4		266.998		N/A	25.5	4.0			270.005			0.000	0.0	-0.300	0.000	268.018	266.640	BLL66	269.03	0.782			
Z2	Z6	266.800	266.542	375	N/A	64.5	0.4	0.013	269.671	269.651	1.004	0.111	0.091	0.8	-0.111	1.283	267.064	266.846	BEL20	267.75	0.456			000 000
																			BEL20	267.75	0.634	64.4	11.8	266.886
																			BEL17	267.89	0.733	64.4	24.0	266.927
																			BDL16	267.89	0.681	64.4	39.3	266.979
Z3	Z4	266.636	266 222	450	N/A	78.5	0.4	0.012	260 747	270.005	1 124	0.180	0.175	1.0	-0.021	1 300	267.065	266.640	BDL12 BIL55	267.78 267.87	0.502 0.575	64.4	59.6	267.048
20	2-7	200.030	200.322	1 400	13//	70.5	0.4	0.013	203.141	210.005	1.134	0.100	0.175	1.0	-0.021	1.300	207.005	200.040	BIL55	267.87	0.575	78.5	14.5	266.719
																			BIL40	268.09	1.026	78.5	35.8	266.834
																			BJL50	268.09	0.938	78.5	52.1	266.922
																			BJL46	267.71	0.443	78.5	73.4	267.037
																			BCL11	267.71	0.532	78.5	75.3	267.037
																			BCL8	268.32	1.108	78.5	63.1	266.982
																			BBL7	268.32	1.195	78.5	47.1	266.895
																			BBL4	268.72	1.661	78.5	34.9	266.829
																			BAL3	268.72	1.737	78.5	20.9	266.753
																			BAL1	268.85	1.911	78.5	12.8	266.709
Z4	Z7	266.242	266.102	450	N/A	35.0	0.4	0.013	270.005	269.236	1.134	0.180	0.185	1.0	-0.052	1.317	266.640	266.508	BKL60	267.83	0.960			
Z 5	Z 6	266.785		300	N/A	21.0	0.8		269.764		1.224		0.010	0.1	-0.229	100000000000000000000000000000000000000	266.856	266.846	BFL24	267.69	0.604			
			is a			0100000 D									soft street and				BFL24	267.69	0.606	20.9	15.7	266.854
																			BFL21	267.75	0.672	20.9	3.5	266.848
Z6	Z7	266.392	266.032	525	N/A	90.0	0.4	0.013	269.651	269.236	1.256	0.272	0.236	0.9	-0.071	1.317	266.846	266.508	BHL35	267.28	0.204			
																			BHL35	267.28	0.481	89.9	16.1	266.569

Table A-1: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2)

U/S	D/S	U/S	D/S	Pipe Dia.	Pipe	Pipe	Pipe	n	U/S MH	_	Design	_	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	In	terpolated H	GL
МН	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			Length	Dist. From	HGL
									Elev.	Elev.			Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																			BHL30	267.42	0.530	89.9	40.3	266.660
																			BGL29	267.45	0.511	89.9	53.5	266.709
																			BGL25	267.59	0.575	89.9	73.6	266.785
																			BJL45	267.71	0.698	89.9	73.0	266.782
																			BJL41	267.78	0.848	89.9	51.7	266.702
																			BIL40	267.78	0.909	89.9	35.4	266.641
																			BIL36	267.64	0.849	89.9	14.1	266.561
Z7	Z8	265.952	265.841	600	N/A	18.5	0.6	0.013	269.236	268.839	1.682	0.476	0.421	0.9	-0.044	1.333	266.508	266.266	BKL56	267.13	0.392			
70	701414			000	11/A														BKL56	267.13	0.444	18.7	14.7	266.456
Z8		265.761		600	N/A	26.5	1.2	0.013	The second second	1700-000	2.379	0.673	0.555	0.8	-0.171	1.350		265.859	N/A	N/A	N/A			
ZCMH1	Y117	W. S. C.	BUCKESSAN COLUMNS	675	N/A	13.0	0.5	grand and collect	268.463	ESCHEDISTRATION OF THE PROPERTY OF THE PROPERT	1.661	0.594	0.552	0.9	0.133	1.350		264.557	N/A	N/A	N/A			
MH2-1	MH2-3	NAME OF TAXABLE PARTY.	The second secon	300	N/A	22.2	1.0		269.280		1.368	0.097	-0.004	0.0	-0.248	1.267	265.169	265.167	FUT	267.71	2.311			
		265.347		300	N/A	39.3	1.0		268.540		1.368	0.097	0.133	1.4	0.545	1.283	266.192	265.228	FUT	266.97	0.548			
	MH2-4			375	N/A	7.1	0.5	maline Process	269.080	was the control of the control	1.123	0.124	0.133	1.1	-0.083	1.283	265.167	265.109	FUT	267.51	2.113			
	Committee of the Commit	264.765		450	N/A	53.1	0.5	X 200	268.960		1.268	0.202	0.157	0.8	-0.145	1.350		264.810	FUT	267.39	2.090			
		264.449		450	N/A	10.4	0.5	SCOR SCORESSORAL	268.550		1.268	0.202	0.186	0.9	-0.089	1.350		264.732	FUT	266.98	1.940			
MH2-6		264.347		450	N/A	5.4	0.5	0.013			1.268	0.202	0.198	1.0	-0.065	1.367	264.732	264.672	FUT	266.89	1.928			
MH2-7		264.270	1970400000000000000000000000000000000000	450	N/A	28.1	0.5	0.0000000000000000000000000000000000000	268.420	1957/50/50/095/095/095/095	VV MANAGONES	100000000000000000000000000000000000000	0.234	1.2	-0.048	1.350		15 00 00 00 00 00 00 00 00 00 00 00 00 00	FUT	266.85	1.948			
MH1	Y3	263.830	263.779	600	N/A	10.3	0.5	0.013	268.180	268.054	1.536	0.434	0.233	0.5	-0.065	1.350	264.365	264.303	FUT	266.61	2.015			

Note: (1) A negative surcharge implies that the pipe is not flowing full

266.886 Interpolated HGL elevation

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

1	U/S	D/S	U/S	D/S	Pipe Dia.		Pipe	Pipe	n	U/S MH		-		Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	Int	arnolated H	GI
1		1000000000	020 00	700				100	11	1020	520	200	W 100 TO 1		1000 00	and the second second	19%		200700000		DE	rieeboaiu	0.000 H-000		0.000
1		3300.2			, , , , , ,						2000					790,000	200								
2 3 3 251-50			(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)	1012-000000	(m)	(h)		(m)		(m)	(m)	(m)	(m)	(m)
A	1	2	265.800	265.200	825	N/A	120.0		0.013	269.380	268.730	1.899	1.015	0.682	0.7	-0.327	1.367	266.298	265.708	N/A	N/A	N/A			
4 S 5 204.44 503.87 0.5 NA 16.5 NA 16.	2	3	265.150	264.800	825	N/A	69.9	0.5	0.013	268.730	268.432	1.899	1.015	0.799	0.8	-0.267	1.367	265.708	265.341	N/A	N/A	N/A			
5 8 00 20 20 20 20 20 20 20 20 20 20 20 20	3		264.747	84-84 (STORE) (CONT. 10)		10000000	42.0	TOOK SHOULD	0.013			Control Control Control	1.112	3.562.365.0 (50.05)	0.7	N. 10 . 10 . 10 . 10 . 10 . 10 . 10 . 10	S. C. C. Market and C. C.			N/A	50,300,000	21.00000000			
8	4	86822				W-1144000000															2500100,000100,00000				
8 8 8 8 8 8 8 8 8 8	2000	15000					DOMESTIC STATE	200-200-200-200	0.0000000000000000000000000000000000000						40.00		Mary 2001 12 Williams		l						
1	Last 1	62500			2000000000	100000000000000000000000000000000000000		100000000000000000000000000000000000000					200 March 2000				777				The second second	2.000			
9 1 0 2 50.5 0 24.0 50.5 0 24.0 50.5 0 24.0 50.5 0 24.0 1.0 2 0 20.5 0 24.0 1.0 2 0 20.5 0 24.0 1.0 2 0 20.5 0 24.0 1.0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2			300000000000000000	SCHOOL STONE		1357	5,002,00008	5000000000	5555-00-00-00-00-00-00-00-00-00-00-00-00	\$100.00 (0.000 (0.000)	and the second second	2120027 120027	500000000000000000000000000000000000000	A 10.00 A 34 II 20.00 A 34	CONTRACT	3310351, 034000					No. Williams	0.08400.005			
14 27 28 48.76 28.605 676 NA 42.00 1 076 NA 42.00 1 08 013 28.807 88.04 28.00 1 10 09 05 05 03 28.006 28.00 1 10 09 05 05 05 05 05 05 05 05 05 05 05 05 05	15	V-000			(ASMIRE)	61336500																			
11 27 27 24 2422 252976 169 NA 42.0 0.0 0.13 26790 288-04 24.0 21 24.0 25.0 24.0 24.0 24.0 24.0 24.0 24.0 24.0 24							17170011111111111	***************************************							10000000		Vanis - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -				100000000000000000000000000000000000000				
12		100000000000000000000000000000000000000			1050	100 to 200 to 200 to	7.00	100	100000000000000000000000000000000000000								0. V. V.					497.797			
14	12	27			450	N/A	59.0	0.5	Contract Contract	STATE OF STA	2007-00-00-00-00-00-00-00-00-00-00-00-00-		The same of the same of	2.000	0.7	0.482	27127		and the second second	1	266.89	0.857			
1	13	14	268.238	268.064	450	N/A	43.5	0.4	0.013	271.187	271.513	1.134	0.180	0.084	0.5	-0.217	1.400	268.471	268.370	28	269.71	1.009			
1	14	15	267.989	267.809	525	N/A	45.0	0.4	0.013	271.513	271.276	1.256	0.272	0.165	0.6	-0.144	1.350	268.370	268.301	B60	269.71	1.110			
17	15	16	267.734	267.494	600	N/A	60.0	0.4	0.013	271.276	270.954	1.373	0.388	0.362	0.9	-0.033	1.350	268.301	267.888	B61	269.63	1.099			
27 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	16	17	267.414	266.070	600	N/A	112.0	1.2	0.013	270.954	269.291	2.379	0.673	0.448	0.7	-0.239	1.350	267.775	266.516			-0.145			
1																						per representation			
18																					AND DESCRIPTION OF THE PARTY OF	15.00 - 10.00 (0.00 cm)	940000000000000	M9000111000011	Service and the service
18																									
17																					6.6700.000-0000.000		ECONOCCOUNT AN	17500 17500 A	A contract of the high region and
17																									
18	17	10	205 000	004.750	675	NI/A	77.5	4.5	0.040	200 204	200 500	0.077	4.000	0.000	0.0	0.070	4.050	000 F40	005 700		2000, 100 and	-	111.9	89.7	267.525
1		1002000			200000000000000000000000000000000000000	20027-0000		1.25					300000000000000000000000000000000000000				2000					100000000000000000000000000000000000000			
20			VONOS ASSESSMENTAL SECTION AND ASSESSMENT	100000000000000000000000000000000000000			17-11-11-11-11-11	S-03-1X							AC SALVANI						No. of Contract Contr				
24 25 25 264.769 264.759 265.379 300 N/A 14.5 1.1 0.013 268.676 268.540 1.266 0.532 0.456 0.454 0.104 0.532 0.365 0.456 0.476 0.104 0.532 0.456 0.45		11000000			Para Control Control	X203350-001		0.000.000000000000000000000000000000000							2000						***************************************				
B5SW 267.30 0.455 104.7 27.1 266.615 B5SW 267.34 0.455 104.7 27.1 266.615 B5SW 267.34 0.461 104.7 42.5 25.6 B5SW 267.34 0.462 104.7 48.0 25.6	1.0000000		100 000000 00000000		110000000000000000000000000000000000000		000000000000000000000000000000000000000	2000.2000.00	100000000000000000000000000000000000000	CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC	197000000000000000000000000000000000000	100 (000)000,000	0.0000000000000000000000000000000000000	***************************************	2000 0000		1000 VS. (2000 A. (2000 A.		TARREST A SAME RESTRICT						
BSE			200.401	200.010	5.5	1.071	104.0	0.4	0.010	200.000	200.470	1.400	0.002	0.000	0.7	0.070	1.000	200.707	200.000				104.7	9.1	266.575
Beford B																					SORGEST STROUGH	VSE-01574035577.	100.400.000.000	90000000	the second of the second of the second
22 25 264.969 264.789 675 N/A 36.0 36																						0.461	104.7	42.5	
22 25 264.969 264.789 675 N/A 36.0 0.5 0.013 268.276 268.540 1.661 0.594 0.043 0.2 0.045 0.2 0.477 0.689 0.489 0.645 0.624 0.645 0.624 0.477 0.699 0.477																				B56E	267.38	0.448	104.7	66.5	266.702
22 25 264.969 264.789 675 N/A 36.0 0.5 0.013 268.476 268.694 1.681 0.594 0.634 1.1 0.911 1.383 266.595 266.147 N/A N/A N/A N/A 1.5 1.1 0.013 268.595 268.695 1.435 0.104 2.288.595 2.286.295 2.286																				B57W	267.42	0.454	104.7	81.9	266.736
22 25 264.969 264.769 675 N/A 36.0 0.5 0.013 268.476 268.540 1.268 0.202 0.046 0.22 0.477 1.002 0.681 1.367 266.147 366 266.742 265.424 265.249 263.961 1.350 N/A 80.5 0.7 0.013 268.247 267.163 2.511 1.875 1.895 1.00 0.688 1.383 266.551 266.349 268.541 2.68																				B57E	267.71	0.699	104.7	101.9	266.781
22 25 264.969 264.789 675 N/A 36.0 0.5 0.013 268.476 268.699 1.435 0.101 0.043 0.4 0.346 1.367 266.185 266.147 267.06 0.645 267.06 0.645 267.06 0.645 267.06 0.645 267.06 0.645 267.06 0.645 268.699 0.604 268.699 0.6																				B162W	267.30	0.462	104.7	23.8	266.608
22 25 264.969 264.789 675 N/A 36.0 0.5 0.013 268.476 268.504 1.661 0.594 0.634 1.1 0.911 1.383 266.555 266.147 N/A N																				B162E	267.38	0.489	104.7	48.0	266.661
22																					CONTRACTOR OF STREET	0.521		STATE OF THE PARTY	The second second second second
23																							104.7	97.2	266.770
24								97721 TX						AND THE PROPERTY OF	0.000				and the second second second second						
25		1/20/2007	was a second second second		NA CONTRACT	000000000000	20000000	800000000	0.0000000000000000000000000000000000000		more and a second	WO MOSCOCKO	A CANADA DA CANADA D	POINT TOWNS	825-7700	and the second	60000000000000		appropriate the second		20.000000000000000000000000000000000000	LECTRONICS:			
26		400000		100000000000000000000000000000000000000	0.000					A CONTRACTOR OF THE PARTY OF TH	The second second	1000			1000										
27 28 263.676 263.361 1350 N/A 90.0 0.4 0.013 268.046 267.503 2.206 3.158 3.187 1.0 0.632 1.433 265.658 265.315 B68 265.315 B68 265.315 B68 0.285	12.5	11/21/2007			1		CILLEGIO CONTRACTO	COLCUMENT					Same and Charles of	ACCORDONACIONOS		100000000000000000000000000000000000000									
28		27.4			0000000		2.0000000000000000000000000000000000000	10000000001	a base transcription						200										
12 265.83 0.706 99.9 15.2 264.894 13 265.83 0.642 99.9 28.0 264.958 14 265.83 0.642 99.9 28.0 264.958 15 265.91 0.609 99.9 50.8 265.071 16 266.06 0.759 99.9 50.8 265.071 17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283	0.00	2000					20070000000000000	BARROWS	NOT THE RESERVE OF TH			1900-000-000-000	400000000000000000000000000000000000000	V 240700000000000	556 550	0.0000000000000000000000000000000000000	0.0 00000000								
13 265.83 0.642 99.9 28.0 264.958 14 265.83 0.642 99.9 28.0 264.958 15 265.91 0.609 99.9 50.8 265.071 16 266.06 0.759 99.9 50.8 265.071 17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283	20	29	263.211	262.874	1500	IN/A	112.5	0.3	0.013	267.503	267.163	2.191	3.872	3.740	1.0	0.604	1.417	205.315	264.819				00.0	15.0	264.904
14 265.83 0.642 99.9 28.0 264.958 15 265.91 0.609 99.9 50.8 265.071 16 266.06 0.759 99.9 50.8 265.071 17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283																							1500593500		and the second party of the second second
15 265.91 0.609 99.9 50.8 265.071 16 266.06 0.759 99.9 50.8 265.071 17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283																								Name and the second	
16 266.06 0.759 99.9 50.8 265.071 17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283																					VID. 000 000 000 000 000 000 000 000 000 0	A0177-05-08-00-00-01		00000000000	Account to the second
17 266.29 0.882 99.9 72.4 265.178 18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283																									
18 266.29 0.882 99.9 72.4 265.178 19 266.17 0.657 99.9 93.4 265.283																					Society of the State of the Sta	820080000000	The second second	57.555.5556.1	recognition and the second
19 266.17 0.657 99.9 93.4 <mark>265.283</mark>																							1,000,000,000		and the second second
																					200720000000000000000000000000000000000	102-0702-020000	0.0000000000000000000000000000000000000	20000000000	

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

U/S	D/S	U/S	D/S	Pipe Dia.	Pipe	Pipe	Pipe	n	U/S MH	D/S MH	Design	Design	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	In	terpolated H	IGL
МН	MH	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			Length	Dist. From	HGL
									Elev.	Elev.			Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																			22	266.25	0.756	99.9	89.6	265.26
																			23	266.24	0.746	99.9	89.6	265.26
																			24	266.10	0.729	99.9	64.9	265.14
																			25	266.21	0.839	99.9	64.9	265.14
																			26	266.21	0.949	99.9	42.6	265.03
29	30	000 004	262.429	1500	N/A	70.0	0.5	0.040	007.400	007.400	0.000	4 000	F 004	4.0	0.405	4 447	004.040	000.070	27	266.00	0.739	99.9	42.6	265.03
30	30 35	262.824 262.335	261.885	1500	N/A	79.0 53.0	0.5	0.013 0.013	267.163 267.122	267.122	2.829 3.688	4.998 6.517	5.861 7.380	1.2 1.1	0.495 0.037	1.417 1.433	264.819 263.872	263.872 263.283	120 N/A	265.55 N/A	0.501 N/A			
31	32		262.909	600	N/A	37.5			266.448		1.373		0.370		0.456		264.115	263.991	Part12	265.30	0.955			
32	30	262.859	262.785	600	N/A	18.5	0.4	0.013 0.013	266.795	267.122	1.373	0.388	0.369	1.0 1.0	0.430	1.333 1.333	263.991	263.872	N/A	N/A	0.955 N/A			
35	100	261.835	261.679	1500	2400	29.5	0.5	0.013	266.843	266.641	3.345	12.040	7.545	0.6	-0.052	1.433	263.283	263.243	B122S	264.97	1.457			
36	68	The second second second	260.762	1500	2400	120.0	0.5	0.013	266.363		3.345	12.040	8.134	0.7	-0.326	1.433	262.572	262.039	B95S	264.27	1.468			
38	40	268.456		375	N/A	70.0	0.8	0.013	271.790		1.420	0.157	0.065	0.4	-0.025	1.350	268.806	268.742	57	270.08	1.044			
39	40		267.451	900	N/A	43.0	0.4		271.464		1.800	1.145	1.056	0.9	0.513	1.367	269.036	268.742	N/A	N/A	N/A			
40	44		267.061	900	N/A	77.5	0.4	A STATE OF THE PARTY OF THE PAR	271.439		1.800	1.145	1.135	1.0	0.471	1.367	268.742	268.272	62	269.77	0.798			
41	44	Control of the Contro	267.061	900	N/A	43.0	0.4	0.013	270.975		1.800	1.145	1.340	1.2	0.369	1.400	268.502	268.272	N/A	N/A	N/A			
42	43		267.911	300	N/A	38.5	1.0		271.446		1.368	0.097	-0.004	0.0	-0.272	1.433	268.324	268.322	13	269.79	1.236			
43	44		267.511	450	N/A	50.0	0.5		271.302		1.268	0.202	0.086	0.4	0.111	1.350	268.322	268.272	67	269.64	1.088			
44	45		266.736	1050	N/A	25.0	0.7	0.013	271.006	270.723	2.639	2.285	2.516	1.1	0.311	1.400	268.272	268.011	N/A	N/A	N/A			
45	46		266.462	1050	N/A	32.0	0.7	100000000000000000000000000000000000000	270.723		2.639	2.285	2.506	1.1	0.275	1.400	268.011	267.743	B145	269.15	0.909			
46	47	266.412	265.452	1050	N/A	120.0	0.8	0.013	270.449	269.754	2.821	2.442	2.662	1.1	0.281	1.400	267.743	266.647	77	268.30	0.327			
47	37	265.402	265.050	1050	N/A	44.0	0.8	0.013	269.754	269.459	2.821	2.442	2.783	1.1	0.195	1.500	266.647	266.221	81	267.95	1.073			
48	49	264.712	264.007	1200	N/A	70.5	1.0	0.013	269.299	268.030	3.447	3.899	3.367	0.9	0.061	1.483	265.973	265.417	87	266.71	0.507			
49	50	264.007	263.887	1200	N/A	12.0	1.0	0.013	268.030	268.145	3.447	3.899	3.384	0.9	0.210	1.483	265.417	265.154	N/A	N/A	N/A			
50	51	263.837	262.437	1200	N/A	87.5	1.6	0.013	268.145	266.532	4.360	4.932	3.513	0.7	0.117	1.483	265.154	264.486	B94N	265.45	0.066			
																			B94N	265.45	0.611	87.4	16.1	264.60
																			B93S	265.60	0.642	87.4	31.7	264.7
																			B93N	266.29	1.148	87.4	55.7	264.9
																			B125S	266.32	1.061	87.4	71.1	265.0
																			B129	266.55	1.471	87.4	47.5	264.8
																			1	266.22	1.262	87.4	31.7	264.7
54 I	0.5		000 044	4050	L NI/A					000 110	0.400				0.040	4 400	004.400	001005	2	265.61	0.776	87.4	15.4	264.6
51	65	262.287	262.011	1350	N/A	39.5	0.7			266.418	3.120	4.466	3.830	0.9	0.849	1.483	264.486	264.035	B94S	265.25	0.534			
52 53	53 54	267.517	267.124	825 825	N/A N/A	65.5 46.5	0.6		271.337 271.276		2.080	1.112	1.118	1.0	0.931	1.350	269.273	268.655	N/A 42	N/A 260 50	N/A 0.705			
54	56		CANCEL OF CHARLES	900	N/A	45.0	0.6	0.013	The second second	270.994	2.381	1.112 1.515	1.121 1.192	1.0 0.8	0.786 0.784	1.450 1.450	268.655 268.374	268.374 267.973	40	269.59 269.40	0.705 0.796			
55	56	267.020		825	N/A	78.5	0.7	0.013	and the second	270.726	2.247	1.201	1.192	1.1	0.764	1.350	268.510	267.973	51	269.40	0.796			
56	57	266.295	530-61-531-61-531-6-661	975	N/A	110.5	1.3	and the second	270.726		3.422	2.555	2.563	1.0	0.703	1.500	267.973	266.558	34	268.07	-0.133			
		200.200	254.000		- 111.1	. 10.0	1.0	0.010	2.0.120	200.202	J.722	2.000	2.000	1.0	5.700	,,,,,,,,	201.010	200.000	34	268.07	1.264	110.5	1.4	266.5
																			35	268.30	1.494	110.5	1.4	266.5
																			36	268.30	1.305	110.5	16.2	266.7
																			37	268.53	1.252	110.5	38.3	267.0
																			38	268.84	1.368	110.5	53.4	267.2
																			39	268.84	1.368	110.5	53.4	267.2
57	97	264.784	264.063	1050	N/A	51.5	1.4	0.013	269.282	268.695	3.731	3.231	2.770	0.9	0.724	1.533	266.558	266.046	31	267.57	0.782			
58	59	267.423		525	N/A	120.0	0.8		270.764		1.777	0.385	0.183	0.5	-0.268	1.367	267.680	266.985	20	268.32	0.410			
59	60	266.413		525	N/A	85.0	1.5		269.720		2.433	0.527	0.437	0.8	0.047	1.350	266.985	266.200	B131	267.30	0.085			
																			B131	267.30	0.853	84.8	1.8	266.2
																			B132	267.42	0.973	84.8	1.8	266.2
																			B133	267.70	1.052	84.8	23.6	266.4
																			B134	267.70	1.052	84.8	23.6	266.4
																			B135	267.89	1.031	84.8	46.3	266.6
																			B136	268.23	1.371	84.8	46.3	266.6

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

U/S	D/S	U/S	D/S	Pipe Dia.	Pipe	Pipe	Pipe	n	U/S MH	D/S MH	Design	Design	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	In	terpolated H	GL
МН	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			Length	Dist. From	HGL
									Elev.	Elev.	5		Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																	e e e		B137	268.29	1.225	84.8	68.6	266.835
																			B138	268.29	1.225	84.8	68.6	266.835
																			21	268.32	1.168	84.8	78.0	266.922
																			22	268.05	0.898	84.8	78.0	266.922
																			23	267.87	0.973	84.8	50.4	266.667
		-													-				24	267.81	0.913	84.8	50.4	266.667
60	62	A CONTRACTOR OF THE PARTY OF TH	264.538	675	N/A	15.0	3.0	0.013	DESCRIPTION OF THE PARTY OF THE	268.734	4.069	1.456	0.708	0.5	0.537	1.333	266.200	265.870	N/A	N/A	N/A			
61	62		265.000	300	N/A	43.5	2.1		268.976		1.982	0.140	0.126	0.9	0.372	1.350		265.870	B91	267.28	0.464			
62	63		264.038	675	N/A	30.0	1.4			268.239	2.779	0.995	0.819	0.8	0.737	1.417	265.870	265.579	7	267.01	0.910			
63	64	VI SAME AND ADDRESS OF THE PARTY OF THE PART	263.306	825	N/A	48.5	1.2	100000000000000000000000000000000000000		267.701	2.942	1.572	1.032	0.7	0.866	1.433	265.579	265.119	5	266.38	0.571			
64	65	The second second	262.161	1200	N/A	85.5	0.9	Sandamarca and	and the second second	266.418	3.270	3.699	4.312	1.2	0.988	1.400	265.119	264.035	N/A	N/A	N/A			
65	67		261.497	1500	N/A	40.5	0.9				3.795	6.706	8.137	1.2	0.674	1.433	264.035	263.479	N/A	N/A	N/A			
66	67		262.422	525	N/A	46.5	0.4	0.013		265.810	1.256	0.272	0.266	1.0	0.601	1.367	263.734	263.479	N/A	N/A	N/A			
67	78	The second secon	260.891	1500	N/A	50.5	1.1	0.013	The second secon	265.678	4.195	7.414	9.630	1.3	0.532	1.433	263.479	262.523	N/A	N/A	N/A			
68	69	FT-10-10-10-10-10-10-10-10-10-10-10-10-10-	259.281	1500	3000	133.0	1.0	0.013	The Court of the C	264.722	4.846	21.806	18.001	8.0	-0.072	1.467	262.039	261.147	N/A	N/A	N/A			
69	70		258.706	1500	3000	57.5	1.0	0.013		264.089	4.846	21.806	19.119	0.9	0.365	1.450	261.146	260.622	N/A	N/A	N/A			
70	70A		258.610	1500	3000	10.2	0.5	0.013		263.870	3.251	14.628	19.121	1.3	0.465	1.450	260.621	260.526	N/A	N/A	N/A			
73	74	LACOTTO SCHOOL ACTION SCHOOL	259.664	525	N/A	99.0	8.0	0.013		264.262	1.777	0.385	0.147	0.4	-0.272	1.483	260.709	260.421	N/A	N/A	N/A			
74	75		259.421	600	N/A	43.5	0.4			263.400	1.373	0.388	0.243	0.6	0.232	1.483	260.421	260.332	N/A	N/A	N/A			
75	80		258.253	1500	3000	57.5	0.5			261.870	3.251	14.628	20.073	1.4	0.320	1.433	260.332	260.261	N/A	N/A	N/A			
76	48	265.346		750	N/A	46.0	0.4	0.013	269.631	269.299	1.594	0.704	0.306	0.4	-0.069	1.517	266.027	265.973	N/A	N/A	N/A			
77	78	263.019		375	N/A	57.5	0.5			265.678	1.123	0.124	0.060	0.5	-0.168	1.300		262.909	N/A	N/A	N/A			
78	68		260.641	1500	N/A	20.0	1.1			265.468	4.195	7.414	9.669	1.3	0.162	1.433	262.523	262.141	N/A	N/A	N/A			
80	PondE2		258.117	1500	3000	21.5	0.4	151000000000000000000000000000000000000		260.700	3.065	13.792	20.088	1.5	0.558	1.433	260.261	260.253	N/A	N/A	N/A			
97	64		263.081	1050	N/A	68.0	1.4			267.701	3.731	3.231	3.116	1.0	0.963	1.533	266.046	265.119	29	267.35	1.074			
310	31		263.109	600	N/A	32.0	0.4	0.013		266.448	1.373	0.388	0.371	1.0	0.389	1.333	264.226	264.115	Part1	264.92	0.464			
500	50		264.737	300	N/A	36.0	1.0	0.013	268.339	268.145	1.368	0.097	0.005	0.1	-0.230	1.450	265.167	265.154	B125N	266.60	1.203			
660	66	Year All Control (Co.) No. Store	262.683	450	N/A	71.0	0.6	0.013	266.585	265.876	1.389	0.221	0.093	0.4	0.229	1.283	263.788	263.734	N/A	N/A	N/A			
ondE2	2811000,0010	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	4.559	N/A	N/A	1.950	N/A	N/A	N/A	N/A	N/A			
ondE2		N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	1.486	N/A	N/A	1.950	N/A	N/A	N/A	N/A	N/A			
O210	0211	256.300		1800	N/A	25.5	1.2			260.000	4.948	12.592	4.559	0.4	-0.919	1.950		256.759	N/A	N/A	N/A			
37	48	265.050	NAME OF TAXABLE PARTY.	1050	N/A	23.5	0.8	A sold Free Property	269.459	Section Section 1	200000000000000000000000000000000000000	2.442	2.803	1.1	0.121	2000	266.221	265.973	89	268.03	1.579			
72	80	259.346	NACES AND ADDRESS OF THE PARTY	750	N/A	28.0	0.5		X-10-00-00-00-00-00-00-00-00-00-00-00-00-	261.870	S.C.C.C.	0.787	-0.029	0.0	0.166	1.383	260.262	260.261	N/A	N/A	N/A			
0211	OUT		255.892	1800	N/A	8.5	1.2				100000000000000000000000000000000000000	12.592	4.559	0.4	-1.035	1.950		256.641	N/A	N/A	N/A			
C120	C166	264.490	The same of the same of	1050	N/A	19.3	0.4	and the second		The second second second	1.995	1.727	1.496	0.9	0.519	1.483	266.059	265.996	Cole Eng.	266.72	0.431			
C140 C150	C40 C59	265.810	CHARLES AND ASSESSMENT	675 450	N/A N/A	118.3	0.4	2000	269.620	CONTRACTOR OF SEC.	1.486	0.532	0.522	1.0	0.359	1.367	266.844	266.317	Cole Eng.	267.94	0.866			
C165	C40	266.750		450	N/A	94.9	1.0		270.310		1.793	0.285	0.191	0.7	-0.172	1.350	267.028	266.526	Cole Eng.	268.47	1.212			
C166		265.500		900 1050	N/A N/A	47.3	0.4		268.640		1.800	1.145	0.830	0.7	0.012	1.367	266.412	266.317	Cole Eng.	N/A	N/A			
C39	11 C140	264.370 267.940	000000000000000000000000000000000000000	450	N/A	28.9 105.2	1.0	4.0000000000000000000000000000000000000	268.110 270.660	TECHNOLOGICAL PROPERTY.	1.995 1.793	1.727 0.285	1.555 0.305	0.9 1.1	0.576 -0.041	1.483	265.996 268.349	265.890 267.274	Cole Eng. Cole Eng.	266.55 268.89	0.324 0.311			
C40	C62			1050	N/A	16.7								12.78					and the second second second					
C42	1	264.960		450	N/A	11.000 A. A.	0.4	300000000000000000000000000000000000000	PERSONAL SAME OF COLUMN	268.640	1.995	1.727	1.313	0.8	0.307	1.350	266.317	266.274	Cole Eng.	N/A	N/A			
C505	1	266.170 265.910		825	N/A	36.8 17.2	0.5	0.013		269.380 269.380	1.268 1.899	0.202 1.015	0.071	0.4 0.4	-0.228 -0.394	1.300	266.392 266.341	266.298 266.298	Cole Eng. Cole Eng.	N/A N/A	N/A N/A			
C506	C505	266.220	DODGE TO THE RESIDENCE OF	750	N/A	23.1	2022	100000000000000000000000000000000000000	19339012120012100000	269.380	000000000000000000000000000000000000000	1.113		725776	500000000000000000000000000000000000000	0.0	The second of the second of		-	15.0000				
C59	C60	265.730		450	N/A	9.7	1.0	0.013 0.013	269.310		2.520 1.793	0.285	0.436	0.4	-0.411 0.346	1.400	266.559 266.526	266.341 266.471	Cole Eng. Cole Eng.	N/A 267.63	N/A 0.874			
C60	9	265.550		450	N/A	37.7	1.0		269.170		555555555555	0.247		0.6	0.346	200000000000000000000000000000000000000	266.471	266.351	Cole Eng.	267.58	0.874			
C62	C63		264.610	1050	N/A	60.5	0.8		268.640		1.552 1.995	1.727	0.170 1.422	0.7	0.471	1.500	266.274	266.351	Cole Eng.	267.56	0.879			
C63	C120	264.580	Security or constitution of	1050	N/A	13.7	0.4	200000000000000000000000000000000000000	potentia concentrativo	meses and the second	120000000000	1.727	1.422	0.8	0.374	in the second	A CONTRACTOR OF THE PARTY.	266.059	Cole Eng.	266.91	0.706			
C75	7	267.270	The second second	300	N/A	32.8	0.4		270.390		1.995					1.483	266.101			268.88				
C75	C150	267.270	NAMES OF THE OWNER, WHEN	300	N/A	E-11(00)5000	1.0	31/03/45/07/190391	AND THE PROPERTY OF THE PARTY O	X102.020X.000000X	1.368	0.097	0.000	0.0	-0.458 -0.342	0.000	20.000000000000000000000000000000000000	267.112	Cole Eng.	1000007000000	1.538			
C40W	C42			450	N/A	12.7 57.8	1.0		270.390 268.640		1.368	0.097	0.000	0.0	-0.342	0.000	267.028	267.028	Cole Eng.	269.88	2.622			
F1	F3	266.490 268.818		450	N/A	57.8 76.5	0.5		272.312		1.268	0.202 0.180	0.073	0.4	-0.263 0.124	1.283	266.677 269.392	266.392 269.153	Cole Eng. 14	267.52 270.59	0.613 0.968			
1001	, ,	50.550 (0.000)	269.150	300	N/A	41.5	0.4 1.5			271.881	1.134 1.675	0.180	0.172 0.045	1.0 0.4	-0.171	1.367	269.392	269.153	N/A	N/A	0.966 N/A			

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

															- interim C			10.00						
U/S	D/S	U/S	D/S	Pipe Dia.	and the same	Pipe	Pipe	n	U/S MH	ema.	Design	Design	Peak	Peak /	and the second second	Time	Max.	Max.	Lot	BF	Freeboard		erpolated H	
МН	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S (1)	to	U/S	D/S	Number				Dist. From	HGL
			7.00	7	7	7	(0/)		Elev.	Elev.		(=3/=)	Flow	Flow		Peak	HGL	HGL				HGL	D/S MH	7.00
F3	F4	(m)	(m)	(mm) 750	(mm) N/A	(m)	(%)	0.040	(m)	(m)	(m/s)	(m³/s)	(m³/s)	0.0	(m)	(h)	(m)	(m)	0	(m)	(m)	(m)	(m)	(m)
F4	55		267.640	825	N/A	114.5	0.5	0.013		271.359	1.782	0.787	0.670	0.9	0.191	1.367	269.153	268.832	9	270.05	0.667			
F100	F3	267.565 268.440		600	N/A	99.0 19.5	0.5		271.359 271.710		1.899 1.373	1.015	0.931	0.9	0.442	1.350	268.832 269.275	268.510	3 N/A	269.67	0.608 N/A			
101	102	***************************************	262.663	525	N/A	52.5	0.4	0.013 0.013	Steel Stone State	265.856	1.539	0.388	0.349	0.9	0.235 -0.185	1.367 1.367	263.318	269.153 263.000	53	N/A 264.55	1.002			
102	103	262.438		750	N/A	72.0	0.0	0.013	265.856		2.391	1.056	0.407	0.7	-0.103	1.367	262.780	262.331	47	264.37	1.360			
103	106	261.760		750	N/A	76.5	1.2	0.013	265.495		2.760	1.220	0.865	0.7	-0.179	1.367	262.331	261.653	41	263.91	1.349			
Y34	106	260.150	Annual Control of the	1500	N/A	10.8	0.5	0.013		265.315	2.683	4.742	5.024	1.1	0.072	1.433	261.722	261.653	FUT	263.89	1.934			
105	106	262.342	and the same of th	450	N/A	120.0	1.0	Acres and the control of the	265.505	- Contraction Contraction	1.793	0.285	0.379	1.3	1.203	1.367	23.5-00.001.33.4026.00000055	261.653	29	263.74	-0.485			
					2000000														29	263.74	1.328	120.0	27.1	262.182
																			28	263.96	1.294	120.0	40.1	262.436
																			27	264.06	1.394	120.0	40.1	262.436
																			26	264.22	1.125	120.0	62.1	262.865
																			25	264.22	1.125	120.0	62.1	262.865
106	107	260.092	259.808	1500	N/A	35.5	0.8	0.013	265.315	265.195	3.578	6.323	6.210	1.0	0.061	1.433	261.653	261.335	N/A	N/A	N/A			
107	108	259.758	259.486	1500	N/A	34.0	0.8	0.013	265.195	265.182	3.578	6.323	6.533	1.0	0.077	1.433	261.335	260.993	70	263.62	2.055			
108	115	259.436	258.824	1500	N/A	76.5	0.8	0.013	265.182	264.761	3.578	6.323	6.685	1.1	0.057	1.433	260.993	260.312	B108S	263.21	1.987			
100	36	261.679	261.398	1500	N/A	53.0	0.5	0.013	266.641	266.363	2.912	5.146	7.834	1.5	0.064	1.433	263.243	262.572	B122S	264.99	1.517			
110	112	262.130	261.790	525	N/A	42.5	0.8	0.013	265.608	265.342	1.777	0.385	0.285	0.7	0.526	1.417	263.181	262.997	67	263.98	0.569			
112	113	261.715	261.445	600	N/A	54.0	0.5	0.013	265.342	265.038	1.536	0.434	0.354	0.8	0.682	1.400	262.997	262.773	B104S	263.55	0.323			
113	114	261.365	261.203	600	N/A	27.0	0.6	0.013	265.038	264.865	1.682	0.476	0.451	0.9	0.808	1.400	262.773	262.610	84	263.21	0.207			
																			84	263.21	0.360	27.1	1.6	262.620
																			85	263.29	0.357	27.1	15.4	262.703
																			86	263.44	0.445	27.1	25.7	262.765
114	115	261.153	260.754	600	N/A	57.0	0.7	0.013	264.865	264.761	1.817	0.514	0.762	1.5	0.857	1.367	262.610	261.302	81	263.13	0.290			
																			81	263.13	1.209	57.2	17.0	261.691
																			82	263.43	1.166	57.2	32.0	262.034
	440			4500	0.400														83	263.43	0.832	57.2	46.6	262.368
115	116	Andrew Control of the Control	258.590	1500	2400	68.0	0.3	0.013	Section Contract Cont	264.341	2.516	9.059	7.703	0.9	0.018	1.433		260.274	B106S	263.53	2.988			
116	125	258.590		1500	2400	21.0	0.3			264.463	2.516	9.059	7.696	0.8	0.184	1.433		260.268	N/A	N/A	N/A			
117	118 119		260.972	450 450	N/A	117.0	0.6	0.013		264.950	1.389	0.221	0.095	0.4	-0.232	1.350		261.521	B109S	263.40	1.278			
118 119	125		260.702 259.452	525	N/A N/A	31.5	0.7		264.950 265.137		1.500 2.176	0.239 0.471	0.263	1.1 0.8	0.149	1.350 1.350		261.062	N/A	N/A	N/A			
120	121	Date Service Statement	Section Contractions	375	N/A	97.5	0.0	***************************************				20000000000	0.356	555,000	-0.182			Service Service Address of the	16	263.24	2.045			
121	123	261.339 260.771		450	N/A	61.0	0.8		265.068		1.420	0.157	0.064	0.4	-0.203	1.367	261.511	261.140	7	263.08	1.339			
123	124	260.289	260.339	450	N/A	48.0 25.5	0.9	0.013	MANAGEMENT OF THE STREET	264.403 264.351	1.701 1.880	0.270 0.299	0.152 0.378	0.6 1.3	-0.081 0.283	1.400		261.022 260.422	3 1	262.83 262.64	1.460 1.388			
124	125	259.929		450	N/A	17.5	2.3	0.013	and the second second	264.463	2.719	0.432	0.378	0.9	-0.081	1.367	THE RESERVE OF THE PARTY OF THE	260.268	N/A	N/A	N/A			
125	126	258.477		1500	2400	37.5	0.3	C. C	100 miles	264.186	2.516	9.059	8.409	0.9	0.291	1.433		260.262	80	262.79	2.292			
126	129	258.371	San	1500	2400	15.5	0.3	CANCELLOS CON TAN	C-CONZIL DI CONCINC	264.080	2.516	9.059	8.406	0.9	0.291	1.450		260.262	77	262.79	2.338			
127	128	260.302		450	N/A	88.5	0.9		Mark Committee Committee Committee	264.067	1.701	0.270	0.380	1.4	1.238	1.367	A CONTRACTOR OF THE PARTY OF TH	260.427	72	262.58	0.360			
128	129	259.456	500000 CONTRACTOR CONT	450	N/A	13.5	0.6	0/10/10/10/10/10	0.000.000.000.000.000.000	264.080	1.389	0.221	0.380	1.7	0.521	1.367	A CONTRACTOR OF THE PARTY OF TH	260.260	N/A	N/A	N/A			
129	180	258.325		1500	2400	35.0	0.3			261.080	2.516	9.059	8.773	1.0	0.435	1.450		260.254	N/A	N/A	N/A			
180	PondE2	258.135		1500	2400	11.5	0.3	XXXXXXXXXX	261.080		2.516	9.059	8.752	1.0	0.619	1.450	Section Contracts	260.253	N/A	N/A	N/A			
1M	110	263.050		450	N/A	120.0	0.7		266.242		1.500	0.239	0.141	0.6	-0.050	1.467		263.181	64	264.32	0.640			
1M	2M	263.004	DOMESTIC DESCRIPTION OF THE	525	N/A	88.0	0.4	107000000000000000000000000000000000000	266.242	200000000000000000000000000000000000000	1.256	0.272	0.190	0.7	-0.079	1.367	10000000000000000000000000000000000000	263.349	Part29	264.67	0.990			
2M	100	262.524		675	N/A	56.0	0.4	100000000000000000000000000000000000000		266.641	1.486	0.532	0.329	0.6	0.150	1.367		263.243	Part47	264.78	1.201			
СМН5	310	263.450		450	N/A	12.5	0.5			266.131	1.268	0.202	0.138	0.7	0.405	1.433		264.226	N/A	N/A	N/A			
DICB1	101	263.361		375	N/A	15.5	1.5		265.800		1.944	0.215	0.076	0.4	-0.179	1.317		263.318	N/A	N/A	N/A			
T1	T2	promotoscororestoway.	269.547	375	N/A	50.5	0.8	100000000000000000000000000000000000000	272.822	200000000000000000000000000000000000000	1.420	0.157	0.026	0.2	-0.270	1.367	A COMMUNICATION ASSESSMENT	269.651	17	271.20	0.914			
T1	Т3	269.917	269.553	375	N/A	56.0	0.7		272.822		1.280	0.141	0.041	0.3	-0.236	1.367		269.691	41	271.14	0.854			
T2	F100	269.397	268.515	525	N/A	98.0	0.9		272.450		1.885	0.408	0.152	0.4	-0.298	1.367		269.275	25	270.49	0.636			
Т3	T4	269.403	268.993	525	N/A	63.0	0.7		272.515		1.602	0.347	0.041	0.1	-0.403	1.367	269.525	269.173	2	270.78	1.025			
T4	T5	268.843	268.247	675	N/A	149.0	0.4	0.013	272.160	271.777	1.486	0.532	0.248	0.5	-0.345	1.367	269.173	268.916	54	270.48	1.077			
T5	41	268.022	267.290	900	N/A	146.5	0.5	0.013	271.777	270.975	2.012	1.280	1.078	0.8	-0.006	1.367	268.916	268.502	Part7	269.70	0.554			

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

U/S	N-2: Pip D/S	e Data a	na Hyai	Pipe Dia.						D/S MH	-		_	Peak /	Interim C			Mov	Lot	DE.	Erochessel	l	tornolated U	ICI
MH	MH	Invert	Invert	/ Height	and the same	Pipe Length	Pipe	n	Cover	Cover	Vel.	Flow	Peak Pipe	Design	Surcharge U/S	Time	Max. U/S	Max. D/S	Lot Number	BF	Freeboard		terpolated H Dist. From	
IVIII	IVII I	iliveit	iliveit	/ Height	vviditi	Lengui	Slope		Elev.	Elev.	VGI.	1 low	Flow	Flow	(1)	Peak	HGL	HGL	Number			HGL	D/S MH	HOL
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)	10.00000	(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
Т6	T7		269.644	300	N/A	13.0	1.0	0.013	272.613		1.368	0.097	0.079	0.8	0.006	1.367	270.080	270.003	72	271.49	1.180			
Т6	T9	269.661	269.269	525	N/A	87.0	0.5	0.013	272.613	272.520	1.333	0.288	0.202	0.7	-0.106	1.367	270.080	269.951	74	271.18	0.870			
T7	T8	269.494	268.474	450	N/A	120.0	0.9	0.013	272.453	271.766	1.653	0.263	0.187	0.7	0.059	1.367	270.003	269.594	B128	270.36	0.127			
																			B128	270.36	0.531	120.2	1.4	269.599
																			63	270.47	0.641	120.2	1.4	269.599
																			64	270.55	0.629	120.2	28.4	269.691
																			65	270.55	0.629	120.2	28.4	269.691
																			66	270.65	0.640	120.2	54.6	269.780
																			67	270.70	0.690	120.2	54.6	269.780
																			68 69	270.70 270.84	0.604 0.744	120.2 120.2	80.0 80.0	269.866 269.866
																			70	270.84	0.654	120.2	106.3	269.956
																			71	270.89	0.704	120.2	106.3	269.956
																			98	271.09	0.926	120.2	99.8	269.934
																			99	271.09	0.926	120.2	99.8	269.934
																			100	270.99	0.916	120.2	73.6	269.844
																			101	270.82	0.746	120.2	73.6	269.844
																			102	270.72	0.735	120.2	47.4	269.755
																			103	270.64	0.655	120.2	47.4	269.755
																			104	270.58	0.683	120.2	21.5	269.667
																			105	270.40	0.503	120.2	21.5	269.667
Т8	T12	268.244	267.881	600	N/A	33.0	1.1	0.013	271.766	271.539	2.278	0.644	0.395	0.6	0.750	1.333	269.594	269.387	Part4	270.20	0.376			
Т9	T10	269.194	269.150	600	N/A	14.5	0.3	0.013	272.520	272.353	1.189	0.336	0.238	0.7	0.157	1.367	269.951	269.927	82	271.14	0.959			
T10	T11	269.100	268.125	600	N/A	97.5	1.0	0.013	272.353	271.833	2.172	0.614	0.415	0.7	0.227	1.367	269.927	269.521	B133	270.20	0.043			
																			B133	270.20	0.444	97.3	1.3	269.526
																			90	270.25	0.432	97.3	16.1	269.588
																			89	270.31	0.492	97.3	16.1	269.588
																			88	270.37	0.445	97.3	41.8	269.695
																			87	270.44	0.515	97.3	41.8	269.695
																			86	270.50	0.465	97.3	68.0	269.805
																			85	270.57	0.535	97.3	68.0	269.805
																			84 83	270.63 270.63	0.486 0.486	97.3 97.3	94.2 94.2	269.914 269.914
																			97	271.13	1.023	97.3	94.2 85.2	269.877
																			96	271.13	1.023	97.3	85.2	269.877
																			95	270.76	0.762	97.3	59.1	269.768
																			94	270.76	0.762	97.3	59.1	269.768
																			93	270.59	0.702	97.3	32.9	269.658
																			92	270.59	0.702	97.3	32.9	269.658
																			91	270.40	0.576	97.3	17.4	269.594
T11	T12	267.967	267.811	750	N/A	39.0	0.4	0.013	271.833	271.539	1.594	0.704	0.510	0.7	0.804	1.483	269.521	269.387	Part3	270.15	0.399			
T12	52		267.595	750	N/A	19.5	0.7		271.539		2.108	0.931	0.853	0.9	0.906	1.483	269.387	269.273	N/A	N/A	N/A			
T13	T14	269.222	268.800	600	N/A	105.5	0.4	0.013	272.254	272.129	1.373	0.388	0.227	0.6	-0.263	1.367	269.559	269.385	B123W	270.50	0.711			
T14	T15	268.750	268.442	600	N/A	44.0	0.7	0.013	272.129	271.890	1.817	0.514	0.333	0.6	0.035	1.350	269.385	269.278	Part14	270.22	0.605			
T15	T21	268.367	268.121	675	N/A	41.0	0.6	00.000.000.000	271.890		1.820	0.651	0.494	8.0	0.236	1.367	269.278	269.155	Part15W	270.06	0.552			
T16	T17		269.251	375	N/A	74.0	0.4		272.395		1.004	0.111	0.040	0.4	-0.215	1.300	269.707	269.406	112	271.11	1.173			
T17	T18		268.884	450	N/A	58.5	0.5	V. 50 CT (CO. CO. CT CT CO. CT CT CO. CT CT CO. CT CT CO. CT CT CT CO. CT	272.209		1.268	0.202	0.097	0.5	-0.220	1.350	269.406	269.294	Part41	270.50	0.864			
T18	T19		268.738	450	N/A	12.0	0.8		271.888		1.603	0.255	0.102	0.4	0.010	1.483	269.294	269.280	Part42	270.30	0.776			
T19	T20		NO ME CONTRACTOR OF THE CONTRA	525	N/A	36.0	0.6		271.741		1.473	0.319	0.141	0.4	0.092	1.483		269.255	Part46	270.21	0.700			
T20	T21	268.265		675	N/A	24.0	0.6		271.527		1.820	0.651	0.351	0.5	0.315	1.317		269.155	Part22	270.46	0.975			
T21	39		267.755	825	N/A	36.0	0.6	1000 C 10	271.662		2.080	1.112	0.819	0.7	0.359	1.500	269.155	269.036	Part16W	269.91	0.525			
CMH3	4		264.635	375	N/A	11.5	0.5		268.180		1.123	0.124	0.048	0.4	-0.044	1.433		265.006	N/A	N/A	N/A			
Y1	Y2	265.512	265.222	300	N/A	29.0	1.0	0.013	268.294	268.509	1.368	0.097	0.098	1.0	-0.047	1.283	265.765	265.465	69	266.80	0.805			

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

											-				- interim C									
U/S	D/S	U/S	D/S	Pipe Dia.	0.00	Pipe	Pipe	n	U/S MH	D/S MH	Design	Design	Peak	Peak /		Time	Max.	Max.	Lot	BF	Freeboard	11000	erpolated H	0.000
МН	МН	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S (1)	to	U/S	D/S	Number				Dist. From	HGL
		(tear)	(\)	(/\	(X	(0/)		Elev.	Elev.	(t>	(m ³ /a)	Flow	Flow		Peak	HGL	HGL		(m)	7	HGL	D/S MH	()
Y2	Y3	(m) 265.172	(m) 264.022	(mm) 300	(mm) N/A	(m) 60.5	(%) 1.9	0.013	(m) 268.509	(m) 268.054	(m/s) 1.886	(m ³ /s) 0.133	(m ³ /s) 0.142	1.1	(m) -0.025	(h) 1.317	(m) 265.447	(m) 264.294	41	(m) 266.52	(m) 0.843	(m)	(m)	(m)
Y3	Y4		263.410	600	N/A	44.5	0.7	0.013	268.054	267.689	1.817	0.133	0.142	0.7	-0.025	1.350	264.183	264.294	37	266.12	1.707			
Y4	Y7			1200	N/A	65.0	0.7	0.013		267.220	2.180	2.466	2.296	0.7	0.037	1.400	264.047	263.385	33	265.85	1.573			
Y5	Y6	Digital Color State Color	264.230	675	N/A	75.5	0.5	0.013	Notice of the contract of	267.827	1.661	0.594	0.590	1.0	0.039	1.383	265.322	264.972	26	266.26	0.708			
Y6	Y7	264.155		750	N/A	82.5	0.5	0.013	II ASSESSMENT AND A PARTY	267.220	1.782	0.787	0.776	1.0	0.067	1.383	264.972	264.289	32	265.75	0.548			
Y7	Y25	NO WARRY WAS A THURST	261.034	1350	N/A	83.2	0.4	0.013	267.220	266.477	2.358	3.376	3.628	1.1	0.011	1.383	262.728	262.352	N/A	N/A	N/A			
Y21	Y22	263.484		375	N/A	63.8	1.0	0.013	267.204	266.726	1.587	0.175	0.153	0.9	-0.097	1.350	263.762	263.165	63	265.38	1.388			
Y22	Y25	262.771	261.882	450	N/A	70.9	1.3	0.013	266.726	266.477	2.004	0.319	0.275	0.9	-0.056	1.367	263.165	262.352	59	265.11	1.715			
Y23	Y24	263.512	263.174	375	N/A	33.8	1.0	0.013	267.341	267.201	1.587	0.175	0.065	0.4	-0.208	1.367	263.679	263.465	49	265.94	2.031			
Y24	Y25	263.024	261.800	525	N/A	116.5	1.1	0.013	267.201	266.477	2.036	0.441	0.382	0.9	-0.084	1.367	263.465	262.352	56	265.14	1.445			
Y25	Y33	260.862	260.558	1500	N/A	76.0	0.4	0.013	266.477	265.820	2.530	4.471	4.303	1.0	-0.010	1.383	262.352	262.085	FUT	264.91	2.325			
Y26	Y27	263.562	262.843	375	N/A	71.9	1.0	0.013	267.410	266.435	1.587	0.175	0.085	0.5	-0.172	1.367	263.765	263.028	FUT	265.84	1.845			
Y27	Y28	262.693	262.277	525	N/A	83.3	0.5	0.013	266.435	265.950	1.405	0.304	0.163	0.5	-0.229	1.367	262.989	262.785	FUT	264.87	1.646			
Y28	Y33	262.247	261.503	525	N/A	74.4	1.0	0.013	265.950	265.820	1.987	0.430	0.389	0.9	0.013	1.383	262.785	262.085	FUT	264.38	1.365			
Y29	Y30		263.726	300	N/A	51.0	1.1	0.013	267.240	266.842	1.435	0.101	0.048	0.5	-0.150	1.367	264.437	263.871	FUT	265.67	1.003			
Y30	Y31	100000000000000000000000000000000000000	263.100	375	N/A	11.2	0.7	0.013	266.842	266.715	1.328	0.147	0.073	0.5	-0.169	1.383	263.387	263.285	FUT	265.27	1.655			
Y31	Y32		262.259	525	N/A	72.2	0.5	0.013		266.108	1.405	0.304	0.125	0.4	-0.287	1.367	262.859	262.571	FUT	265.15	2.056			
Y32	Y33			525	N/A	71.2	1.0	0.013	The state of the s	265.820	1.987	0.430	0.283	0.7	-0.169	1.367	262.571	262.085	FUT	264.54	1.737			
Y33	Y34		260.208	1500	N/A	71.2	0.5	0.013		265.456	2.683	4.742	5.036	1.1	0.057	1.383	262.085	261.722	FUT	264.25	1.935			
T101	T102	269.597	269.263	450	N/A	83.5	0.4	0.013		272.327	1.134	0.180	0.116	0.6	-0.176	1.367	269.871	269.563	B7S	270.63	0.529			
T102	T104	269.213	269.177	450	N/A	9.0	0.4	0.013	272.327	272.440	1.134	0.180	0.113	0.6	-0.100	1.383	269.563	269.478	B5N	270.85	1.057			
T103 T104	T104 T105	\$1835000000000000000000000000000000000000	269.252	375 675	N/A	45.0	0.8	0.013		272.440	1.420	0.157	0.144	0.9	-0.088	1.367	269.899	269.533	B4E	270.61	0.481			
T104	T106	268.952 268.586	268.636	675	N/A N/A	79.0	0.4	0.013 0.013		272.063 271.980	1.486	0.532 0.532	0.400	0.8	-0.149 0.036	1.367	269.478 269.297	269.297	B2W B1N	270.60 270.31	0.892 0.783			
T106	T107	268.504	268.554 268.360	675	N/A	8.0 36.0	0.4	0.013	000000000000000000000000000000000000000	271.935	1.486 1.486	0.532	0.425 0.616	0.8 1.2	0.036	1.383	269.297	269.270 269.110	B1S	270.31	0.763			
T107	T5		268.248	675	N/A	12.5	0.5	0.013		271.777	1.661	0.594	0.614	1.0	0.125	1.367	269.110	268.916	N/A	N/A	N/A			
70A	75	258.610	258.512	1500	3000	21.8	0.5	0.013		263.400	3.251	14.628	19.558	1.3	0.416	1.433	260.526	260.332	N/A	N/A	N/A			
V1	V3	261.668	260.688	300	N/A	24.5	4.0	0.013		264.642	2.736	0.193	0.027	0.1	-0.219	1.350		261.387	BF27/28	263.31	1.331			
V2	V3	Yacate (Newscapes)	260.618	375	N/A	59.5	1.0	0.013	264.947	264.642	1.587	0.175	0.091	0.5	-0.064	1.333		261.387	BJ68/69	262.60	0.846			
V3	V5	260.538	260.282	450	N/A	36.5	0.7	0.013	264.642	264.402	1.500	0.239	0.144	0.6	0.399	1.417	261.387	261.296	BE22	262.65	1.033			
V4	V5	261.021	260.357	375	N/A	83.0	0.8	0.013	264.768	264.402	1.420	0.157	0.169	1.1	0.619	1.317	262.015	261.296	BG38/39	262.52	0.275			
		20 13																	BG38/39	262.52	0.844	83.1	17.3	261.446
																			BG40/41	262.52	0.733	83.1	30.2	261.557
																			BG42/43	262.52	0.621	83.1	43.1	261.669
																			BH49/50	262.60	0.566	83.1	58.7	261.804
																			BH51/52	262.60	0.455	83.1	71.6	261.915
																			BH53	262.60	0.381	83.1	80.1	261.989
																			BI54/55	262.93	0.908	83.1	57.3	261.792
																			BI56/57	262.93	1.020	83.1	44.4	261.680
																			BJ62/63	262.87	1.096	83.1	28.7	261.544
V5	\/0	260 207	260,000	525	N/A	20.5	0.7	0.042	264 400	264 420	1.660	0.260	0.007	0.0	0.504	1 202	264 206	261.460	BJ64/65	262.60	0.937	83.1	15.8	261.433
V6	V9 V7			525 300	N/A N/A	28.5	0.7			264.136		0.360	0.287	0.8	0.564	1.383		261.169	BE18/19	262.38	0.854			
V6 V7	V9	No. of the Control of	261.210	450	N/A N/A	9.0	1.0	107000000000000000	264.061	26.60.00 - 0.00.00.00.00.00	114 1000000000	0.097	0.019	0.2	-0.194	1.267	1-12-45-00- Table-New Y	261.347	N/A BC13/14	N/A	N/A 0.563			
V8	V9		260.038 260.183	300	N/A	107.0 19.5	0.5 1.1		264.167 264.162			0.202 0.101	0.128 0.085	0.6	0.324 0.689	1.283	261.347 261.387	261.169 261.169	BC13/14 BD15/16	262.14 262.36	0.563 0.743			
V9	V10	COLUMN CONTRACTOR CONT	259.508	525	N/A	22.5	2.0	68100,01500,0500	264.136	100000000000000000000000000000000000000	2.810	0.608	0.537	0.8	0.686	1.367	261.367	260.828	N/A	N/A	0.743 N/A			
V10	70A		259.080	600	N/A	17.0	2.0	0.013	Vallage Control of the Control of th	263.870	3.071	0.868	0.535	0.6	0.808	1.367	260.828	260.534	N/A	N/A	N/A			
Y101	Y102		266.768	375	N/A	65.5	2.0	V. S. C.	270.964	200000000000000000000000000000000000000	NAMES OF TAXABLE PARTY.	0.248	0.226	0.9	-0.045	1.350	STREET, STREET	267.066	32	268.65	0.012			
						30.0					,				2.0,0				32	268.65	0.858	65.7	24.3	267.562
																			31	268.70	0.908	65.7	24.3	267.562
																			30	268.91	0.609	65.7	49.2	268.071
																			29	269.11	0.809	65.7	49.2	268.071
																			28	269.33	0.719	65.7	64.4	268.381

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

								or the 1							Interim C					F				
U/S	D/S	U/S	D/S	Pipe Dia.		Pipe	Pipe	n	U/S MH	D/S MH	Design	1000		Peak /	Surcharge	4772	Max.	Max.	Lot	BF	Freeboard	3034V. (1984)	terpolated H	
МН	MH	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S (1)	to	U/S	D/S	Number			100000000000000000000000000000000000000	Dist. From	HGL
		()	(\	(/\	(\)	/0/ \		Elev.	Elev.	(l-)	(m ³ /s)	Flow (m ³ /s)	Flow		Peak	HGL	HGL		(100)	()	HGL	D/S MH	()
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(111 /5)	(111 /5)		(m)	(h)	(m)	(m)	27	(m) 269.55	(m) 0.939	(m) 65.7	(m) 64.4	(m) 268.381
Y102	Y103	266.618	265 648	525	N/A	97.0	1.0	0.013	269.863	268 810	1 987	0.430	0.442	1.0	-0.077	1.367	267.066	266.093	91	267.53	0.939	05.7	04.4	200.301
		200.010	200.040	525	14/74	57.0	1.0	0.010	200.000	200.010	1.507	0.400	0.442	1.0	-0.017	1.007	207.000	200.000	91	267.53	1.083	97.2	12.4	266.217
																			92	267.78	1.333	97.2	12.4	266.217
																			93	267.98	1.313	97.2	34.4	266.437
																			94	267.98	1.313	97.2	34.4	266.437
																			95	268.01	1.122	97.2	56.4	266.658
																			96	268.29	1.402	97.2	56.4	266.658
																			97	268.29	1.184	97.2	78.2	266.876
																			98	268.49	1.384	97.2	78.2	266.876
Y103	Y5		264.688	600	N/A	73.0	1.1	Company of the Company	268.810		2.278	100000000000000000000000000000000000000	0.536	8.0	-0.001	1.367	266.090	265.322	FUT	267.24	0.920			
Y110	Y111	266.309	265.877	600	N/A	108.0	0.4	0.013	269.419	270.586	1.373	0.388	0.399	1.0	-0.096	1.383	266.813	266.291	1	268.09	1.047			
																			1	268.09	1.060	107.9	105.3	266.800
																			2	268.09	1.060	107.9	105.3	266.800
																			3	268.10	1.144	107.9	90.0	266.726
																			4	268.42	1.464	107.9	90.0	266.726
																			5	268.77 268.77	1.920 1.920	107.9 107.9	68.0 68.0	266.620 266.620
																			7	268.93	2.186	107.9	46.0	266.514
																			8	269.15	2.406	107.9	46.0	266.514
																			9	269.37	2.733	107.9	24.0	266.407
																			10	269.37	2.733	107.9	24.0	266.407
Y111	Y112	265.652	265,184	825	N/A	117.0	0.4	0.013	270.586	272,103	1.698	0.908	0.633	0.7	-0.315	1.367	266.162	265.704	11	269.63	3.238	101.0	24.0	200.101
Y112	Y113	265.109		900	N/A	70.0	0.4		272.103		1.800	1.145	0.643	0.6	-0.305	1.383	265.704	265.344	26	269.89	3.956			
Y113	Y114		264.397	900	N/A	88.0	0.4	151000000000000000000000000000000000000			1.800	1.145	0.786	0.7	-0.305	1.383	265.344	264.980	B111S	268.07	2.496			
Y114	Y115	264.347	264.277	900	N/A	17.5	0.4	0.013	270.497	270.335	1.800	1.145	0.785	0.7	-0.267	1.400	264.980	264.874	B114S	268.06	2.850			
Y115	Y116	264.202	263.720	975	N/A	120.5	0.4	0.013	270.335	269.418	1.898	1.417	1.045	0.7	-0.303	1.400	264.874	264.479	B109W	267.25	2.146			
Y116	Y117	263.670	263.624	975	N/A	11.5	0.4	0.013	269.418	269.228	1.898	1.417	1.124	8.0	-0.166	1.417	264.479	264.341	B109E	267.25	2.541			
Y117	Y4		262.890	1200	N/A	76.0	0.4	0.013	269.228	267.689	2.180	2.466	1.729	0.7	-0.053	1.400	264.341	264.047	FUT	267.66	3.087			
Y1110	Y111	265.813		750	N/A	14.0	0.4	0.013	270.628		1.594	0.704	0.050	0.1	-0.397	1.667	266.166	266.162	N/A	N/A	N/A			
Z1	Z4		266.998	300	N/A	25.5	4.0		270.871				0.000	0.0	-0.300	0.000	268.018		BLL66	269.03	0.782			
Z2	Z6	266.800	266.542	375	N/A	64.5	0.4	0.013	269.671	269.651	1.004	0.111	0.091	8.0	-0.111	1.283	267.064	266.846	BEL20	267.75	0.456			
																			BEL20	267.75	0.634	64.4	11.8	266.886
																			BEL17	267.89	0.733	64.4	24.0	266.927
																			BDL16 BDL12	267.89 267.78	0.681 0.502	64.4 64.4	39.3 59.6	266.979
Z3	Z4	266.636	266 322	450	N/A	78.5	0.4	0.013	269.747	270 005	1 134	0.180	0.175	1.0	-0.021	1 300	267.065	266.640	BIL55	267.78	0.502	04.4	39.0	267.048
		200.000	200.022			, 0.0	5.4	0.010	200.141	2, 5.000	1.104	0.100	0.170	1.0	J.UZ 1	1.000	207.000	200.040	BIL55	267.87	0.921	78.5	14.5	266.719
																			BIL40	268.09	1.026	78.5	35.8	266.834
																			BJL50	268.09	0.938	78.5	52.1	266.922
																			BJL46	267.71	0.443	78.5	73.4	267.037
																			BCL11	267.81	0.532	78.5	75.3	267.048
																			BCL8	268.32	1.108	78.5	63.1	266.982
																			BBL7	268.32	1.195	78.5	47.1	266.895
																			BBL4	268.72	1.661	78.5	34.9	266.829
																			BAL3	268.72	1.737	78.5	20.9	266.753
		,																	BAL1	268.85	1.911	78.5	12.8	266.709
Z4	Z 7	266.242	100	450	N/A	35.0	0.4		270.005		1.134	0.180	0.185	1.0	-0.052	1.317	266.640	266.508	BKL60	267.83	0.960			
Z5	Z6	266.785	266.617	300	N/A	21.0	8.0	0.013	269.764	269.651	1.224	0.086	0.010	0.1	-0.229	1.433	266.856	266.846	BFL24	267.69	0.604			
																			BFL24	267.69	0.606	20.9	15.7	266.854
70	77	000.000	000 000	505	hire 1			0.5/-	000 = : 1	000 000		0.000	0.555				202.2	000	BFL21	267.75	0.672	20.9	3.5	266.848
Z6	Z7	266.392	266.032	525	N/A	90.0	0.4	0.013	269.651	269.236	1.256	0.272	0.236	0.9	-0.071	1.317	266.846	266.508	BHL35	267.28	0.204	00.0	40.1	000 505
																			BHL35	267.28	0.481	89.9	16.1	266.569

Table A-2: Pipe Data and Hydraulic Simulation Results for the 100-Year, 4-Hour Chicago Storm (Pond E2 - Interim Conditions)

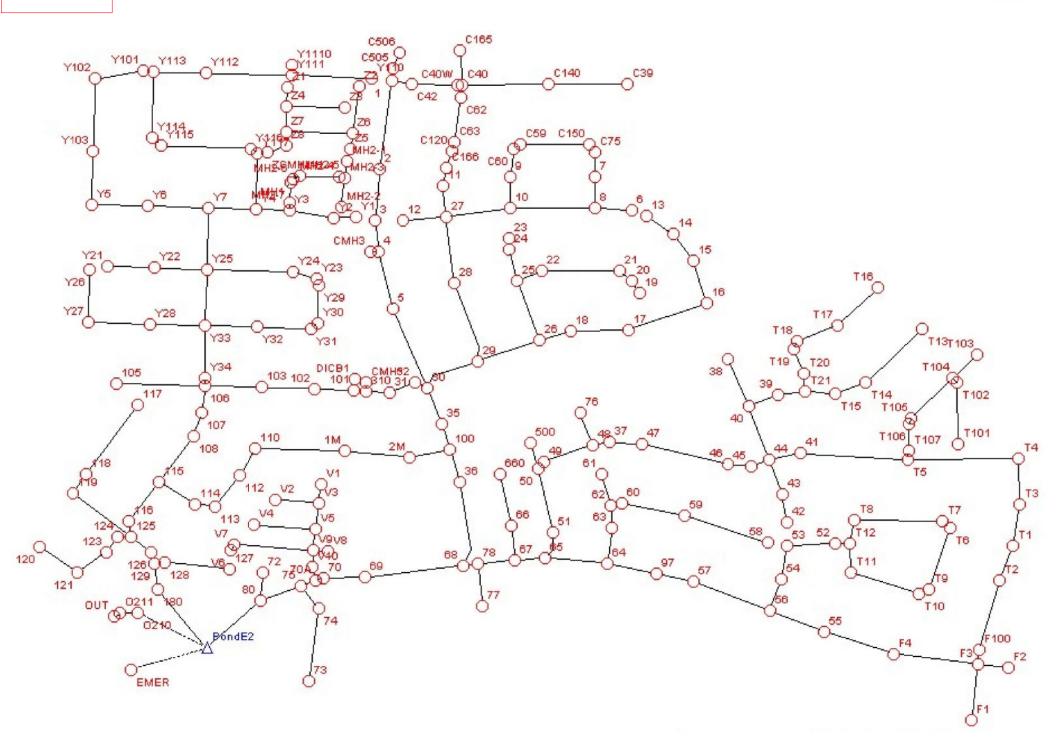
1116																	and the second	100000000			1	140		
U/S	D/S	U/S	D/S	Pipe Dia.	and the same	Pipe	Pipe	n	U/S MH	D/S MH	Design	Design	Peak	Peak /	Surcharge	Time	Max.	Max.	Lot	BF	Freeboard	In	terpolated H	GL
MH	MH	Invert	Invert	/ Height	Width	Length	Slope		Cover	Cover	Vel.	Flow	Pipe	Design	U/S	to	U/S	D/S	Number			Length	Dist. From	HGL
									Elev.	Elev.			Flow	Flow	(1)	Peak	HGL	HGL				HGL	D/S MH	
		(m)	(m)	(mm)	(mm)	(m)	(%)		(m)	(m)	(m/s)	(m ³ /s)	(m ³ /s)		(m)	(h)	(m)	(m)		(m)	(m)	(m)	(m)	(m)
																			BHL30	267.42	0.530	89.9	40.3	266.660
																			BGL29	267.45	0.511	89.9	53.5	266.709
																			BGL25	267.59	0.575	89.9	73.6	266.785
																			BJL45	267.71	0.698	89.9	73.0	266.782
																			BJL41	267.78	0.848	89.9	51.7	266.702
																			BIL40	267.78	0.909	89.9	35.4	266.641
																			BIL36	267.64	0.849	89.9	14.1	266.561
Z7	Z8	265 952	265.841	600	N/A	18.5	0.6	0.013	269 236	268.839	1.682	0.476	0.421	0.9	-0.044	1.333	266.508	266.266	BKL56	267.13	0.392	00.0	1.1.1	
		200.002	200.041		1 117 1	10.0	0.0	0.010	200.200	200.000	1.002	0.470	0.421	0.5	-0.044	1.000	200.000	200.200	BKL56	267.13	0.444	18.7	14.7	266.456
Z8	ZCMH1	265.761	265.443	600	N/A	26.5	10	0.042	268.839	268.463	2.379	0.673	0.555	0.8	0.474	4 252	266.190	265.859	N/A	N/A	N/A	10.7	14.7	200,430
	200 CO.	The state of the s		Marie Control of the			1.2	0.013		100	2 2 2	100000000000000000000000000000000000000	0.555		-0.171	1.350								
ZCMH1	Y117	263.915	WALK STATE OF THE PARTY OF THE	675	N/A	13.0	0.5	0.013	CONTRACTOR OF THE PARTY OF THE	BECKEL STATE OF SECTION OF SECTIO	1.661	0.594	0.555	0.9	0.017	1.350	STATISTICS OF STATISTICS	264.341	N/A	N/A	N/A			
MH2-1		265.117	264.955	300	N/A	22.2	1.0	0.013	269.280	269.080	1.368	0.097	-0.004	0.0	-0.248	1.267	265.169	265.167	FUT	267.71	2.311			
MH2-2	MH2-3	265.347	264.955	300	N/A	39.3	1.0	0.013	268.540	269.080	1.368	0.097	0.133	1.4	0.545	1.283	266.192	265.228	FUT	266.97	0.548			
MH2-3	MH2-4	264.875	264.840	375	N/A	7.1	0.5	0.013	269.080	268.960	1.123	0.124	0.133	1.1	-0.083	1.283	265.167	265.109	FUT	267.51	2.113			
MH2-4	MH2-5	264.765	264.499	450	N/A	53.1	0.5	0.013	268.960	268.550	1.268	0.202	0.157	0.8	-0.145	1.350	265.070	264.810	FUT	267.39	2.090			
MH2-5	MH2-6	264.449	264.397	450	N/A	10.4	0.5	0.013	268.550	268.460	1.268	0.202	0.186	0.9	-0.089	1.350	264.810	264.732	FUT	266.98	1.940			
MH2-6	MH2-7	264.347	264.320	450	N/A	5.4	0.5	0.013	268.460	268.420	1.268	0.202	0.198	1.0	-0.065	1.367	264.732	264.672	FUT	266.89	1.928			
MH2-7	MH1	264.270	264.130	450	N/A	28.1	0.5	0.013	No. of the Control of		1.268	0.202	0.234	1.2	-0.048	1.350		264.471	FUT	266.85	1.948			
MH1	Y3		263.779	600	N/A	10.3	0.5	0.013		268.054			0.234	0.5	-0.142		264.288		FUT	266.61	2.092			

Note: (1) A negative surcharge implies that the pipe is not flowing full

266.886 Interpolated HGL elevation

A-1: XPSWMM MODEL SCHEMATIC





TOWN OF CALEDON PLANNING



ATTACHMENT

Calculation Sheets





Water Resources and **Environmental Consultants**



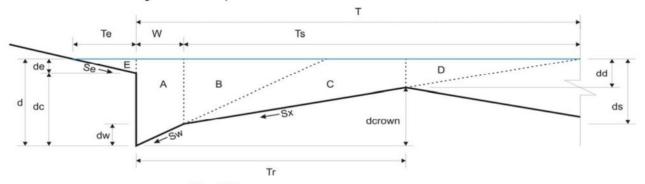
TOWN OF CALEDON

PLANNING RECE Palculation Sheet B-1: Flow Depth and Spread at Location with Highest Peak Flow on a Typical Street

Sub-catchme	-t/-\	MOZENE and MOZECE	I Winsels Conditions
	ent(s)	M075NE and M075SE	
Location		Dougall Avenue	12.5 m wide road
Q _{M075NE}		2.593	
Q _{M075SE}		0.764	
Q _{combined} (2)	1	81 62/4	for 100-Year Storm
Tr	(m)	6.25	
So	(m/m)	0.005	
W	(m)	0.000	
Sw	(m/m)	0.000	
Т	(m)	8.257	
Sx	(m/m)	0.03	
n _{road}		0.013	
dc	(m)	0.15	
Se	(m/m)	0.03	
n _{shoulder}		0.025	
dw	(m)	0.000	
Ts	(m)	8.257	
ds	(m)	0.248	
d	(m)	0.248	
d _{crown}	(m)	0.187	
dd	(m)	0.060	dd < 0.15 m, the max. depth over road crown of an arterial road
de	(m)	0.098	
Те	(m)	3.257	Flow is contained within ROW
Q _{area(A+B)}	(m ³ /s)	0.000	
Q _{area(B)}	(m ³ /s)	0.000	
Q _{area(A)}	(m ³ /s)	0.000	
Q _{area(B+C+D)}	(m ³ /s)	1.645	
Q _{area(D)}	(m ³ /s)	0.038	
Q _{area(B+C)}	(m^3/s)	1.607	
Q _{area(E)}	(m^3/s)	0.072	
Q _{area(A+B+C+E)}	(m^3/s)	1.678	
Q _{two sides}	(m ³ /s)	3.357	
d _{Flow} (3)	(m)	Very amplified out	d _{flow} < 0.30 m, the maximum allowable depth of flow
A _{flow two sides}	(m ²)	2.243	
V	(m/s)	1.497	
v×d	(m²/s)		$v \times d < 0.65 \text{ m}^2/\text{s}$

Notes:

- (1) 100-year flow from DDSWMM model (Chicago storm).
- (2) The computations assume that the total incoming flow is equally divided on both sides on the road.
- (3) Computations based on methodology described in MTO Drainage Management Manual, 1997, Ch.4, pp. 59-60. So is the longitudinal road slope



Equations:
$$Q_{area(A+B)} = 0.375 \times So^{0.5} \times d^{2.667} / (n_{road} \times Sw)$$

$$Q_{area(B)} = 0.375 \times So^{0.5} \times (ds)^{2.667} / (n_{road} \times Sw)$$

$$Q_{area(B+C+D)} = 0.375 \times So^{0.5} \times (ds)^{2.667} / (n_{road} \times Sx)$$

$$Q_{area(D)} = 0.375 \times So^{0.5} \times (dd)^{2.667} / (n_{road} \times Sx)$$

$$Q_{area(E)} = 0.375 \times So^{0.5} \times (de)^{2.667} / (n_{shoulder} \times Se)$$

CALCULATION SHEET B-2A: POND E2 EAST OVERLAND FLOW ROUTE

EAST OVERLAND FLOW ROUTE AT DOUGALL AVENUE (NORTH SIDE OF POND) - CURB CUT WEIR

2.960 m³/s for 100-yr event (on MAJ road segment) Approaching flow = Curb cut width = 15.5 m as per DSEL grading plan Curb cut height = 0.050 m as per DSEL Maximum flow depth at gutter = 0.293 m $(0.15 \text{ m} + 0.03 \times 4.75 \text{ m} = 0.293 \text{ m} \text{ for flow contained within RW})$ $(0.15 \text{ m} + 0.03 \times 4.75 \text{ m} - 0.050 \text{ m} = 0.243 \text{ m})$ Average head of water over curb cut = 0.243 m Curb cut weir coefficient = 1.84 3.406 m³/s for 100-yr event Maximum flow through cub cut =

Therefore the capacity of the curb cut (3.406 m³/s) is higher than the computed overland flow (2.960 m³/s)

EAST OVERLAND FLOW ROUTE DOWNSTREAM OF CURB CUT (NORTH SIDE OF POND)

$Q = 1/n \times AR^{2/3}S^{1/2}$			
	Min. Slope		Max. Slope
normal depth =	0.224	m	0.074
n =	0.03		0.03
Channel width =	15.5	m	15.5
A (area of flow) =	3.471	m^2	1.144
wetted perimeter =	15.948	m	15.648
R (hydraulic radius) =	0.218	m	0.073
S (slope) =	0.005		0.197
Q (flow) =	2.960	m^3/s	2.960
velocity =	0.85	m/s	2.59

CALCULATION SHEET B-2B: POND E2 WEST OVERLAND FLOW ROUTE

WEST OVERLAND FLOW ROUTE AT DOUGALL AVENUE (NORTH SIDE OF POND) - CURB CUT WEIR

1.805 m³/s for 100-yr event (on MAJ road segment) Approaching flow = Curb cut width = 15.5 m as per DSEL grading plan Curb cut height = 0.050 m as per DSEL Maximum flow depth at gutter = 0.293 m $(0.15 \text{ m} + 0.03 \times 4.75 \text{ m} = 0.293 \text{ m} \text{ for flow contained within RW})$ $(0.15 \text{ m} + 0.03 \times 4.75 \text{ m} - 0.050 \text{ m} = 0.243 \text{ m})$ Average head of water over curb cut = 0.243 m Curb cut weir coefficient = 1.84 3.406 m³/s for 100-yr event Maximum flow through cub cut =

Therefore the capacity of the curb cut (3.406 m³/s) is higher than the computed overland flow (1.808 m³/s)

WEST OVERLAND FLOW ROUTE DOWNSTREAM OF CURB CUT (NORTH SIDE OF POND)

$Q = 1/n \times AR^{2/3}S^{1/2}$			
	Min. Slope		Max. Slope
normal depth =	0.166	m	0.056
n =	0.03		0.03
Channel width =	15.5	m	15.5
A (area of flow) =	2.572	m^2	0.867
wetted perimeter =	15.832	m	15.612
R (hydraulic radius) =	0.162	m	0.056
S (slope) =	0.005	m/m	0.184
Q (flow) =	1.805	m^3/s	1.805
velocity =	0.70	m/s	2.08

TOWN OF CALEDON PLANNING

Felc No.: 552-05 / July 2018

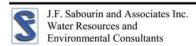
JFSA Ref. No.: 552-05 / July 2018

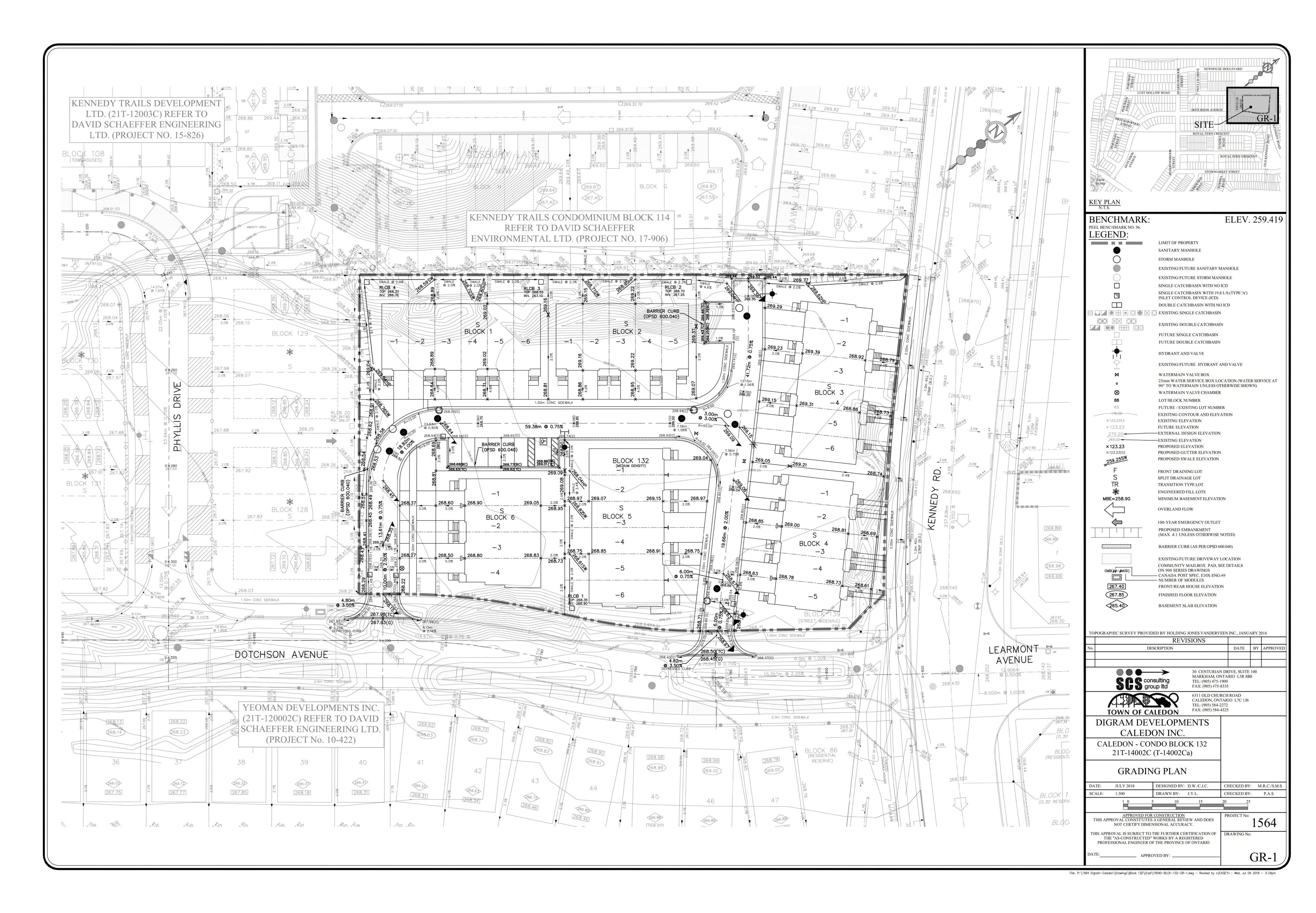
ATTACHMENT

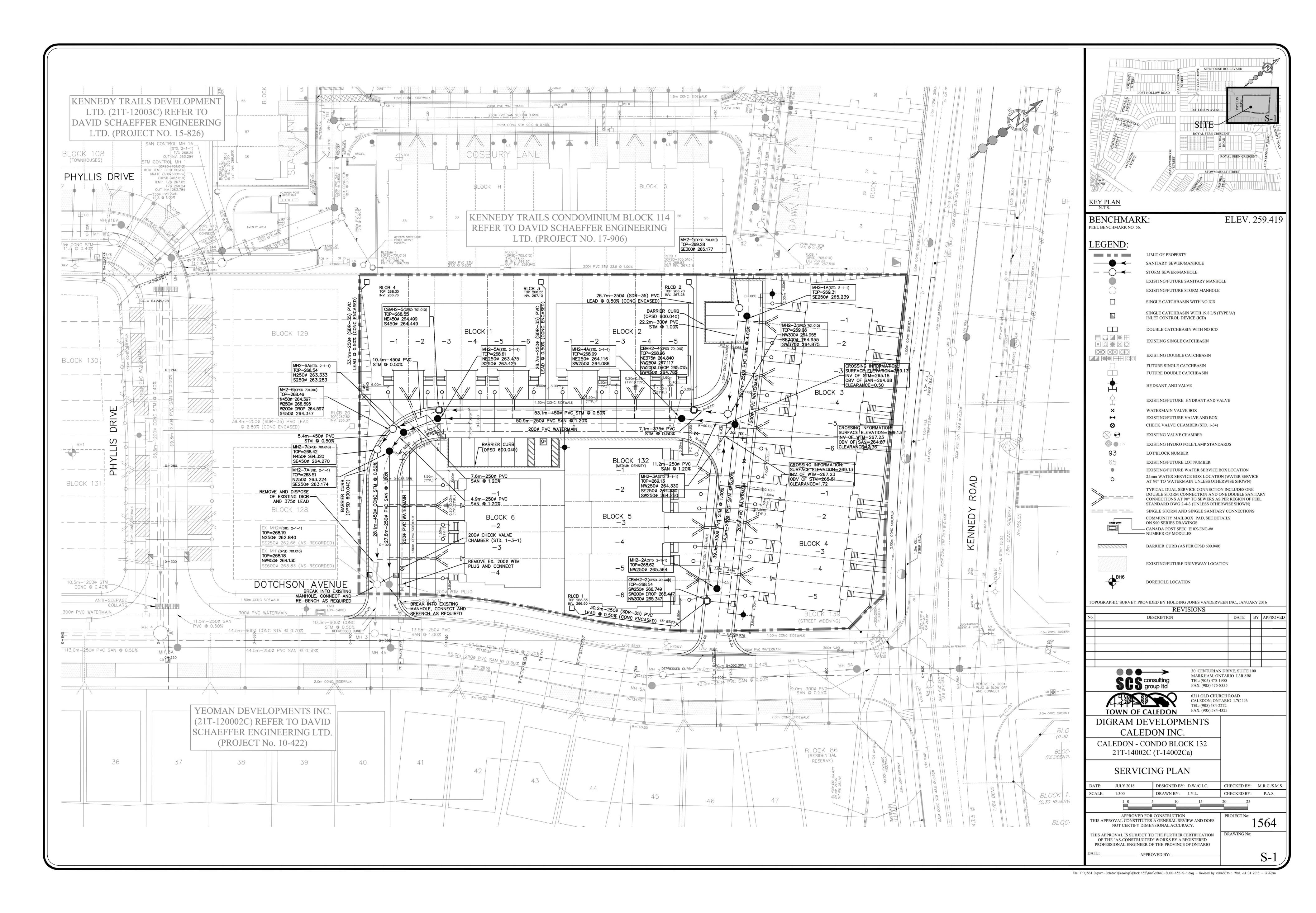
Drawings GR-1, S-1 and STM-1 (July 2018, SCS Consulting Group Limited)

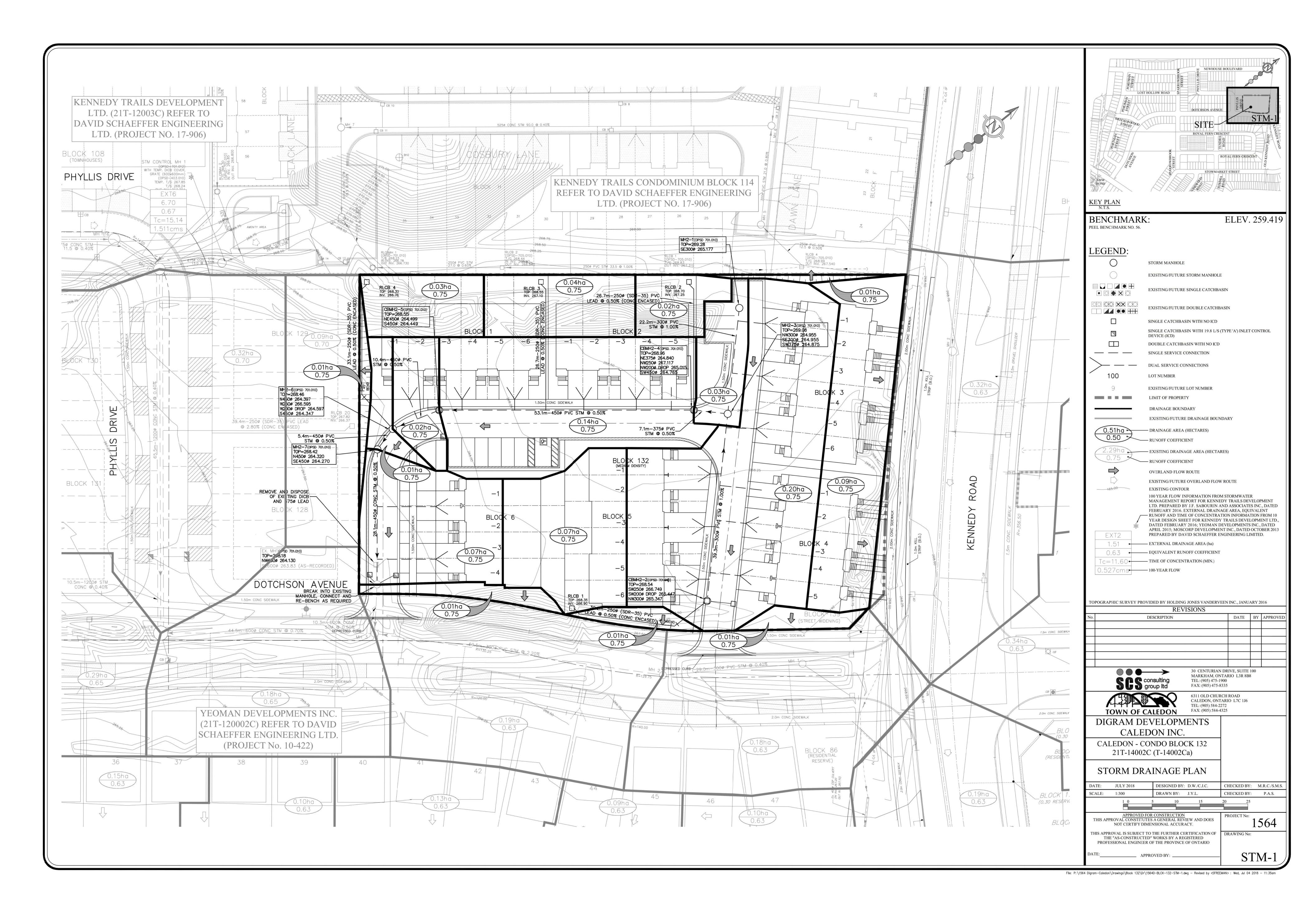












ATTACHMENT B RELEVANT REPORT EXCERPTS



February 18, 2014

Report No. 130177-H4

North Star Investments 1862 Albion Road Rexdale, Ontario M9W 5T2

Attention: Mr. Tony Ferrara

HYDROGEOLOGIC STUDY FOR PROPOSED RESIDENTIAL SUBDIVISION 12654, 12728 AND 12738 KENNEDY ROAD TOWN OF CALEDON, ONTARIO

Prepared for:

North Star Investments



CANADA ENGINEERING SERVICES INC.
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Toronto, Ontario M1T 2H6
Phone 416 492 4000
Fax 416 492 4001
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Hydrogeologic Study 12654, 12728 and 12738 Kennedy Road, Town of Caledon, Ontario February 18, 2014

130177-H4

Underlying the till unit are silty fine sands which have been encountered at depths of 3.0 m to 4.9 m below grade and extended down to the bases of all boreholes at a depth of 6.6 m. These silty fine sand deposits are very dense and are confined under hydrostatic pressure with water levels when allowed to slowly rise, reaching the existing ground surface.

2.2 TOPOGRAPHY

The site is located at 12654, 12728 and 12738 Kennedy Road, Town of Caledon, Ontario, in a predominantly residential area with recently built residential houses and new residential subdivision sites surrounding it.

For convenience Kennedy Road is taken as running in a North-South direction for this report. The site is bounded by Kennedy Road on the east side, new residential subdivision sites on the north, south and west sides. Further east, beyond the Kennedy Road are newly built residential houses and further north beyond the residential subdivision site are vacant farm lands.

The subject lands are gently sloping toward the rear of the subject property in a southwest direction. The site is currently a vacant field with forested lands in the middle portion of 12728 Kennedy Road which at the time of the investigation was almost fully covered with snow. Generally surface water flow is expected to flow naturally toward the south, draining into tributaries of the Etobicoke Creek and is then carried to Lake Ontario, via Etobicoke Creek. Groundwater level in this surficial layer generally parallels the ground surface at depths varying from 0.6 m to 1.5 m in all the boreholes. These groundwater levels more or less parallel the contour of the land.

The project area is relatively flat with a gradual slope to the southwest of the site. The elevation of the site ranges from a maximum of approximately 267 m to the northeast to approximately 261 m to the southwest. Site topography is somewhat influenced by two man-made ponds on the subject property. Based on the aerial photographs, it has been established that these ponds did not exist on the site prior to 1988. Surface water drainage from the site is primarily from ground surface to Etobicoke Creek to the south of the site, and then to Lake Ontario.

Due to the fair permeability surficial silty sand at the site, surface water infiltration will tend to be significantly high following rainfall events. The coefficient of permeability of this surficial silty sand at the site is expected to be around 10^{-3} cm/second.

3.0 HYDROGEOLOGIC PROCEDURE

The field work for the boreholes was carried out with two track-mounted drill rigs with continuous flight solid stem auger equipment on December 23, 2013 and on January 2, 2014. It was supervised by an engineer from our office. A total of eight boreholes were put down over the entire site.

> Hydrogeologic Study 12654, 12728 and 12738 Kennedy Road, Town of Caledon, Ontario

February 18, 2014

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Each of the eight boreholes was put down at the site to a depth of 6.6 m and one piezometer was placed in each borehole. From the boreholes, soil samples were taken at 500 mm intervals between the ground surface and a depth of 3.0 m and thereafter at depths of 1.5 m to the termination of the boreholes.

The samples were taken by means of a split-spoon sampler, in accordance with the requirements of the Standard Penetration Test, (CSA test specifications A119.1).

All the samples taken were brought back to our laboratory where moisture content tests, three grain size analysis tests and further visual observations were carried out. Our field and laboratory findings are plotted on the borehole log numbers 1 to 8.

The locations and elevations of the boreholes were established by the property owner's surveyor, Calder Engineering Limited and are shown on drawing numbers 1 and 2. The geotechnical terms and symbols used in this report are shown in Appendix "A".

3.1 HYDROGEOLOGY

Regional ground water flow at the site is directed to the south, toward Lake Ontario, through the Etobicoke Creek. There are essentially three ground water flow systems present across the site. A shallow unconfined flow system within the medium permeability soils near the surface and a confined slow moving system consisting of silty fine sand, trapped by the dense sand till above. A deeper aquifer system below the silty fine sand within the bedrock at a depth of around 20 m to 30 m used for private well water supplies. Our boreholes do not extend to this aquifer system, but this data was obtained from MOE well records and this final flow system is not expected to be affected by the development of the subdivision. Some groundwater recharge is also expected to occur upstream from higher lands.

All boreholes encountered water in the surficial silty sand layer which was close to the ground surface. No water or silt or sand seams were found within the silty sand till, though they may be present between soil samples.

All boreholes were terminated within the very dense silty fine sand. Piezometers installed in this stratum and sealed in the relatively impermeable silty sand till rose slowly in the piezometers to close to ground surface, indicating that this is a confined aquifer.

Water infiltration and recharge rates for the site and surrounding areas are well covered in a report prepared by Shaheen and Peaker Limited for the site and surrounding areas, referred to as the Mayfield Community. They estimated a recharge rate for the area on the order of 150 mm/a.

Hydrogeologic Study 12654, 12728 and 12738 Kennedy Road, Town of Caledon, Ontario February 18, 2014

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Detailed soil descriptions are shown on the borehole log numbers 1 to 8. Grain size analysis curves of the different soil types found at the site were plotted and are shown in Figure No. 1.

3.3 STRATIGRAPHY

The site consists mainly forested and grass lands with residential houses in front. A defined topsoil layer was found at the surface of all the boreholes, This was followed by a layer of silty sand, which in some places was reworked or relocated at the site.

Below this was a very dense silty sand till. This in turn was underlain by a very dense silty fine sand, confined aquifer where all the boreholes were terminated at a depth of 6.6 m.

3.4 GROUND AND SURFACE WATER FLOW

One piezometer was installed in each of the boreholes and was extended down close to the base of the borehole. The piezometers were sealed in the dense silty sand till and at the surface and therefore the water levels obtained from these piezometers were of the confined very dense silty fine sand below the silty sand till. Water levels in borehole numbers 1, 2, 3, 4, 5 and 6 were at depths of 1.8 m (elev 282.28 m), 1.6 m (elev 282.16 m), 1.5 m (281.84 m), 1.5 m (elev 280.06 m), 2.7 m (elev 277.54 m) and 2.3 m (elev 280.02 m) respectively. The soils in borehole numbers 1 to 6 caved at depths of 3.2 m, 5.2 m, 4.1 m, 3.1 m, 3.1 m and 5.2 m respectively. The water levels taken in these piezometers represent the confined aquifer water levels and are summarized in the table below:

Date Taken	Borehole Number (Depth/Elevation: (m))												
	1	2	3	4	5	6	7	8					
Upon Completion date: Dec 23, 2013	5.50/ 261.56	5.5/ 261.42	N/A	N/A	4.0/ 257.93	4.60/ 258.81	4.60/ 259.4	N/A					
Upon Completion date: Jan 2, 2014	N/A	N/A	0.60/ 265.42	4.60/ 260.77	N/A	N/A	N/A	0.30/ 263.32					
January 2, 2014	1.13/ 265.9	1.90/ 265.02	N/A	N/A	1.3/ 260.63	0.60/ 262.81	1.7/ 262.3	N/A					
January 9, 2014	1.14/ 265.91	1.90/ 265.02	1.40/ 266.22	2.40/ 262.97	1.60/ 260.93	0.70/ 262.91	1.90/ 262.5	0.00/ 263.62					

De-watering is recommended prior to excavation to maintain stable soil footings and to assist in maintaining stable soil slopes for open cut excavation.

Feb 16, 2022

