



TOWN OF CALEDON
PLANNING
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Toronto, ON | cfcrozier.ca
M5B 1M4

MEMO

DATE September 1, 2020 **PROJECT NO.** 1716-5554
RE Floodplain Analysis
12476 Regional Road 50, Town of Caledon, Region of Peel

TO Anthony Syhlonyk, Planner (Toronto and Region Conservation Authority)
FROM Rebecca Archer, P.Eng.
CC Bikram Dhillon (BVD Petroleum Inc.)

INTRODUCTION

C.F. Crozier and Associates Inc. (Crozier) was retained by BVD Petroleum Inc. (the Owner) to complete a floodplain analysis to support the Site Plan Application for a proposed development at 12476 Regional Road 50 in the Town of Caledon. The purpose of this memo is to document the methodology and results of the floodplain analysis and demonstrate that the proposed site grading design meets the criteria of the Toronto and Region Conservation Authority (TRCA) for the lands in the vicinity of the site.

BACKGROUND

The subject property is approximately 0.95 ha and is currently vegetated undeveloped land. The property is located in a commercial/industrial area, bounded by George Bolton Parkway to the south, an industrial property to the west, Robinson Creek to the north, and a gas station and Robinson Creek to the east. It is understood that the proposed development includes the construction of a five-storey hotel and associated parking area. A grading plan for the site was completed by Flora Designs Inc. in July 2020.

Robinson Creek is a tributary of the Humber River, making the site part of the TRCA Regulated Area of the Humber River Watershed. A portion of the Robinson Creek valley corridor encroaches into the subject property, bordering the future parking lot area along the north and east limits of the site. As a result, based on the extents of the creek's high-water elevation corresponding to the Regional rainfall event, part of the north and east portions of the property are located within the floodplain. As such, a floodplain analysis is required to satisfy TRCA requirements prior to proceeding with a Site Plan Application.

METHODOLOGY

The floodplain analysis included the following tasks:

- Reviewing the Robinson Creek hydraulic (HEC-RAS) model, prepared by Cole Engineering February 2016, provided by the TRCA.

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- Reviewing the Humber River Hydrology update, prepared by Civica Infrastructure, April 2018.
- Establishing existing Regional floodplain conditions on the site based on the approved (2016) HEC-RAS model with updated hydrology and updated sections based on more recent topographic and LiDAR surveys completed for the area.
- Interpolating proposed cross sections for the site and updating the HEC-RAS model accordingly. Existing and proposed conditions were compared to assess the impact of flooding on-site and off-site.

EXISTING CONDITIONS

Hydrologic Verification

A review of the hydrologic modeling completed for this site showed that the catchment areas provided in the 2016 Hydraulic Assessment were consistent with those in the current approved 2018 Humber River Hydrology Update. The site is located in Catchment 22.23 in the approved Humber River Hydrology model. The TRCA model adds flows from Catchments 22.23, 22.24, 22.25 and 22.23 downstream of the site, for a total drainage area of 145.57 ha. Of this area approximately 8.74 ha of area 22.23 is upstream of the site, reducing the upstream drainage area to 90.04 ha.

In order to confirm if the upstream drainage areas flowed south down Highway 50 (bypassing the site) or west through the smaller streets to be picked up by the tributary that flows through the site, we completed the following background review:

- As-built drawings were obtained and reviewed to determine the location of sewers, ditches and culverts in the area. As-built drawings used for this review are provided as an attachment to this report;
- 1m Bare earth LiDAR data from 2019, purchased from first base solutions, was used to confirm the upstream drainage patterns; and,
- A site walk (on June 22, 2020) was completed to confirm drainage at the intersection of Healey Road and Queen Street South (Highway 50).

Based on this background review we have determined that:

- Area 22.30 drains to a culvert west of Highway 50. This culvert conveys runoff under the railroad tracks to a ditch which flows south between Hardwick Road and Highway 50. This ditch terminates at a retaining wall just north of the intersection of Healey Road and Queen Street South (Highway 50), where it is picked up by a culvert which connects to a storm sewer under the sidewalk on the west side of Highway 50.
- A similar system picks up the ditch on the east side of the road and convey runoff to a separate storm sewer on the west side of the road.
- Runoff from area 22.25 is also picked up by this sewer system, which continues south along Highway 50 to outlet back into roadside ditches just North of McEwan Drive.

The findings of this background review are provided in **Figure 1**. Copies of the as built drawings and photographs from the site walk are provided as attachments.

Based on the findings of the background review the approved Humber River model (Civica 2018) was trimmed and modified to reflect detailed external drainage conditions. A copy of the update model is included as an attachment to this report. Modifications to the model included adding additional ADDHYD and splitting catchments. Table 1 shows the changes in area applied to the VO model.

Table 1 – Changes to VO model

TRCA ID	TRCA Area	Vo Model		Transposition of Flow		
		Crozier ID	Crozier Area	Flow Nodes	Crozier Incremental Area	Crozier Cumulative Area
22.24	48.66 ha	22.24	48.66 ha	1	28.31 ha	28.31 ha
				2	20.35 ha	48.66 ha
22.23	64.27	22.23 NW	8.25 ha		3.81 ha	52.47 ha
				Site	4.44 ha	56.91 ha
		22.23 NE	8.88 ha		8.88 ha	65.79 ha
22.30	21.04 ha	22.30	21.04 ha		21.04 ha	86.83 ha
22.25	11.60 ha	22.25	11.60 ha	3	11.60 ha	98.43 ha
22.23	64.27	22.23	47.14 ha	4	10.95 ha	109.38 ha
				5	21.08 ha	130.46 ha
				6	15.11 ha	145.57 ha
22.22	134.85	22.22	134.85 ha	7	134.85 ha	
Total				8		280.42 ha

Note: Area 22.23 was divided into six area to better define the contributing drainage to the site. The area in the total area for this catchment in the TRCA model is 64.27 ha.

The results of the VO model were used to calculate flows at previously defined flow nodes used in the 2016 Cole Hydraulic update. Flows for nodes 2, 3, 6, 7, and 8 were calculated directly from the model. Using the MTO equation for the transposition of flood discharge updated flows were calculated immediately downstream of the site and at previously defined flow change locations 1, 4, and 5 within the model. **Table 1** provides a summary of the flows used in the model.

Table 1 Summary of Steady Flow Data

Node	Station	U/S Drainage Area (ha)	Hurricane Hazel Peak Flow (m ³ /s)
1	2224.08	28.31	4.76
2	2223.56	48.66	7.15
Site	2223.463	56.91	7.24
3	2223.45	98.43	13.35
4	2223.37	109.38	15.17
5	2223.20	130.46	17.32
6	2223.04	145.57	18.80
7	--	134.85	19.75
8	2219.56	280.42	38.54

Existing floodplain

The topographic survey indicates that the property has split drainage where approximately half of the property slopes towards Robinson Creek, while the other half slopes towards George Bolton Parkway. Based on the mapping provided by TRCA, the Regional floodplain extends onto approximately one third of the property. A review of the existing HEC-RAS model shows that cross-section stations 2223.44 (downstream) through 2223.47 (upstream) bound the site. As a result, these cross sections are the focus of this analysis.

In addition to the updates made to the model flow files discussed earlier in this memo, the existing sections were also updated based on the topographic and LiDAR surveys completed for the site. Existing stations 2223.44 to 2223.46 were updated based on the collected information, and five (5) additional sections were also created within the property to better represent the site geometry and improve model accuracy. Refer to **Figure 2** Floodplain Mapping (with culvert) for the locations of the existing and new cross-sections as well as the existing and proposed floodlines. Existing flood elevations and velocities for Robinson Creek are provided in **Table 3**. Note that two scenarios were analyzed; the first represents the actual existing conditions, which includes the rating curve representing the culvert at station 2223.44 and the second represents idealized “baseline” conditions, which does not consider the culvert at station 2223.44.

Table 3 Existing Flood Elevations and Velocities for Robinson Creek

Station	Location	Actual Conditions			Idealized Conditions		
		Regional Peak Flow (m ³ /s)	Regional Flood Elevation (m)	Regional Flood Velocity (m/s)	Regional Peak Flow (m ³ /s)	Regional Flood Elevation (m)	Regional Flood Velocity (m/s)
2223.47	U/S of the site	7.15	238.91	0.30	7.15	238.90	0.30
2223.463	Within the site (additional)	7.24	238.79	1.25	7.24	238.76	1.35
2223.462	Within the site (additional)	7.24	238.73	1.16	7.24	238.47	1.97
2223.461	Within the site (additional)	7.24	238.77	0.36	7.24	238.32	1.03
2223.46	Within the site	7.24	238.76	0.24	7.24	238.22	0.84
2223.452	Within the site (additional)	7.24	238.76	0.21	7.24	238.20	0.58
2223.451	Within the site (additional)	7.24	238.76	0.14	7.24	238.20	0.32
2223.45	D/S of the site	13.35	238.76	0.42	13.35	238.16	0.91
2223.44	D/S of the site	13.35	238.75	0.56	13.35	237.91	2.23

PROPOSED CONDITIONS

A grading and servicing plan for the site was completed by Flora Designs Inc. in July 2020. The grading plan proposes fill to accommodate parking lot area within portions of the existing floodplain with 3:1 slopes projecting into the valley along the north and east property lines, sloping towards Robinson Creek, raising the site above the Regional flood elevation. This results in the entire parking lot area being free of flooding. Drainage on site is designed to capture rainfall within an internal network of catch basins and catch basin manholes, which ultimately outlet to the

storm sewer on George Bolton Parkway. Overland flow during major storm events is directed towards the creek.

It is noted that the portions of the proposed development limit for this site have been established based on the previously approved 15m valley corridor (centred on existing channel axis), as part of the development application for the previous land owner (Farview Holdings / Pilla Investments Inc.). For reference, the Farview Holdings Site Servicing and Grading Plan illustrating the 15m valley corridor, as approved by the TRCA Executive Committee on September 07, 2007 is attached to this memo.

Figure 3 Floodplain Mapping - Without Culvert illustrates the idealized condition comparing the existing and proposed base condition floodplain mapping with the downstream Highway 50 culvert removed. The proposed cross sections overlaid against the existing cross sections are also attached. **Table 4** and **Table 5** summarize the results of the proposed conditions model.

Table 4 Summary of Flood Elevations for the Proposed Conditions Model

Station	Location	Proposed Regional Peak Flow (m ³ /s)	Actual Regional Flood Elevation (m)			Idealized Regional Flood Elevation (m)		
			Pre	Post	Diff.	Pre	Post	Diff.
2223.47	U/S of the site	7.15	238.91	239.93	0.02	238.90	238.87	-0.03
2223.463	Within the site (additional)	7.24	238.79	238.80	0.01	238.76	238.70	-0.06
2223.462	Within the site (additional)	7.24	238.73	238.76	0.03	238.47	238.40	-0.07
2223.461	Within the site (additional)	7.24	238.77	238.77	0.00	238.32	238.32	0.00
2223.46	Within the site	7.24	238.76	238.77	0.01	238.22	238.27	0.05
2223.452	Within the site (additional)	7.24	238.76	238.77	0.01	238.20	238.25	0.05
2223.451	Within the site (additional)	7.24	238.76	238.77	0.01	238.20	238.25	0.05
2223.45	D/S of the site	13.35	238.76	238.76	0.00	238.16	238.21	0.05
2223.44	D/S of the site	13.35	238.75	238.75	0.00	237.91	237.92	0.01

Table 5 Summary of Channel Velocities for the Proposed Conditions Model

Station	Location	Proposed Regional Peak Flow (m ³ /s)	Actual Regional Flood Velocities (m/s)			Idealized Regional Flood Velocities (m/s)		
			Pre	Post	Diff.	Pre	Post	Diff.
2223.47	U/S of the site	7.15	0.30	0.29	-0.01	0.30	0.31	0.01
2223.463	Within the site (additional)	7.24	1.25	1.34	0.09	1.35	1.59	0.24
2223.462	Within the site (additional)	7.24	1.16	1.19	0.03	1.97	2.15	0.18
2223.461	Within the site (additional)	7.24	0.36	0.68	0.32	1.03	1.24	0.21
2223.46	Within the site	7.24	0.24	0.39	0.15	0.84	0.74	-0.10
2223.452	Within the site (additional)	7.24	0.21	0.39	0.18	0.58	0.68	0.10
2223.451	Within the site (additional)	7.24	0.14	0.33	0.19	0.32	0.58	0.26
2223.45	D/S of the site	13.35	0.42	0.33	-0.09	0.91	0.58	-0.33
2223.44	D/S of the site	13.35	0.56	0.46	-0.10	2.23	2.15	-0.08

Modeling results show that the proposed grading does not impact the actual flood elevations downstream of the site, however, water surface elevation across the property are increased by a maximum of 0.03 m at Station 2223.462, while the water surface elevation for the cross section immediately upstream (Station 2223.47) is increased by 0.02 m. At the next upstream cross section (Station 2223.48), the water level converges to a zero difference between the existing and proposed conditions. This change in water elevation is considered minor and within the tolerance of the model. The model also shows a change in velocity ranging between -0.10 m/s to 0.32 m/s within the site. Station 2223.45 downstream of the site experiences an decrease in velocity of 0.09 m/s, while Station 2223.47 upstream of the site experiences a decrease of 0.01 m/s. Therefore, the changes in velocities outside of the site are minor and do not increase flood risk for other property owners. A digital copy of the HEC-RAS model, with scenarios showing the updated existing and proposed geometries as well as the updated peak flows is included with this memo.

CUT AND FILL ANALYSIS

The idealized no culvert “baseline” condition is the focus of the cut and fill analysis for the subject site. Any fill placed within the backwater areas caused by the downstream culvert constriction at Highway 50 have been omitted for the purpose of this analysis, which is understood to be in line with the TRCA policies for this area.

Under proposed conditions, fill is proposed to increase the elevation of the banks along the site limits to lift the site above the floodplain providing a minimum of 0.30m of freeboard above the Regional water level, while cut is proposed at the bottom of the channel to create a more consistently sloping channel with defined low flow geometry. We understand that the finished floor elevation (FFE) of the proposed hotel will be at an elevation of at least 239.60 m, allowing for a minimum of 0.64 m of freeboard above the Regional flood elevation. Refer to **Figure 4** for an illustration of the proposed cut and fill areas, including provided cross-sections at each HEC-RAS River Station ID.

The grading plan maintains the existing drainage pattern and has no negative impact on the conveyance of the floodplain. The proposed channel grading has a maximum slope within the floodplain of 3:1 and does not utilize retaining walls.

The seven (7) proposed cross sections overlaid against the existing cross-sections are illustrated on all attached figures for reference. The incremental and cumulative cut and fill volumes under proposed conditions are summarized in **Table 5**.

Table 5 Incremental Cut/Fill Analysis Results

Elevation Increment (masl)	Cumulative Cut/Fill Volume		Incremental Cut/Fill Volume		
	Ex. Storage Volume (m ³)	Prop. Storage Volume (m ³)	Ex. Incremental Storage Volume (m ³)	Prop. Incremental Storage Volume (m ³)	Fill in Floodplain* (m ³)
236.8 to 237.1	4.88	7.76	4.88	7.76	-2.88
237.1 to 237.4	104.58	145.57	99.7	137.81	-38.11
237.4 to 237.7	505.05	533.35	400.47	387.78	12.69
237.7 to 238.0	1191.50	1171.69	686.45	638.34	48.11
238.0 to 238.3	2197.21	1975.29	1005.71	803.6	202.11
238.3 to 238.6	3310.26	2861.53	1113.05	886.24	226.81
238.6 to 239.76	3915.14	3338.75	604.88	477.22	127.66
				Total	576.39

* Negative value indicates cut.

Based on **Table 5**, the cumulative fill volume below the floodplain elevation is greater than the cumulative cut volume, resulting in increased floodplain storage volume under proposed conditions. In fact, the net cut and fill volume proposed is a fill of 576 m³. In addition, the cut volume is proposed at lower elevations than the fill volume. As noted above, the filling activities proposed for this site fully respect the previously approved development limit for the subject lands as based on the accepted 15m valley corridor.

CONCLUSIONS AND RECOMMENDATIONS

The proposed grading design has no adverse impacts on water levels and channel velocities upstream or downstream of the subject property.

We trust that this analysis meets the TRCA requirements to support the proposed development of the site and recommend that the floodplain analysis contained in this memo is accepted such that the Owner can proceed with the Site Plan Application for the proposed works.

Please contact the undersigned if you have any questions.

Sincerely,

C.F. CROZIER & ASSOCIATES INC.



Isabelle Cléroux, E.I.T.
Land Development

C.F. CROZIER & ASSOCIATES INC.

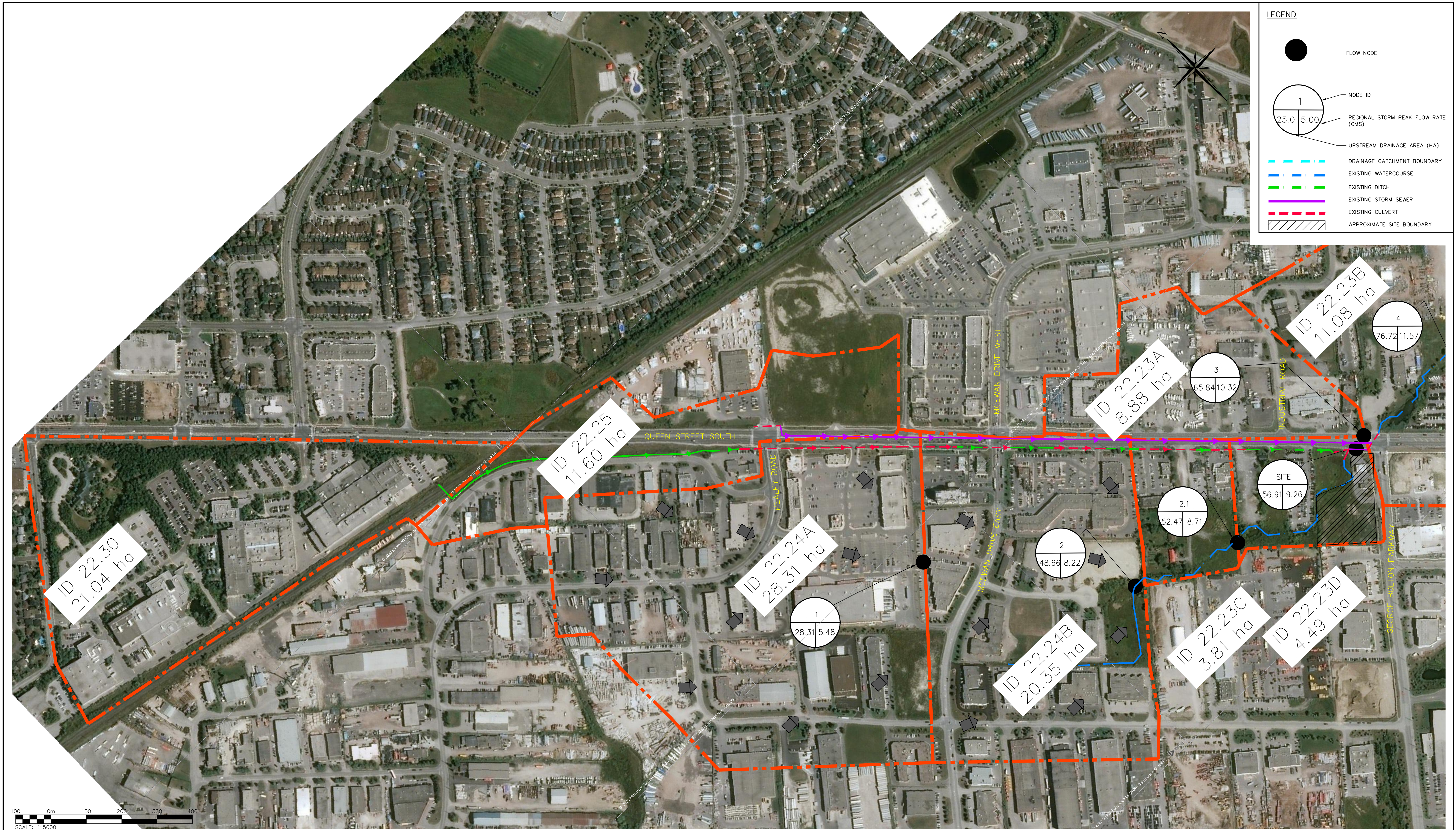


Rebecca Archer, P.Eng.
Senior Project Engineer

Encl.

- Figure 1** Hydrology Mapping
- Figure 2** Floodplain Mapping – With Culvert
- Figure 3** Floodplain Mapping – Without Culvert (Idealized)
- Figure 4** Cut/Fill Mapping
- As-Constructed Drawings for Highway 50
- Photographs from the June 22, 2020 site walk
- Farview Holdings Site Servicing and Grading Plan – TRCA Approved Drawing
- Existing and Proposed Cross Sections (August 2020)
- HEC-RAS Model Output Tables
- Digital copy of the updated HEC-RAS Model
- Digital copy of the updated Hydrology Model (Visual OTTHYMO)

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LEGEND

- FLOW NODE
- NODE ID
- REGIONAL STORM PEAK FLOW RATE (CMS)
- UPSTREAM DRAINAGE AREA (HA)
- DRAINAGE CATCHMENT BOUNDARY
- EXISTING WATERCOURSE
- EXISTING DITCH
- EXISTING STORM SEWER
- EXISTING CULVERT
- APPROXIMATE SITE BOUNDARY

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Town

No.	ISSUE	DATE: MM/DD/YYYY	Engineer
0	ISSUED FOR TRCA APPROVAL	09/01/2020	

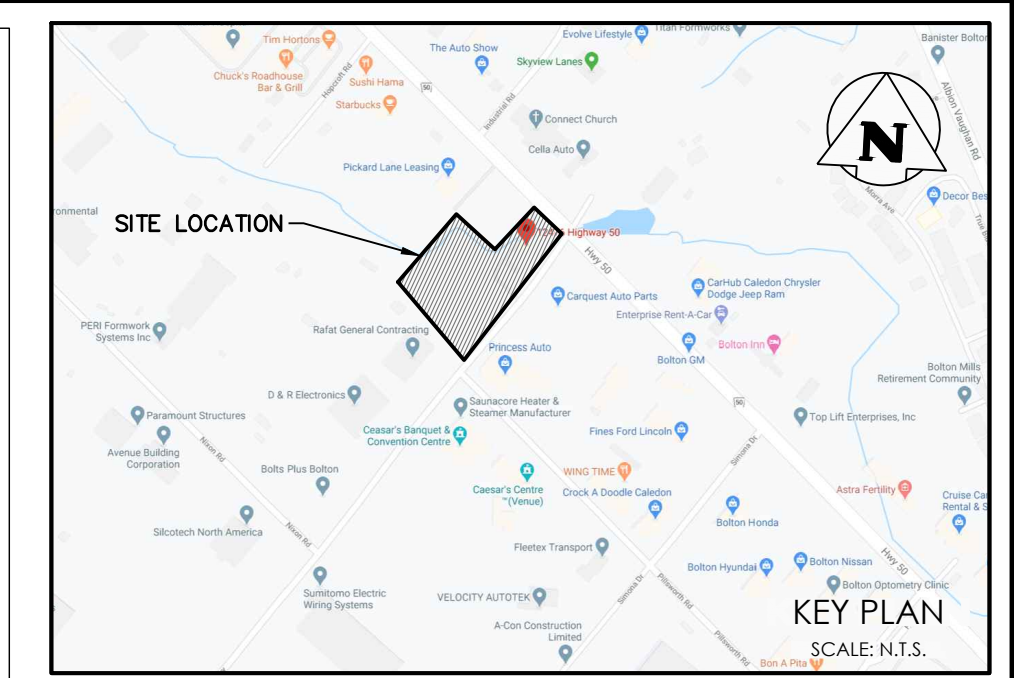
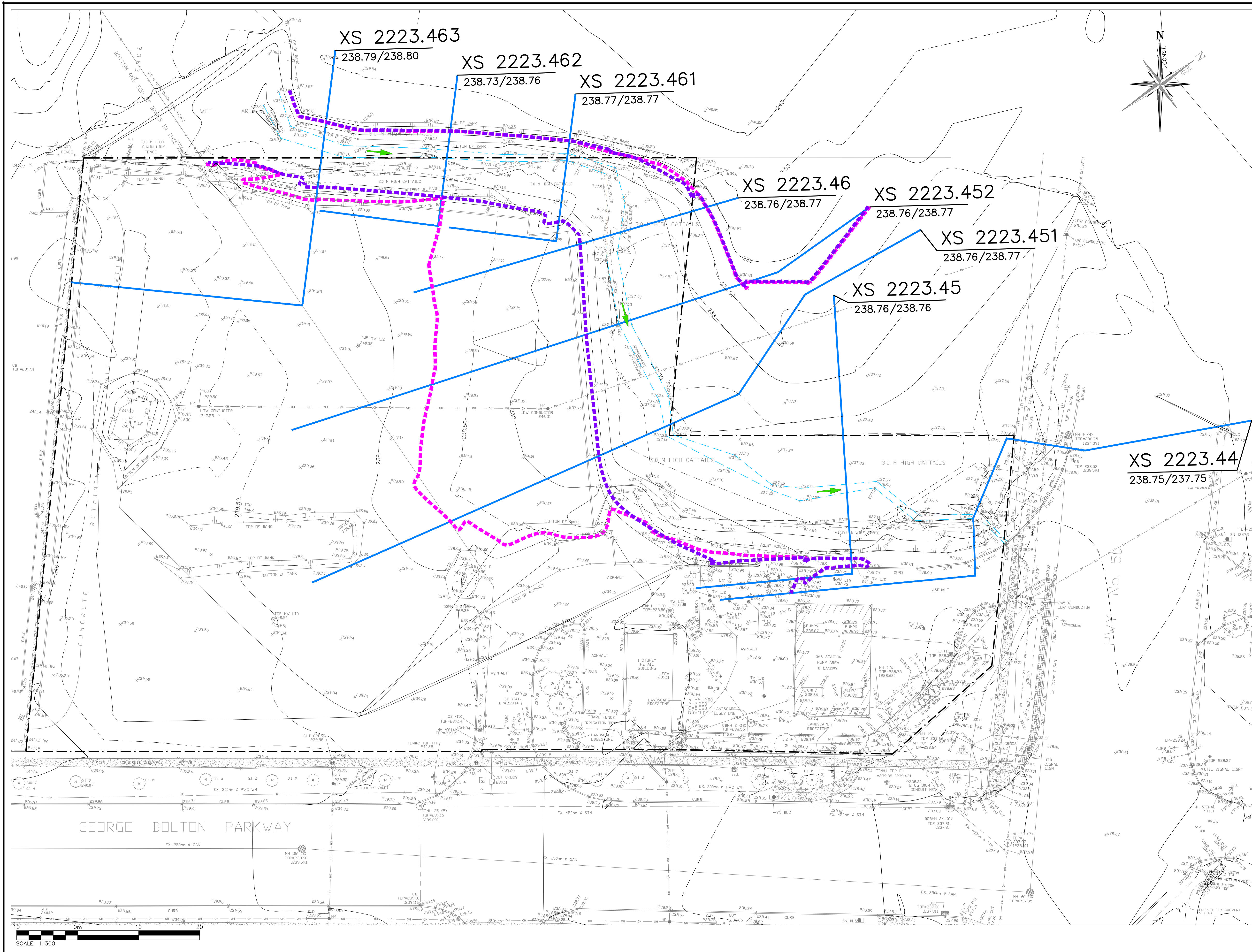
Project: 12476 REGIONAL ROAD 50
TOWN OF CALEDON

Drawing: STORM DRAINAGE PLAN

Drawn By: B.L. Design By: B.L. Project: 1716-5554

Check By: R.S.A. Check By: Scale: 1:3000 Drawing: FIG 1

57 John Street West
P.O. Box 1011
Bradford, ON L3Z 2B4
905-952-3111 T
www.cfcrozier.ca



LEGEND

- PROPERTY LINE
- - - EXISTING CONTOUR (0.5m)
- - - EXISTING CONTOUR (1.0m)
- EXISTING WATERCOURSE
- CROSS-SECTION
- CROSS-SECTION I.D.
- XS ID
- 215.00/215.00
- EX. REG. FLOODLINE WSEL W/ CULVERT (m)
- PROP. REG. FLOODLINE WSEL W CULVERT (m)
- EXISTING REGULATORY FLOODLINE (WITH CULVERT)
- PROPOSED REGULATORY FLOODLINE (WITH CULVERT)

A	ISSUED FOR SUBMISSION	2020/AUG/24
No.	ISSUE / REVISION	YYYY/MM/DD

ELEVATION NOTE:
ELEVATIONS SHOWN ON THIS PLAN ARE DERIVED FROM THE MINISTRY OF TRANSPORTATION BENCHMARK No. 758056
ELEVATION = 000.000m

LOCAL BENCHMARK:
BM1 = CUT CROSS IN SIDEWALK
ELEVATION = 239.58m
BM1 = CUT CROSS IN SIDEWALK
ELEVATION = 239.59m

SURVEY NOTES:
PRELIMINARY SURVEY COMPLETED BY VAN HARTEN SURVEYING INC. (2020/MAY/06)
BEARINGS ARE UTM GRID, DERIVED FROM RTN OBSERVATIONS
UTM ZONE 17, NAD83 (GRS) (2010.0)
DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.9996781

SITE PLAN NOTES:
DESIGN ELEVATIONS ARE BASED ON SITE PLAN BY ANTRIX ARCHITECTS INC.

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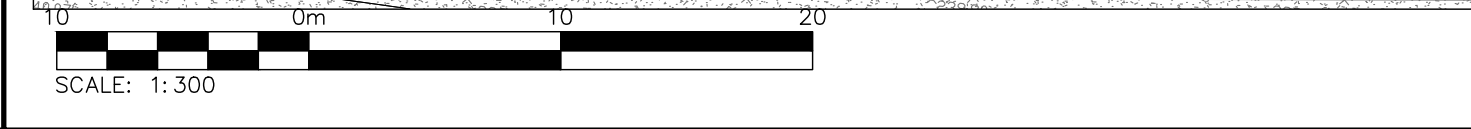
Project
**12476 HWY 50
TOWN OF BOLTON, ON**

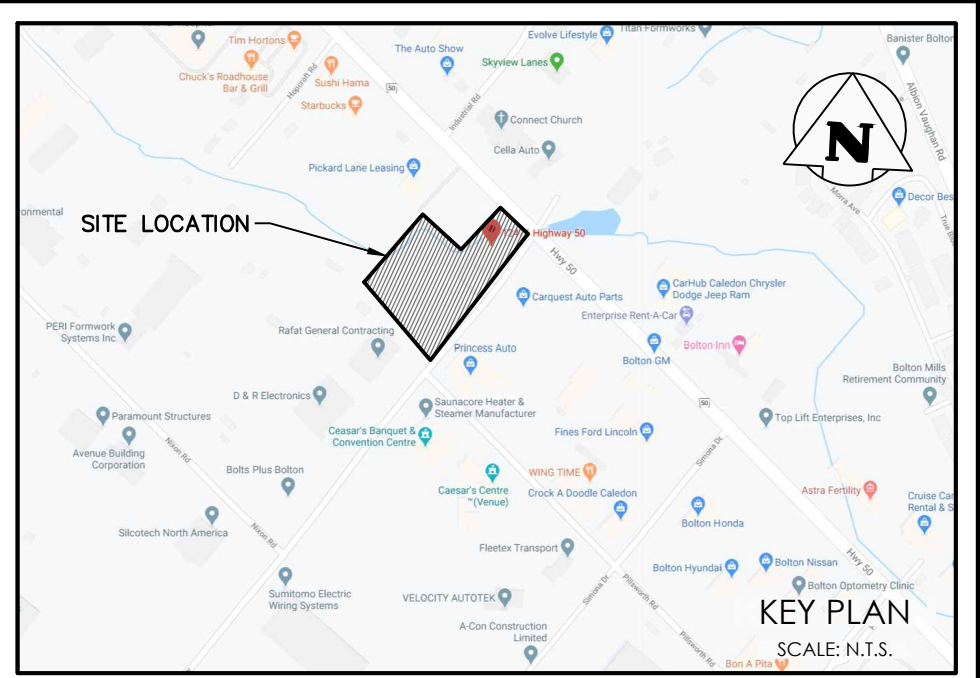
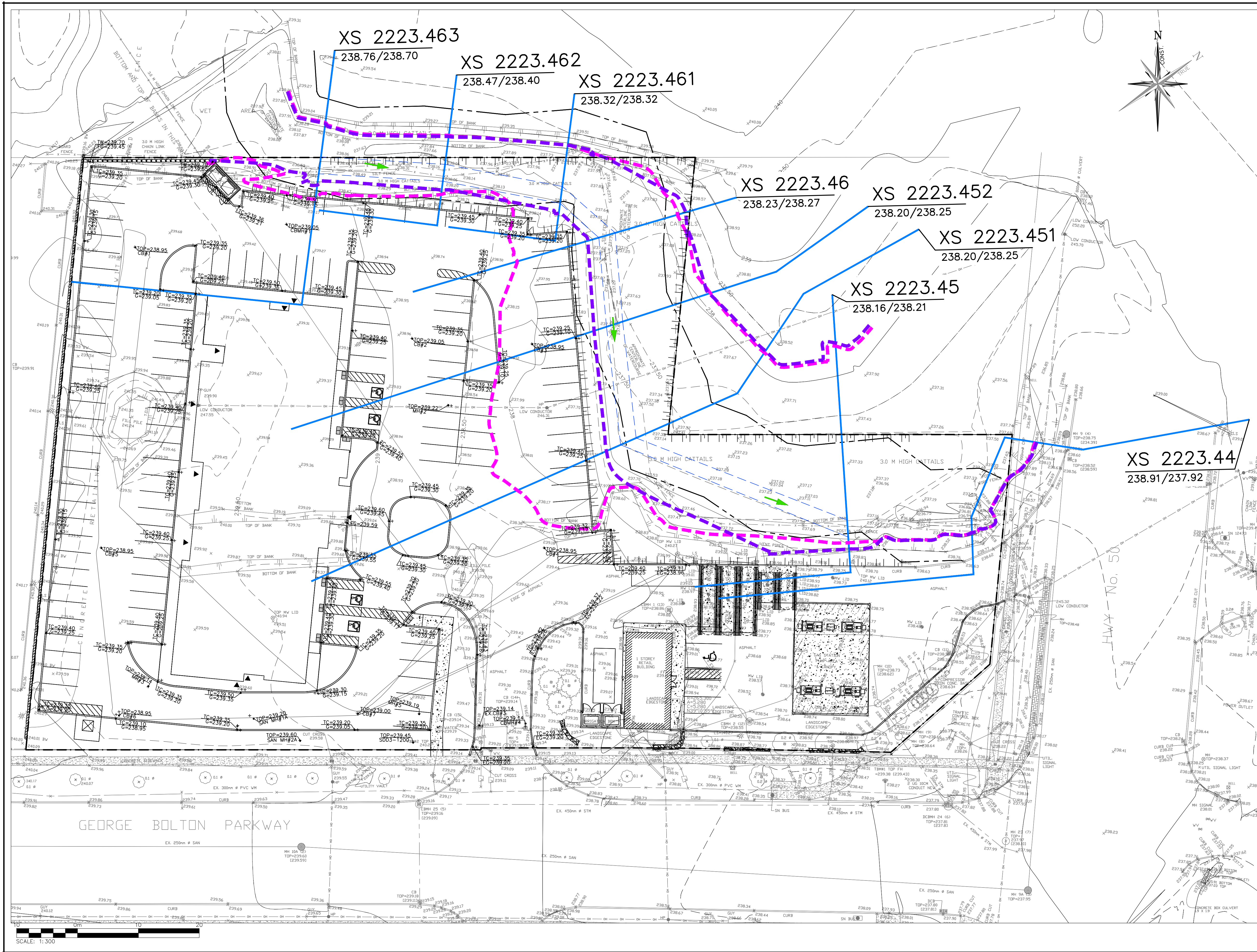
Drawing
FLOODPLAIN MAPPING – WITH CULVERT

CROZIER CONSULTING ENGINEERS
211 YONGE STREET
SUITE 301
TORONTO, ON M5B 1M4
416-477-3392 T
WWW.CFCROZIER.CA

Drawn	B.L.	Design	I.C.	Project No.	1716-5554	
Check	R.A.	Check	D.W.	Scale	1:300	
					Dwg.	FIG 2.

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LEGEND

- PROPERTY LINE
- EXISTING CONTOUR (0.5m)
- EXISTING CONTOUR (1.0m)
- PROPOSED WATERCOURSE
- PROPOSED 3:1 GRADING
- CROSS-SECTION
- XS ID**
- 215.00/215.00** EX. REG. FLOODLINE WSEL W/O CULVERT (m) | PROP. REG. FLOODLINE WSEL W/O CULVERT (m)
- EXISTING REGULATORY FLOODLINE (WITHOUT CULVERT)
- PROPOSED REGULATORY FLOODLINE (WITHOUT CULVERT)

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Project
**12476 HWY 50
TOWN OF BOLTON, ON**

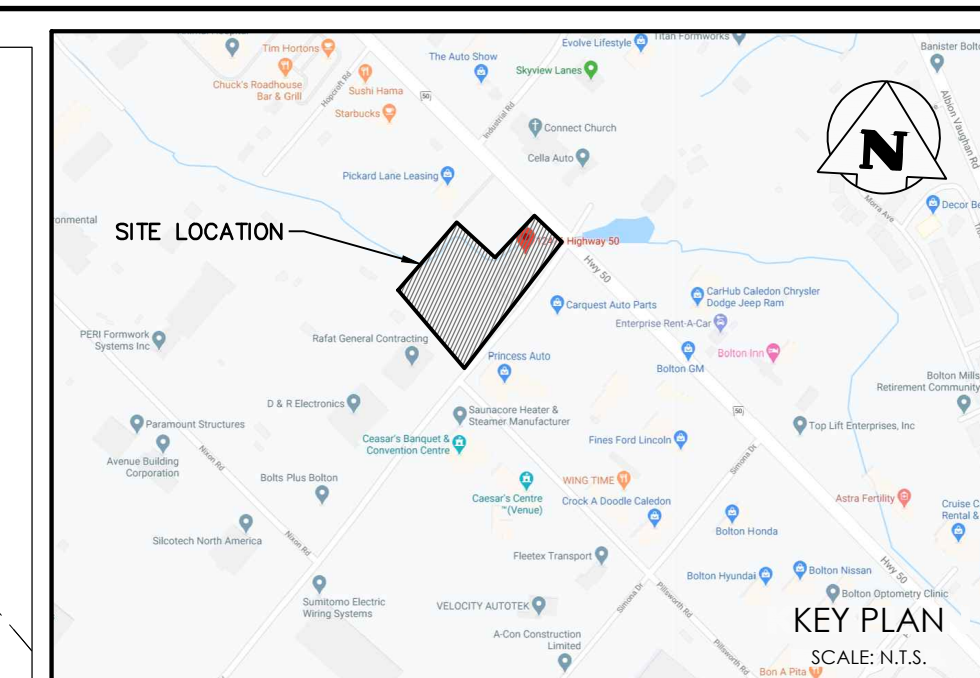
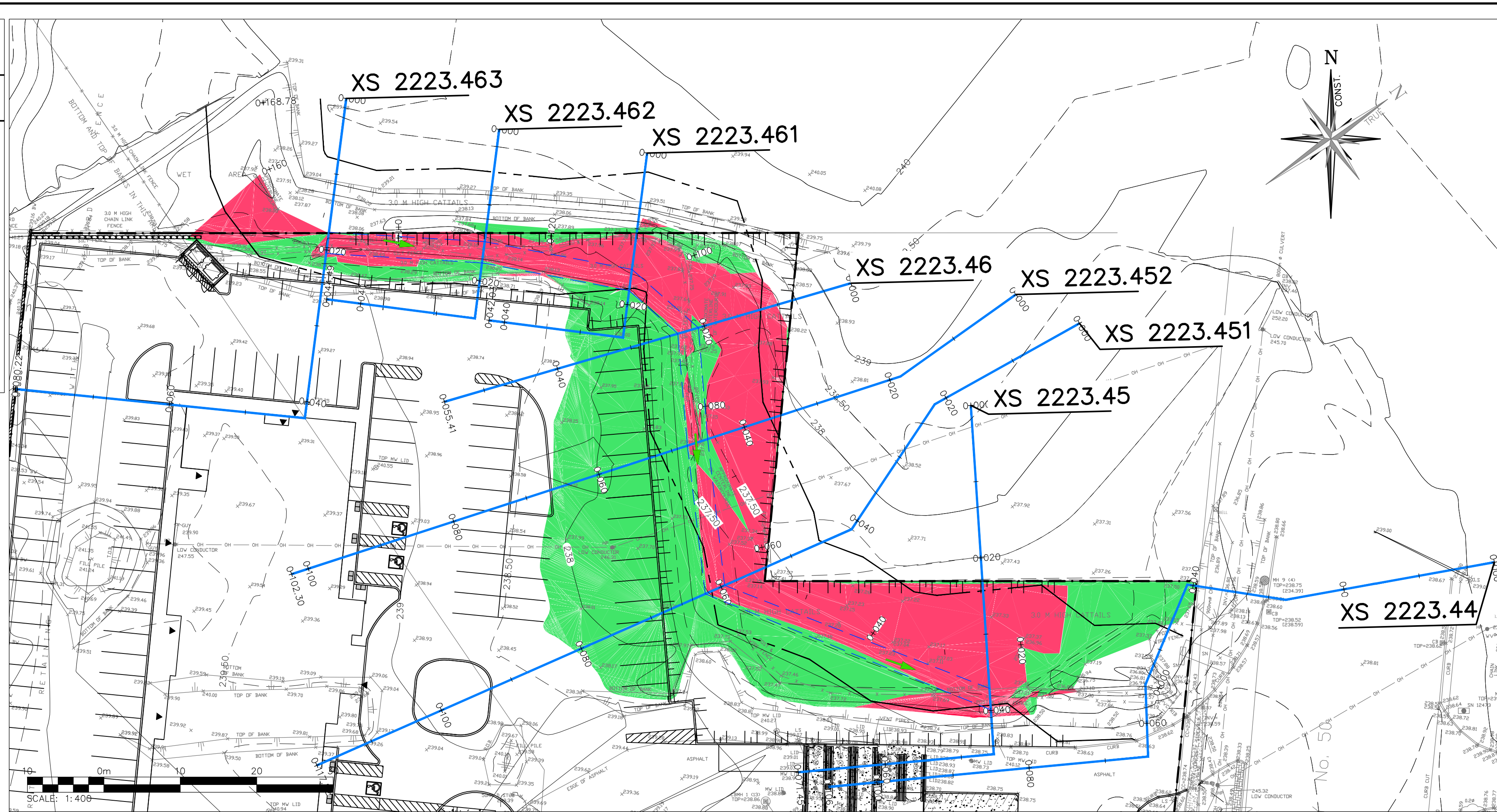
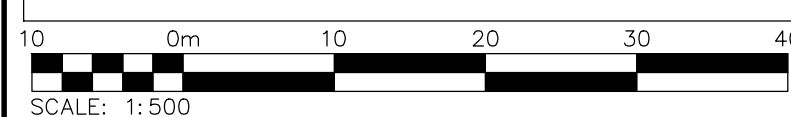
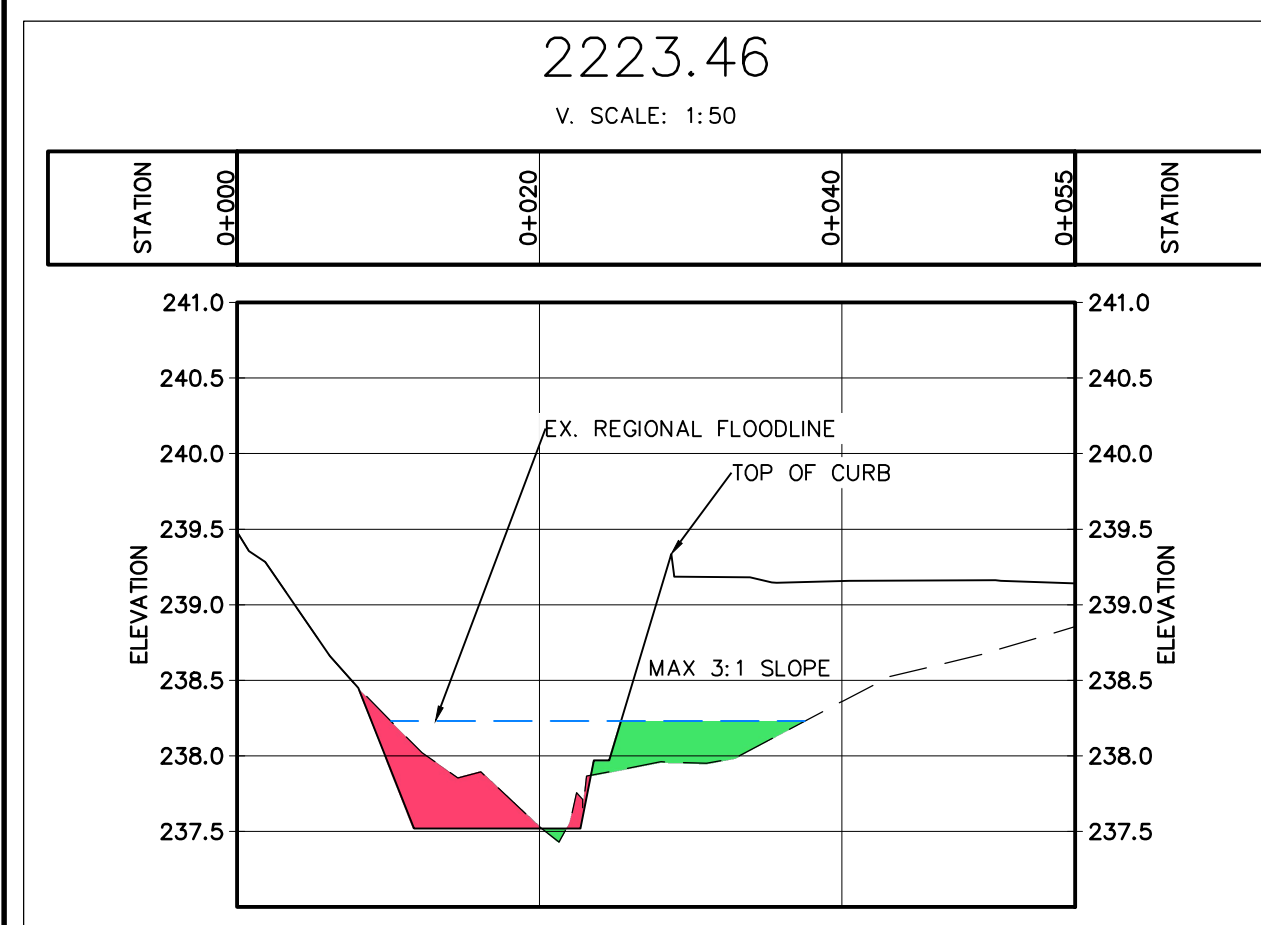
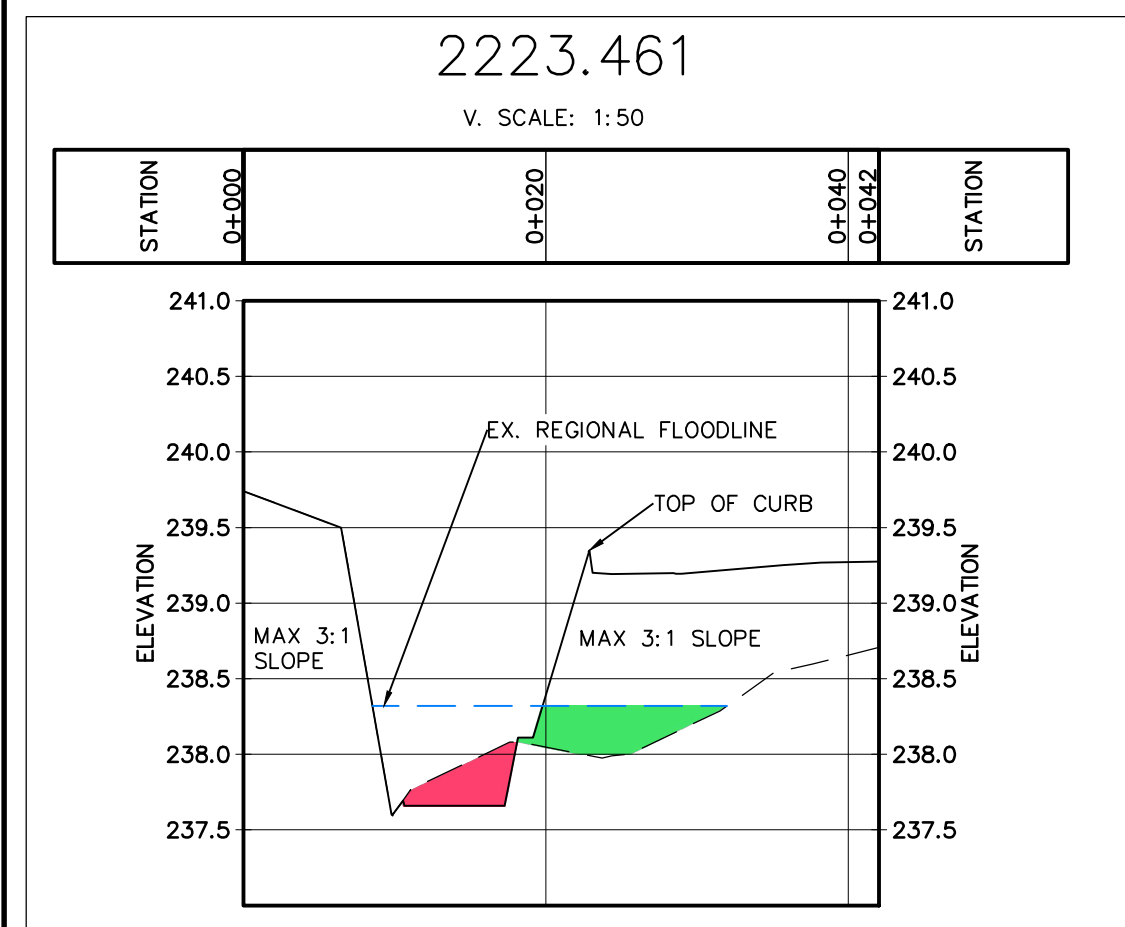
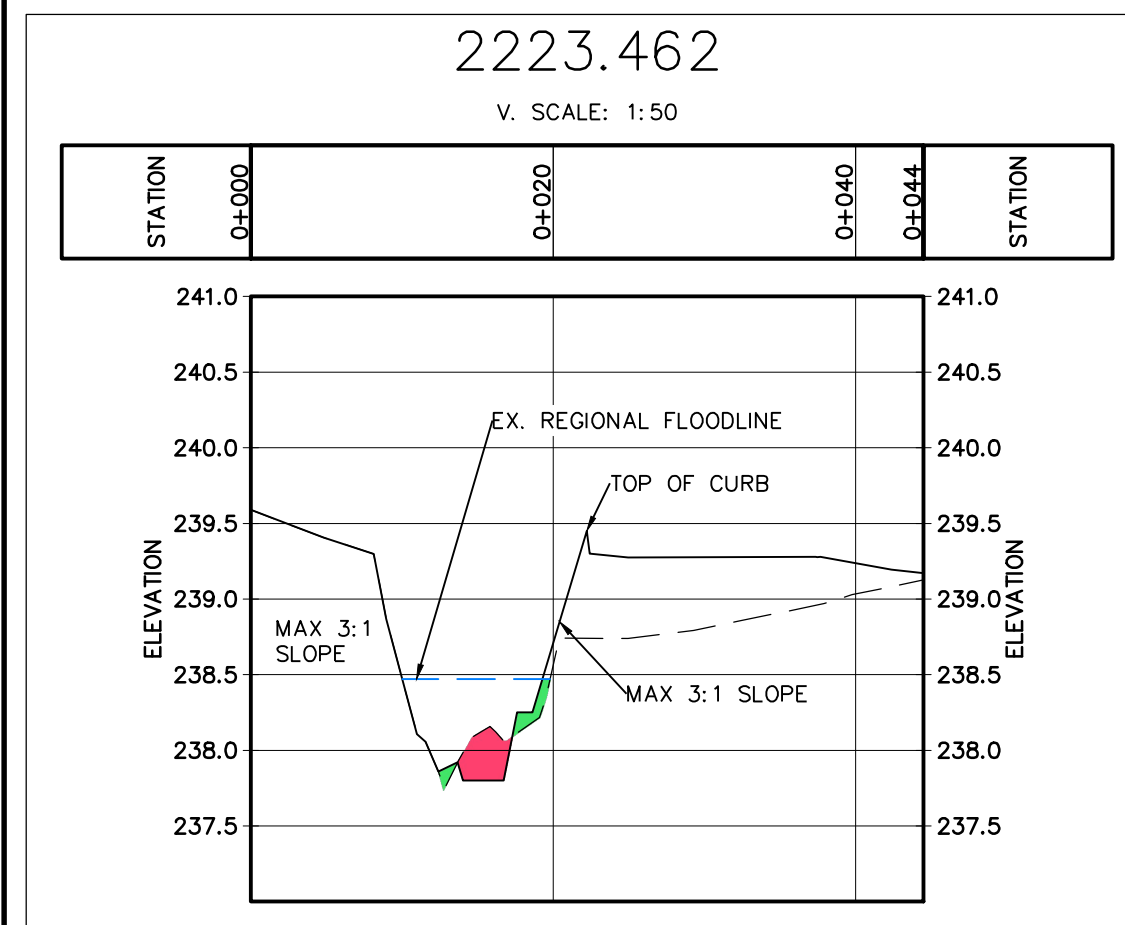
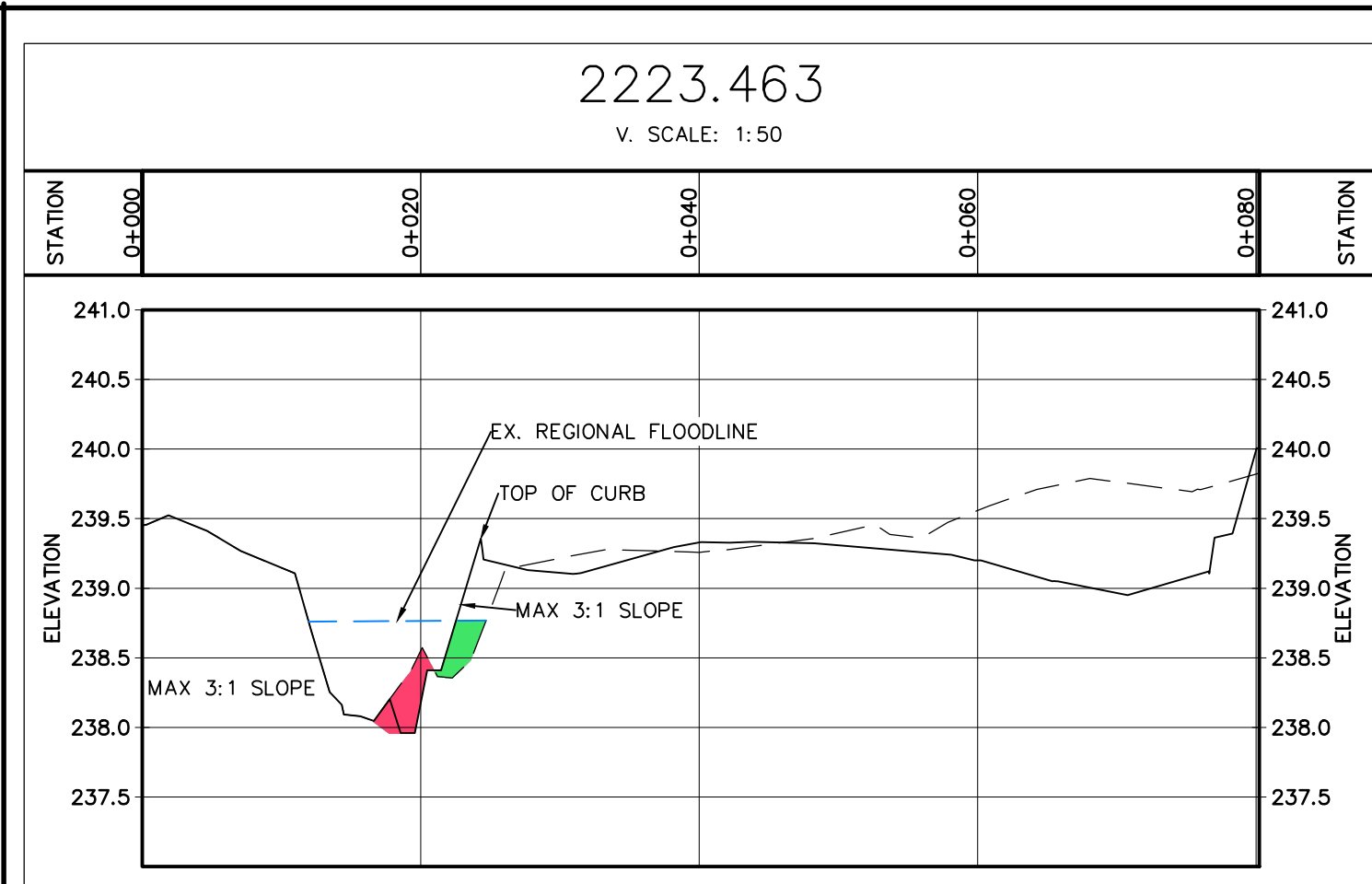
Drawing
**FLOODPLAIN MAPPING – WITHOUT
CULVERT**

CROZIER
CONSULTING ENGINEERS

211 YONGE STREET
SUITE 301
TORONTO, ON M5B 1M4
416-477-3392 T
WWW.CFCROZIER.CA

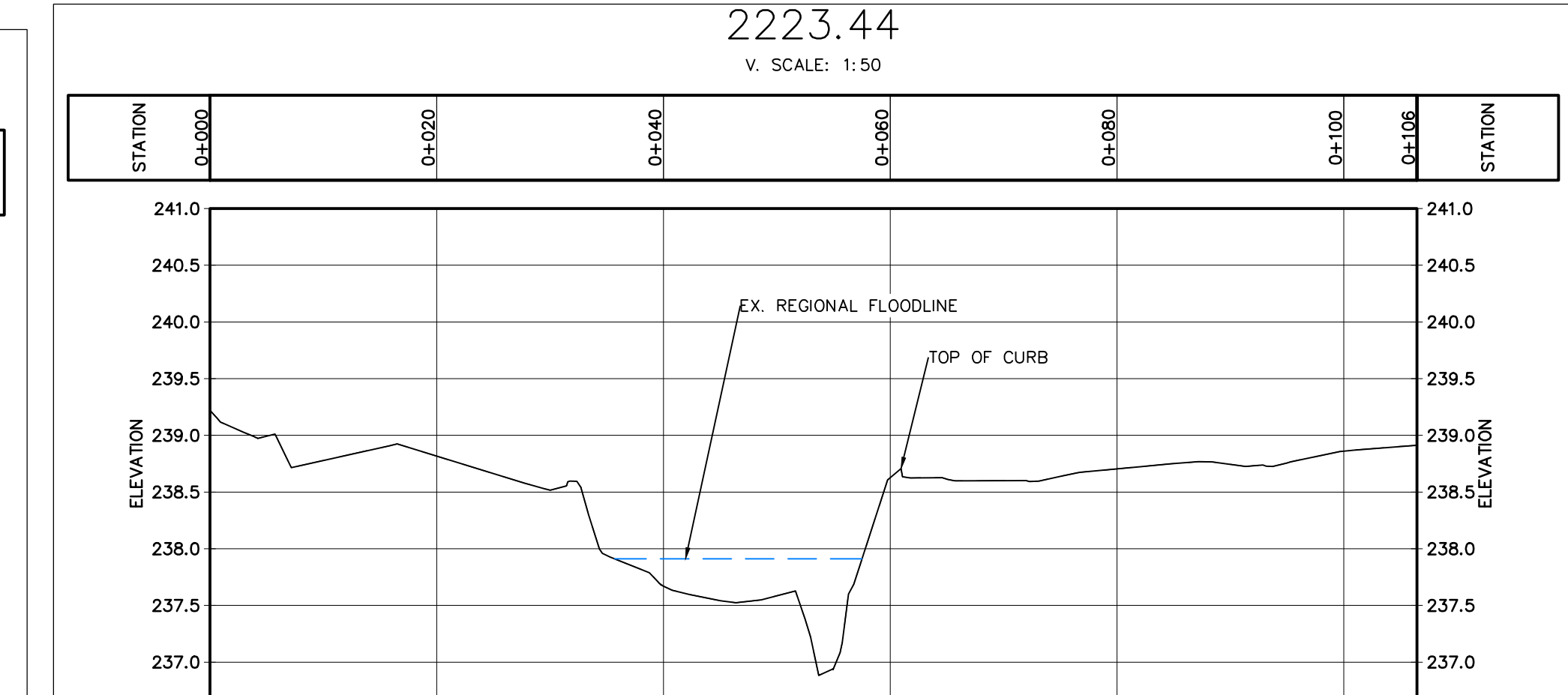
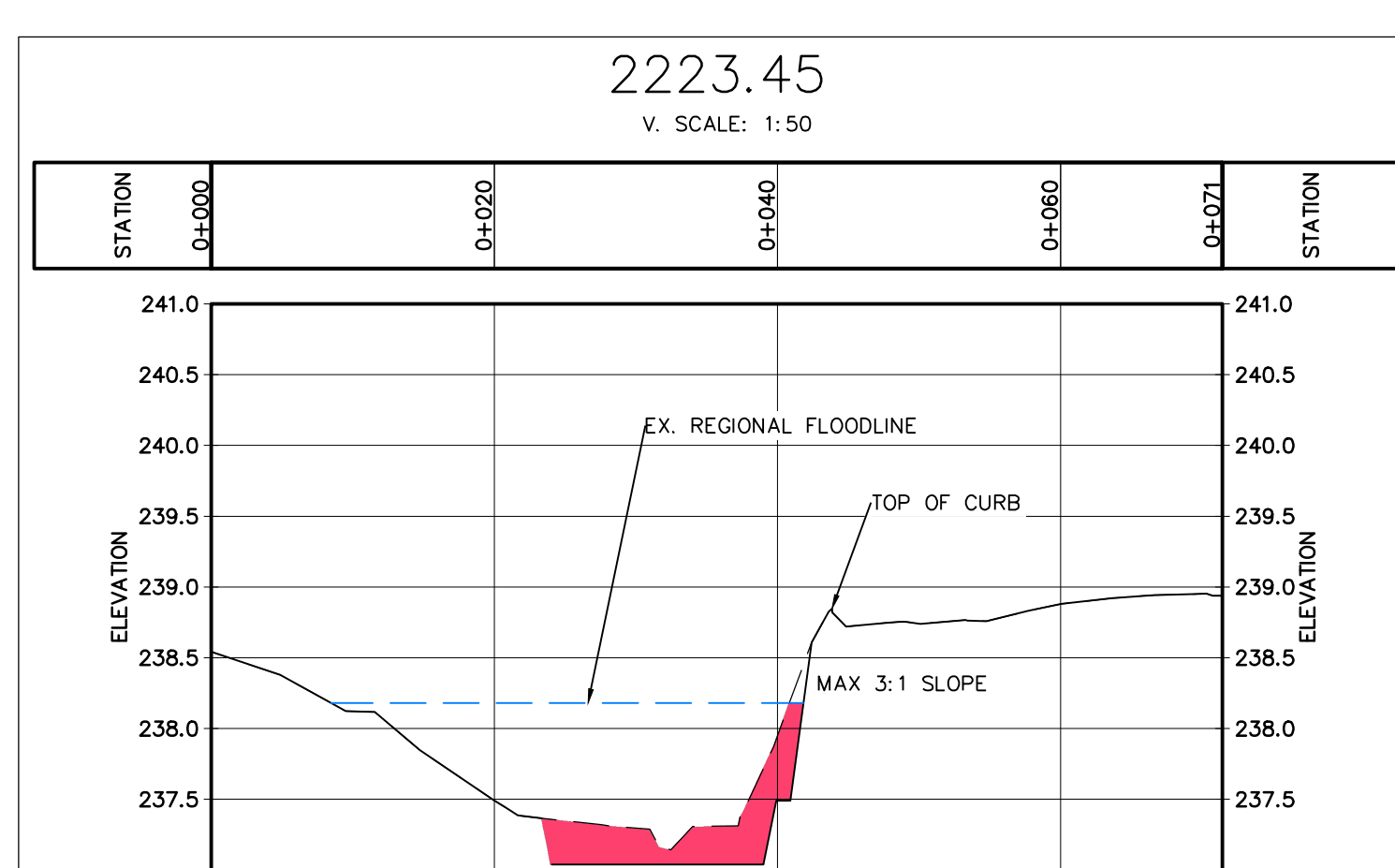
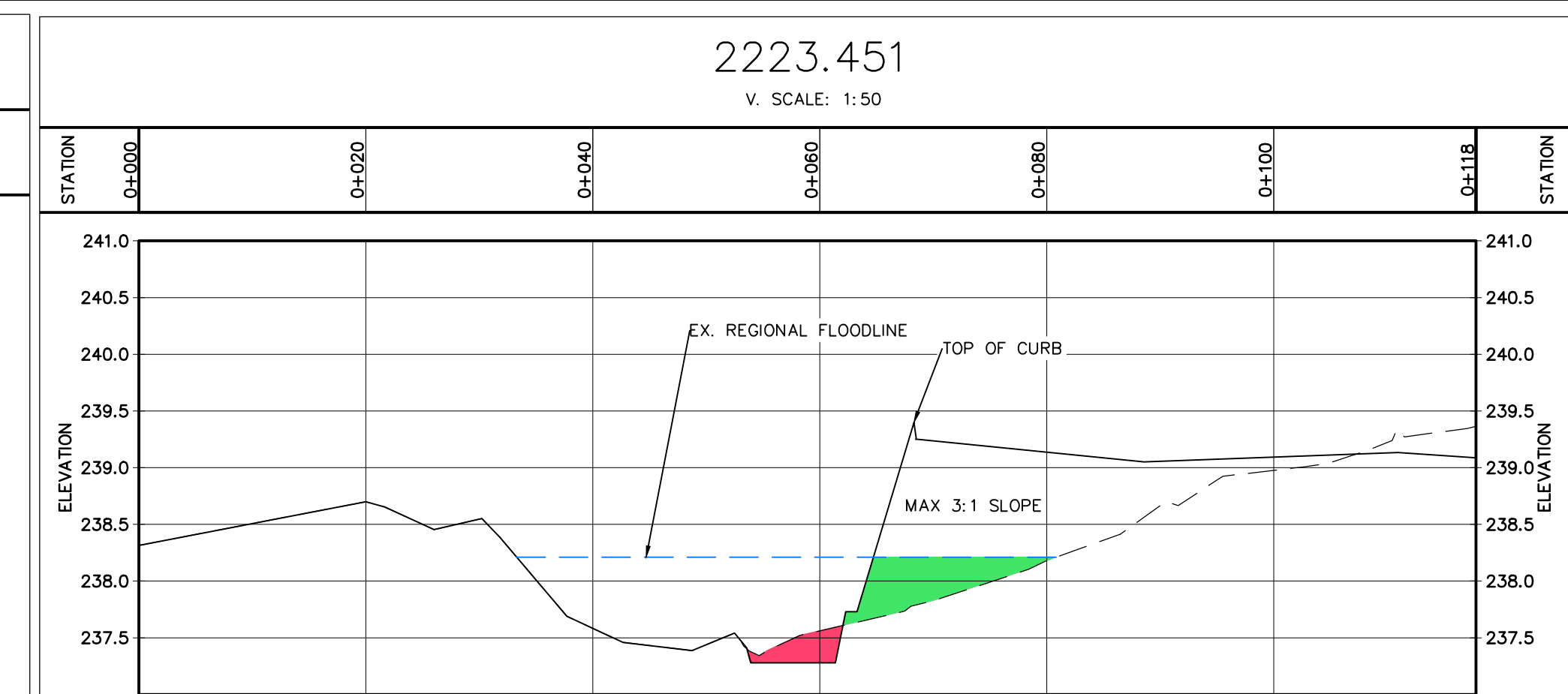
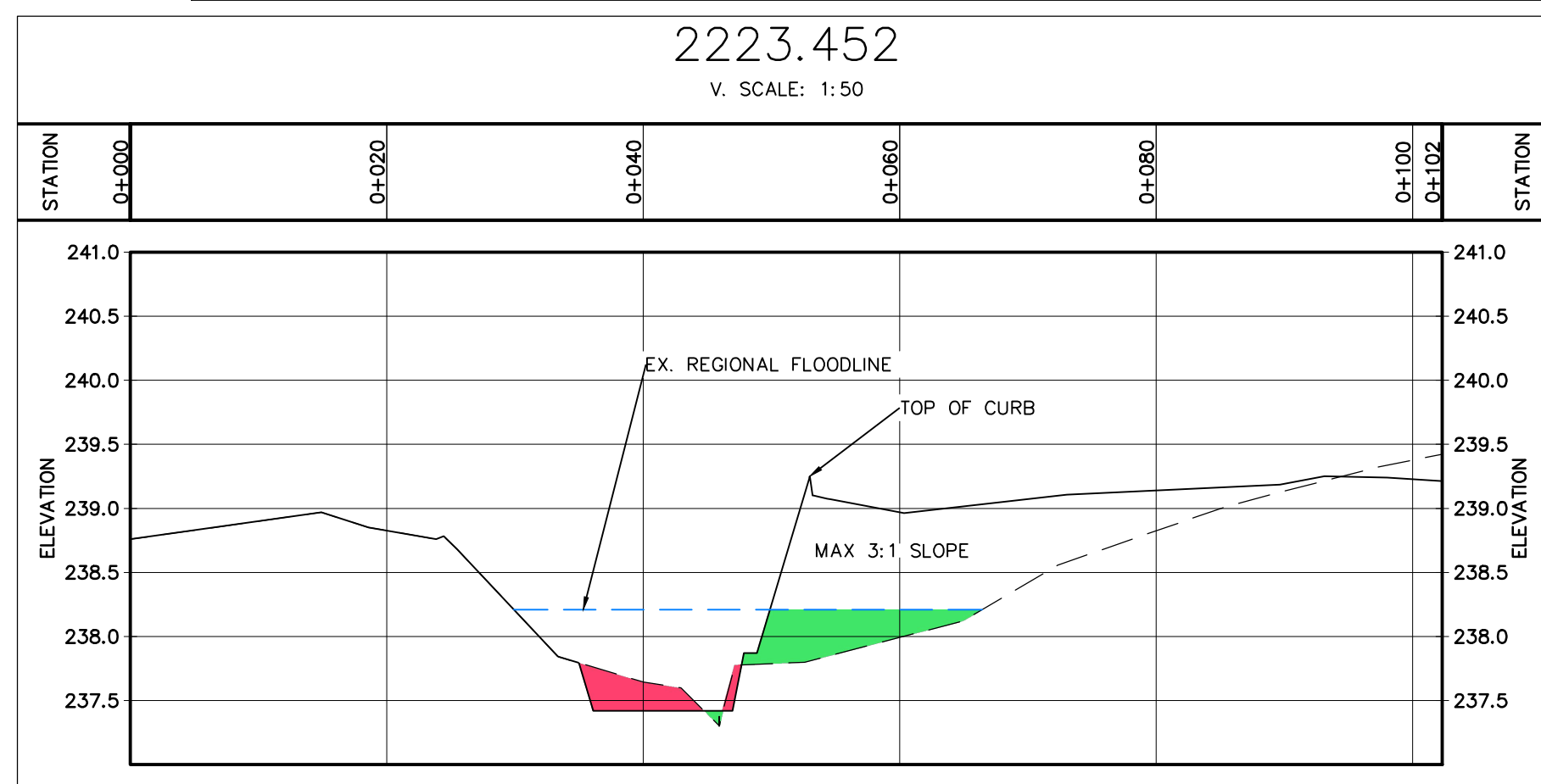
Drawn	B.L.	Design	I.C.	Project No.	1716-5554
Check	R.A.	Check	D.W.	Scale	1:300
				Dwg.	FIG 3.

N:\1700\1716 - 8VD Petroleum Inc\5554 - 12476 Hwy 50\CAD\Civil_Sheets\5554_CTDwg_027\2020 12.16.20 PM.DWG to PDF.pc3



LEGEND

- PROPERTY LINE
- - - EXISTING CONTOUR (0.5m)
- - - EXISTING CONTOUR (1.0m)
- PROPOSED WATERCOURSE
- PROPOSED 3:1 GRADING
- CROSS-SECTION
- XS ID
- CROSS-SECTION I.D.
- █ PROPOSED CUT AREA
- █ PROPOSED FILL AREA
- EXISTING PROFILE
- PROPOSED PROFILE



A	ISSUED FOR SUBMISSION	2020/AUG/24
No.	ISSUE / REVISION	YYYY/MMM/DD

ELEVATION NOTE:
ELEVATIONS SHOWN ON THIS PLAN ARE DERIVED FROM THE MINISTRY OF TRANSPORTATION BENCHMARK No. 758056
ELEVATION = 000.000m

LOCAL BENCHMARK:
TM1 = CUT CROSS IN SIDEWALK
ELEVATION = 239.58m

TM1 = CUT CROSS IN SIDEWALK
ELEVATION = 239.59m

SURVEY NOTES:
PRELIMINARY SURVEY COMPLETED BY VAN HARTEN SURVEYING INC. (2020/MAY/06)
BEARINGS ARE UTM GRID, DERIVED FROM RTN OBSERVATIONS
UTM ZONE 17, NAD83 (GRS) (2010.0)
DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.9996781

SITE PLAN NOTES:
DESIGN ELEMENTS ARE BASED ON SITE PLAN BY ANTRIX ARCHITECTS INC.

DRAWING NOTES:
THIS DRAWING IS THE EXCLUSIVE PROPERTY OF C.F. CROZIER & ASSOCIATES INC. AND THE REPRODUCTION OF ANY PART OF IT WITHOUT PRIOR WRITTEN CONSENT OF THIS OFFICE IS STRICTLY PROHIBITED.
THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, LEVELS, AND DATUMS ON SITE AND REPORT ANY DISCREPANCIES OR OMISSIONS TO THIS OFFICE PRIOR TO CONSTRUCTION.
THIS DRAWING IS TO BE READ AND UNDERSTOOD IN CONJUNCTION WITH ALL OTHER PLANS AND DOCUMENTS APPLICABLE TO THIS PROJECT. DO NOT SCALE THIS DRAWING.
ALL EXISTING UNDERGROUND UTILITIES TO BE VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO CONSTRUCTION.

Stamp	Stamp
-------	-------

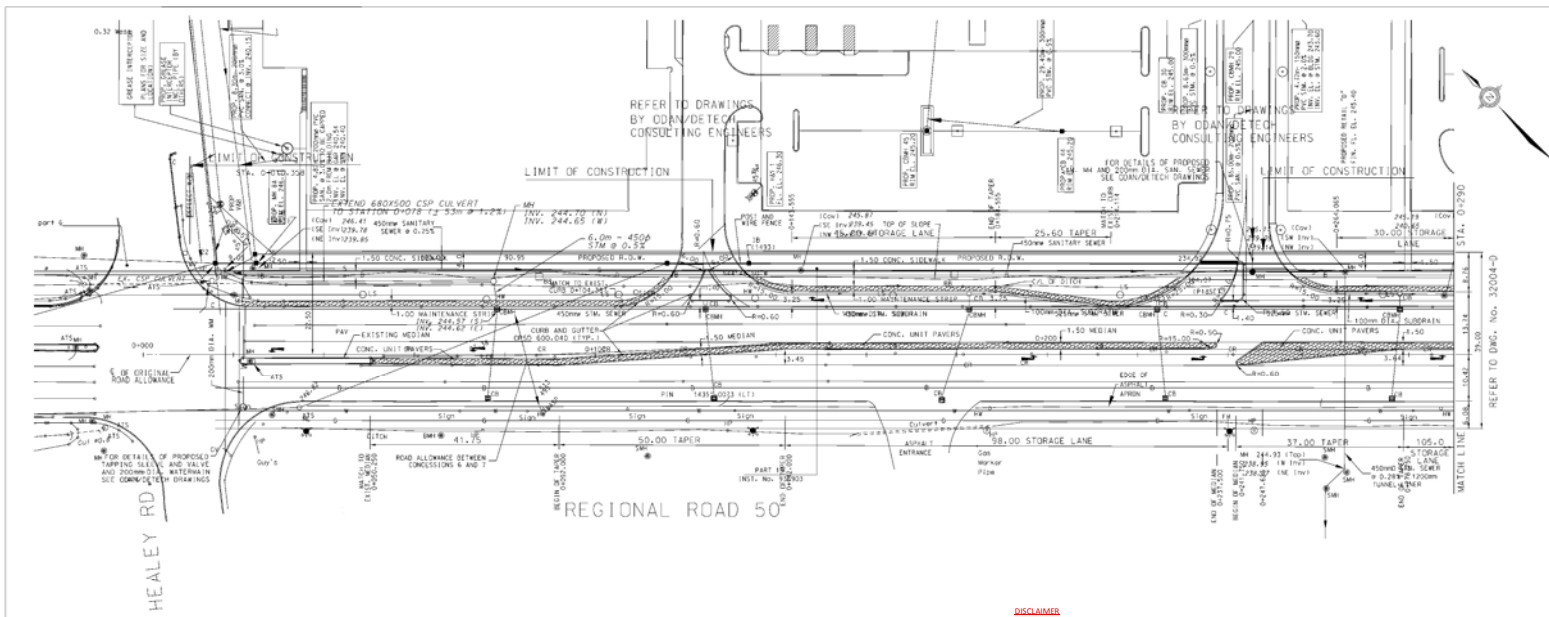
Project
12476 HWY 50
TOWN OF BOLTON, ON

Drawing
CUT/FILL MAPPING

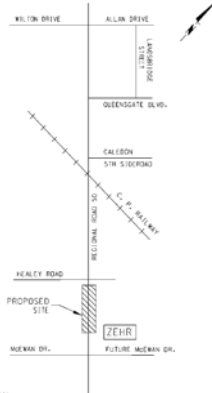
CROZIER
CONSULTING ENGINEERS

211 YONGE STREET
SUITE 301
TORONTO, ON M5B 1M4
416-477-3392 T
WWW.CFCROZIER.CA

Drawn	B.L.	Design	I.C.	Project No.	1716-5554
Check	R.A.	Check	D.W.	Scale	1:500
				Dwg.	FIG 4.



REVISIONS			
DATE	AS CONTRACTED	DETAILS	INTL
FEB. 07/05			



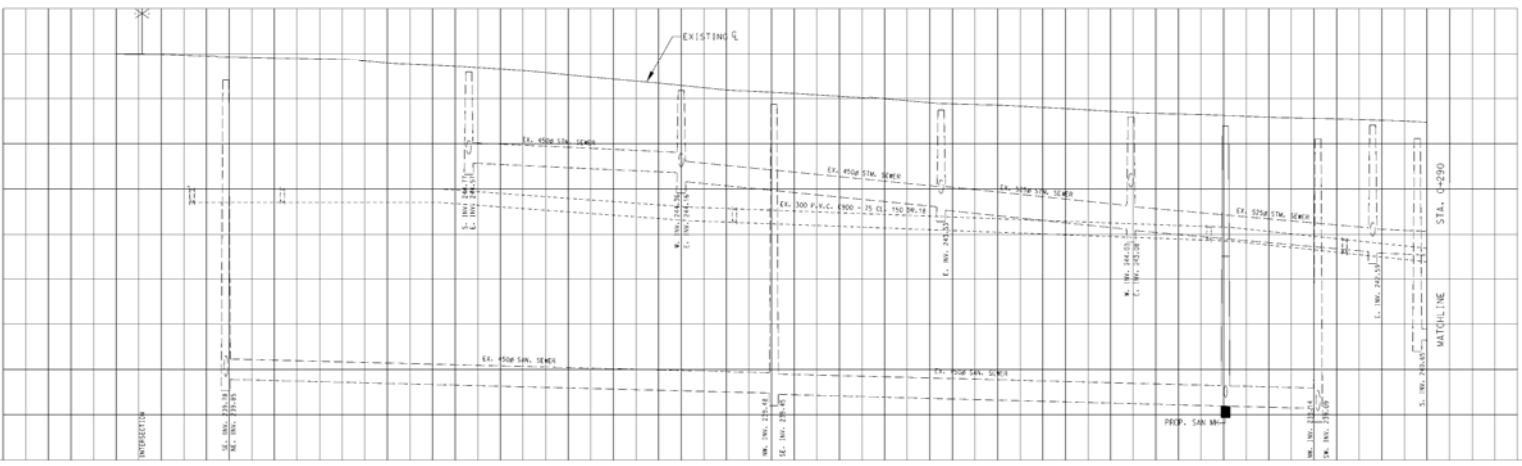
KEY PLAN
NOT TO SCALE

GENERAL NOTES:
 THE POSITION OF POLE LINES, CONCRETE, WATERMAIN, SEWER AND UNDERGROUND AND ABOVE GROUND UTILITIES IS NOT NECESSARILY SHOWN ON THE EXISTING PLANS. THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO SUCH UTILITIES AND STRUCTURES.
 EXISTING TOPOGRAPHICAL INFORMATION SUPPLIED BY SPEIGHT, VAN NOSTRAND & GIBSON LTD.
 BENCHMARK DATA DERIVED FROM INFORMATION FROM SPEIGHT, VAN NOSTRAND & GIBSON LTD.
 CONTRACTOR TO PROVIDE AS-BUILT DRAWINGS UPON COMPLETION OF ALL WORK TO THE ENGINEER.

LEGEND:

DISCLAIMER
 These records are based upon available and unverified information and may prove inaccurate. The Region of Peel disclaims any responsibility should these records be relied upon to the detriment of any person.

NOTE:
 SIDEWALK RAMPS TO BE PROVIDED AT ENTRANCES.

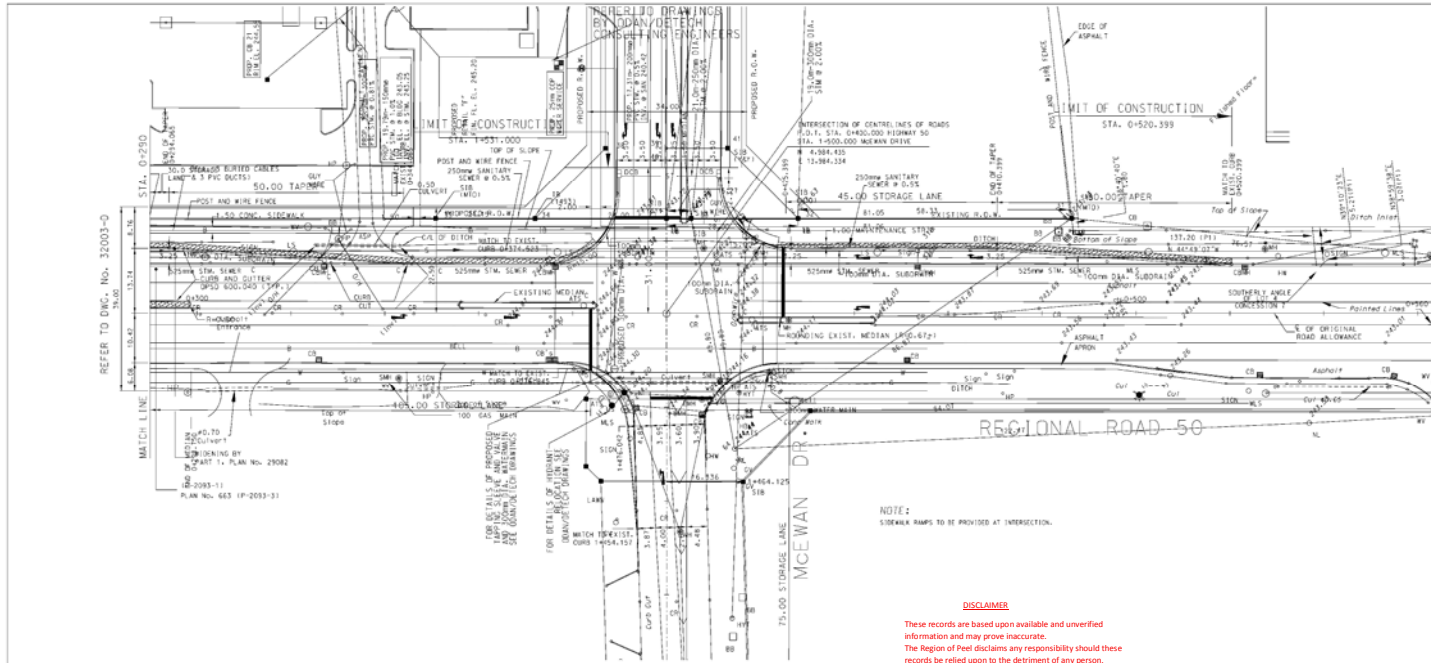


STATIONING	VERTICAL ELEVATION
0+000	246.914
0+020	246.920
0+040	246.878
0+060	246.782
0+080	246.680
0+100	246.522
0+120	246.500
0+140	246.160
0+160	246.046
0+180	246.595
0+200	246.828
0+220	246.717
0+240	246.645
0+260	246.510
0+280	246.517

SCALE	DATE	SHEET	PROJECT NO.
1:1000	OCT. 24, 2002	2 OF 3	2194

REGIONAL ROAD 50
 PLAN AND PROFILE
 STA. 0+000 TO STA. 0+290
 DRAWING NO. 32003-D

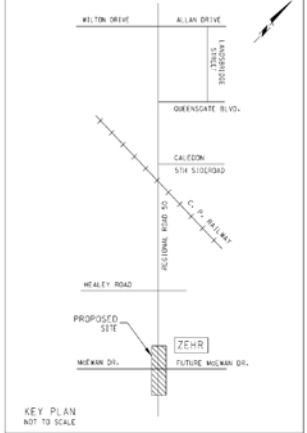




NOTE:
SIDEWALK RAMPS TO BE PROVIDED AT INTERSECTION.

DISCLAIMER
These records are based upon available and unverified information and may prove inaccurate. The Region of Peel declines any responsibility should these records be relied upon to the detriment of any person.

REVISIONS			
DATE	BY	DESCRIPTION	WT.
FEB. 07/05	AS CONSTRUCTED	DETAILS	



KEY PLAN
NOT TO SCALE

GENERAL NOTES:
THE POSITION OF POLE LINES, CONDUITS, WATERMANS, SEWERS AND UNDERGROUND AND ABOVE GROUND UTILITIES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING THE WORK, THE CONTRACTOR SHALL INFORM HIMSELF OF THE EXACT LOCATION OF ALL UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

EXISTING TOPOGRAPHICAL INFORMATION SUPPLIED BY SPEIGHT, VAN NOSTRAND & EISSON LTD.
NEIGHBORLY DATA DERIVED FROM INFORMATION FROM SPEIGHT, VAN NOSTRAND & EISSON LTD.
CONTRACTOR TO PROVIDE AS-BUILT DRAWINGS UPON COMPLETION OF ALL WORKS TO THE ENGINEER.

LEGEND:

BENCH MARK No. 758056

SEARCHED BRIDGE CARRYING HWY. NO OVER C.P.R., 2.0 m SOUTH OF JETTY OF HWY. 50 AND KING ST. IN THE TOWN OF CALTON ROAD DISTRICT AND 0.4 m SOUTH OF SOUTH JETTY OF HWY. 50 AND HEALEY RD.

TABLE IS SET HORIZONTALLY ON THE FACE OF FOOTING OF PIER COLUMN (END S.E. SOUTH OF TRACKS), 1.4 m BELOW TOP OF FOOTING, 1.8 m S.E. OF N.W. CORNER OF HIGH CROSSING LEVEL AND 4.2 m EAST OF CENTERLINE OF HWY. 50. HAVING A FINISHED ELEVATION OF 251.263 METERS.

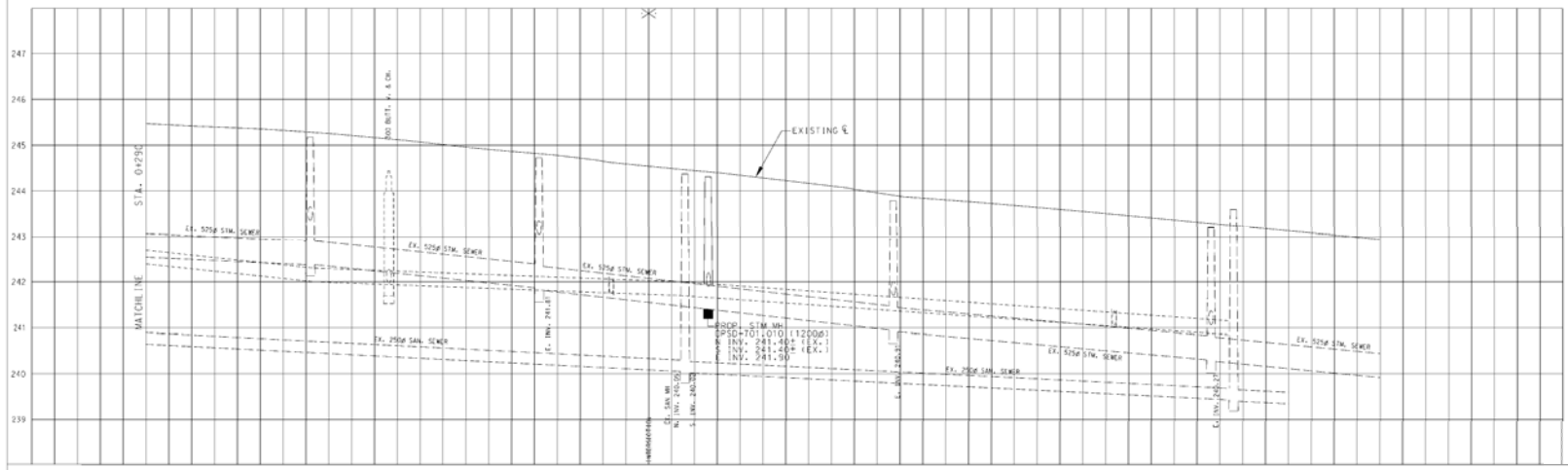
225 Dufferin Drive, Suite 300
Markham, Ontario
L3R 9W4
Tel: (905) 479-8075, Fax: (905) 479-8070

24 Scrimshaw Ave.
Markham, Ontario
L3R 9W4
Tel: (905) 479-8075, Fax: (905) 479-8070

Region of Peel
Public Works

REGION FILE: D-50143E

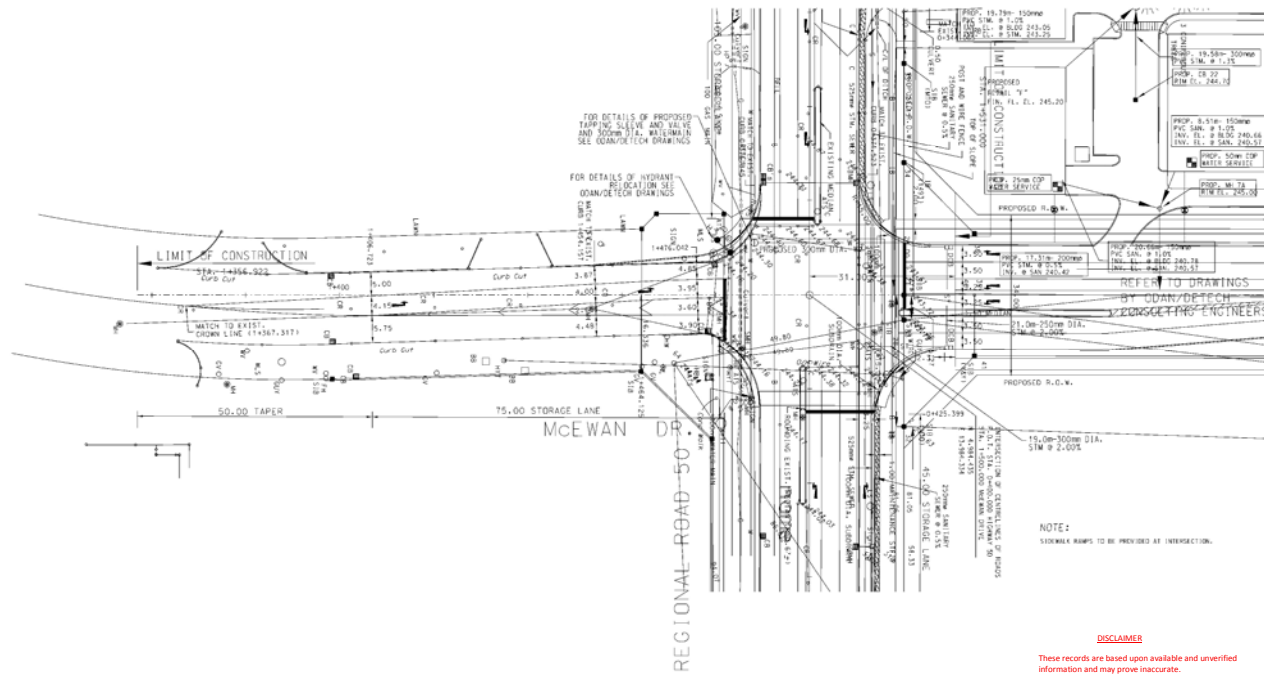
REGIONAL ROAD 50
PLAN AND PROFILE
STA. 0+290 TO STA. 0+560



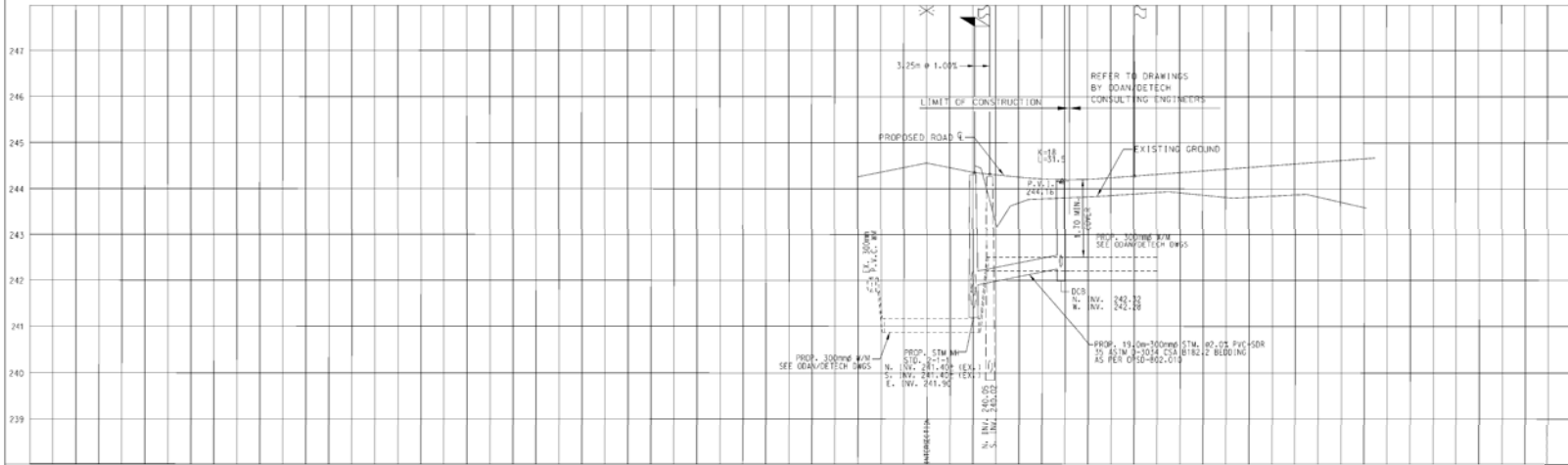
STATIONING	ELEVATION
0+300	245.420
0+350	245.373
0+340	245.165
0+360	244.960
0+380	244.777
0+400	244.511
0+420	244.345
0+440	244.109
0+460	243.883
0+480	243.684
0+500	243.506
0+520	243.317
0+540	243.127
0+560	242.938

SCALE	DATE	PROJECT NO.
VERT. 1:200	OCT. 22, 2000	2194

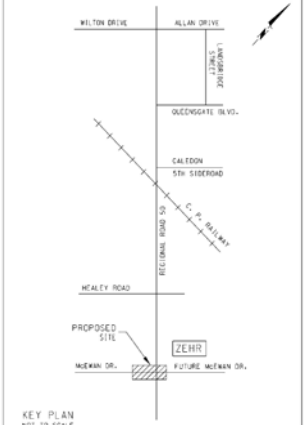
CHECKED BY: [Signature] DATE: OCT. 22, 2000 SHEET: 5 of 5 DRAWING NO.: 32004-0



DISCLAIMER
 These records are based upon available and unverified information and may prove inaccurate.
 The Region of Peel disclaims any responsibility should these records be relied upon to the detriment of any person.

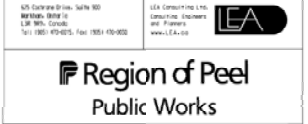


REVISIONS		
DATE	DETAILS	INT.
FEB. 07/05	AS CONSTRUCTED	



GENERAL NOTES:
 THE POSITION OF POLE LINES, CONDUITS, WATERLINES, SEWERS AND UNDERGROUND AND ABOVE GROUND UTILITIES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS AND WHILE CARE HAS BEEN TAKEN TO LOCATE ALL UTILITIES AND STRUCTURES, THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION OF ALL UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.
 EXISTING TOPOGRAPHICAL INFORMATION SUPPLIED BY SPEIGHT, VAN NOSTRAND & GIBSON LTD.
 BOUNDARY DATA DERIVED FROM INFORMATION FROM SPEIGHT, VAN NOSTRAND & GIBSON LTD.
 CONTRACTOR TO PROVIDE AS-BUILT DRAWINGS UPON COMPLETION OF ALL WORKS TO THE ENGINEER.

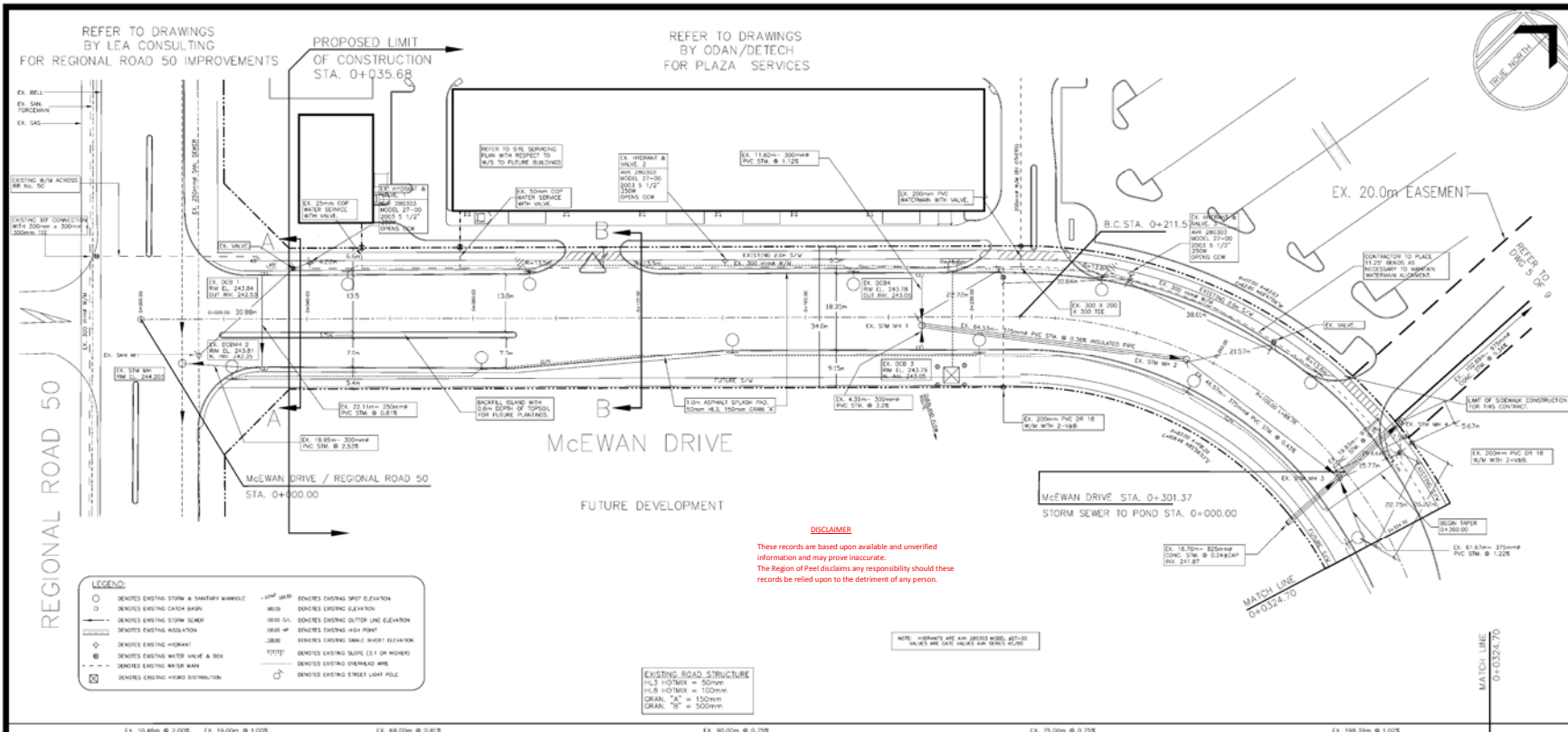
LEGEND:
 BENCH MARK NO. 758056
 CONCRETE BENCH MARK (ELEVATION 242.00) OVER C.P. 11, 2.0 M SOUTH OF JCT OF HWY. 50 AND KING ST. IN THE TOWN OF CALEDON (SEE TOWN AND C.P. 11 NORTH OF SOUTH JCT OF HWY. 50 AND HEALEY RD. TABLE IS SET HORIZONTALLY IN N.E. FACE OF FOOTING OF PIER COLUMN 17ND S-4, SOUTH OF TRACKS, 1.4 M BELOW TOP OF FOOTING, 1.4 M S.E. OF N.W. CORNER, IS ON ABOVE GROUND LEVEL AND 4.1 M EAST OF CENTRELINE OF HWY. 50, HAVING A PUBLISHED ELEVATION OF 257.260 Meters.
 45 Caledon Drive, Suite 800
 Markham, ON M3J 1K7
 Tel: (905) 476-0100 Fax: (905) 476-0100
 www.lea.ca



REGION F ILE: D-50143E
 McEWAN DRIVE
 PLAN AND PROFILE
 STA. 0+000 TO STA. 0+031

DATE	DESIGNED BY	CHECKED BY	PROJECT NO.
DEC. 22, 2002	PK	PK	2194
DRAWING NO.	SHEET	OF	
32005-D	4	9	

CHANG#	DATE/TIME/DESCRIPTION	ELEVATION
		1+450 244.56
		1+450.46 244.25
		1+451.71 244.32
		1+4520 244.27
		1+453+46 244.16
		244.25
		244.39
		244.54



KEY PLAN

Scale: N.T.S.

SUBJECT LANDS

NOTE:

THE POSITION OF POLE LINES, CONDUITS, WATERWAYS, SEWERS AND UNDERGROUND AND ABOVE GROUND UTILITIES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS AND MORE SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXACT LOCATION OF ALL UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

THE CONTRACTOR MUST CHECK AND VERIFY ALL DIMENSIONS ON THE JOB AND REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. ALL DIMENSIONS AND SPECIFICATIONS AND INSTRUMENTS OF SERVICE AND THE PROPERTY OF THE INSURER WHICH MUST BE RETURNED AT THE COMPLETION OF WORK.

THIS DRAWING IS NOT TO BE SCALED

THIS PLAN MUST NOT BE USED TO SITE THE PROPOSED BUILDINGS.

THE APPROVAL OF THIS PLAN DOES NOT EXEMPT THE OWNER'S CONTRACTOR FROM OBTAINING, BUT NOT LIMITED TO THE FOLLOWING PERMITS: ROAD CUT, SEWER PERMITS, REGULATION OF SERVICE, ENFORCEMENT AGREEMENT, APPROACH APPROVAL, PERMITS, ETC.

EXISTING TOPOGRAPHICAL INFORMATION SUPPLIED BY SPECO-T, VAN HOUSTON & OREN LTD.

BOUNDARY DATA DERIVED FROM INFORMATION FROM SPECO-T, VAN HOUSTON & OREN LTD.

NO.	REVISIONS	DATE	BY
10	AS-BUILT PLANS REVISED AS PER REGION OF PEEL COMMENTS	JULY 29/09	F.P.B.
9	AS-BUILT PLANS REVISED AS PER REGION OF PEEL COMMENTS	JUNE 3/09	F.P.B.
8	AS-BUILT CONDITIONS	SEPT 7/05	F.P.B.
7	CURB ALIGNMENT ADJUSTED	NOV 8/03	KO
6	REGION COMMENTS	OCT 16/03	KO
5	AS PER REGION & TOWN COMMENTS	JUN 13/03	KO
4	ISSUED FOR TENDER	APR 23/03	KO
3	AS PER REGION & TOWN COMMENTS	APR 14/03	KO
2	AS PER NEW SITE PLAN	FEB. 19/03	KO
1	AS PER REGION & TOWN COMMENTS	JAN. 22/03	KO

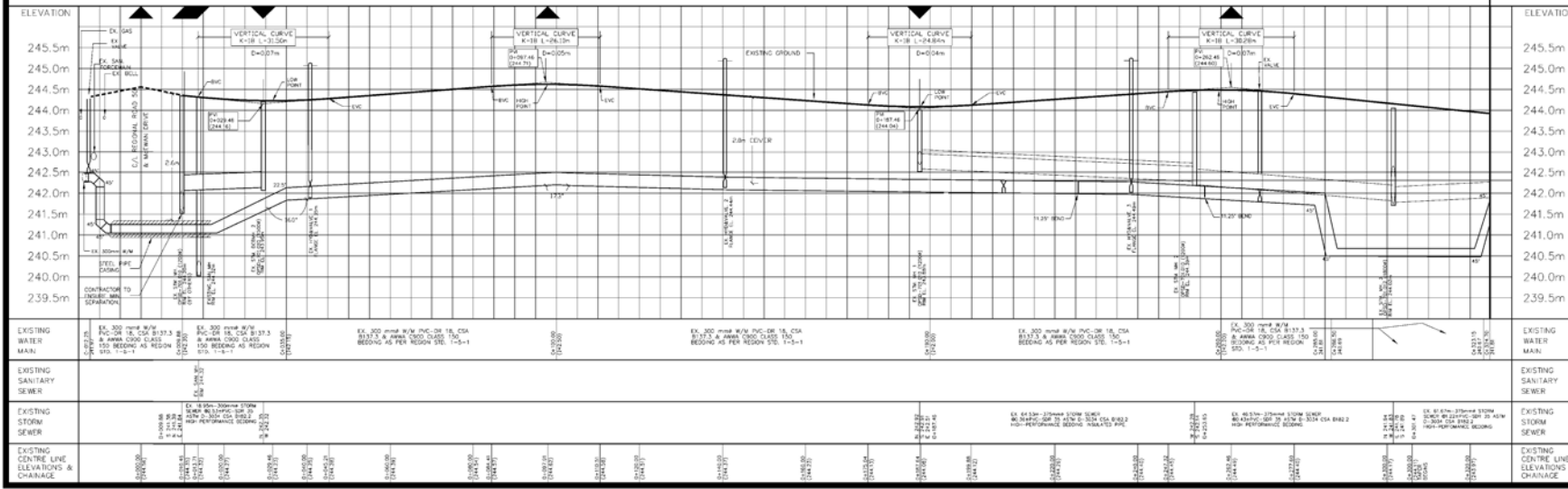
APPROVED FOR CONSTRUCTION

THIS APPROVAL CONSTITUTES A GENERAL REVIEW AND DOES NOT CONSTITUTE A PROFESSIONAL DESIGN OR GUARANTEE.

THIS APPROVAL IS SUBJECT TO THE FURTHER CERTIFICATION OF ME AS CONDUCTED BY WORKS BY A REGISTERED PROFESSIONAL ENGINEER OF THE PROVINCE OF ONTARIO.

DATE: _____ APPROVED BY: _____

H. KRPAN, P. ENG.



BENCH MARK:

ELEVATIONS ARE BASED ON THE CANADIAN GEODETIC DATUM AND WERE DERIVED FROM TOWN OF CALEDON BENCH MARK 79966, HAVING A PUBLISHED ELEVATION OF 241.36m.

METRIC NOTE:

ALL DIMENSIONS AND ELEVATIONS ON THIS PLAN ARE SHOWN IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 3.2808.

REGION OF PEEL

TOWN OF CALEDON

PROJECT: McEWAN DRIVE EXTENSION BOLTON, ONTARIO

CONSULTANT: ODAN/DETECH

THE ODAN/DETECH GROUP - CONSULTING ENGINEERS
5230 SOUTH SERVICE ROAD, BURLINGTON, ONTARIO L7L 5K2
OFF: (905) 632-3611 FAX: (905) 632-3363

REGION FILE No. 0-50143E

SCALE: 1:500 HORIZ 1:50 VERT

DATE: OCTOBER 2002

PROJ. NO. 00260

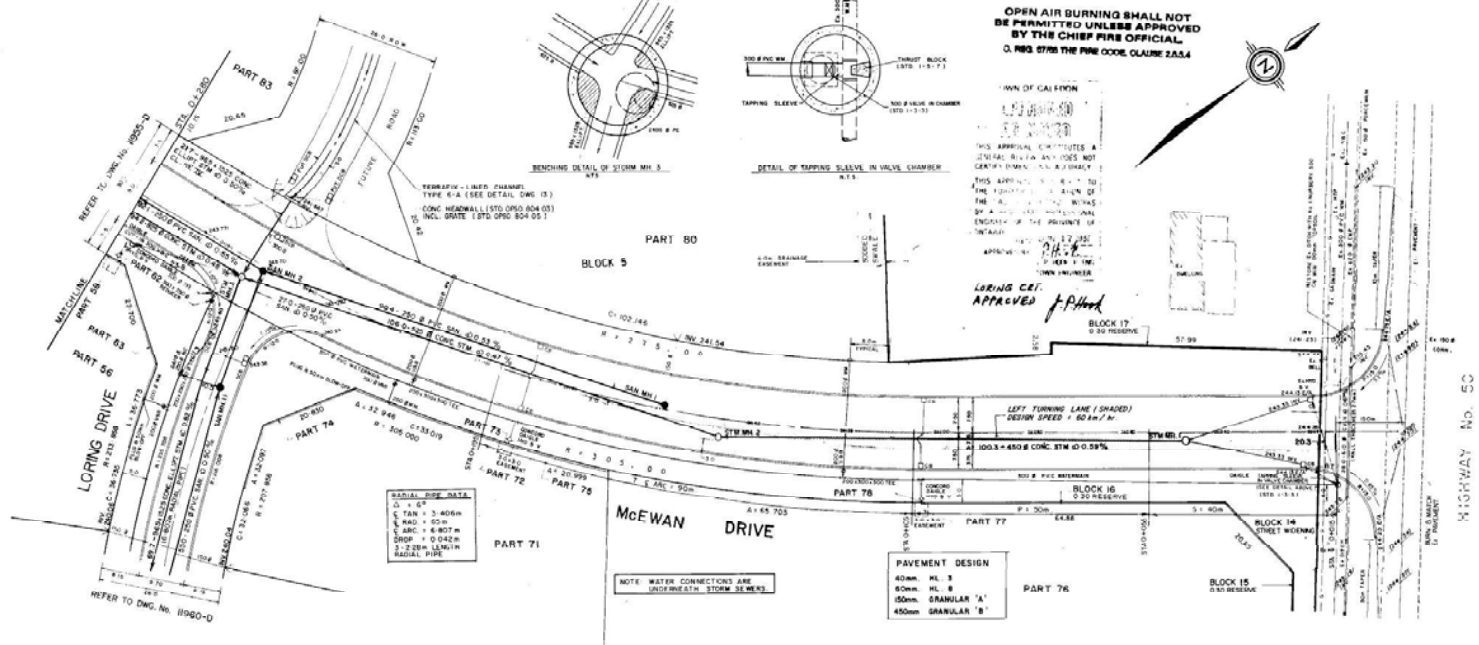
DRAWN BY: K.O. CHECKED BY: D.C.S.

38829-D

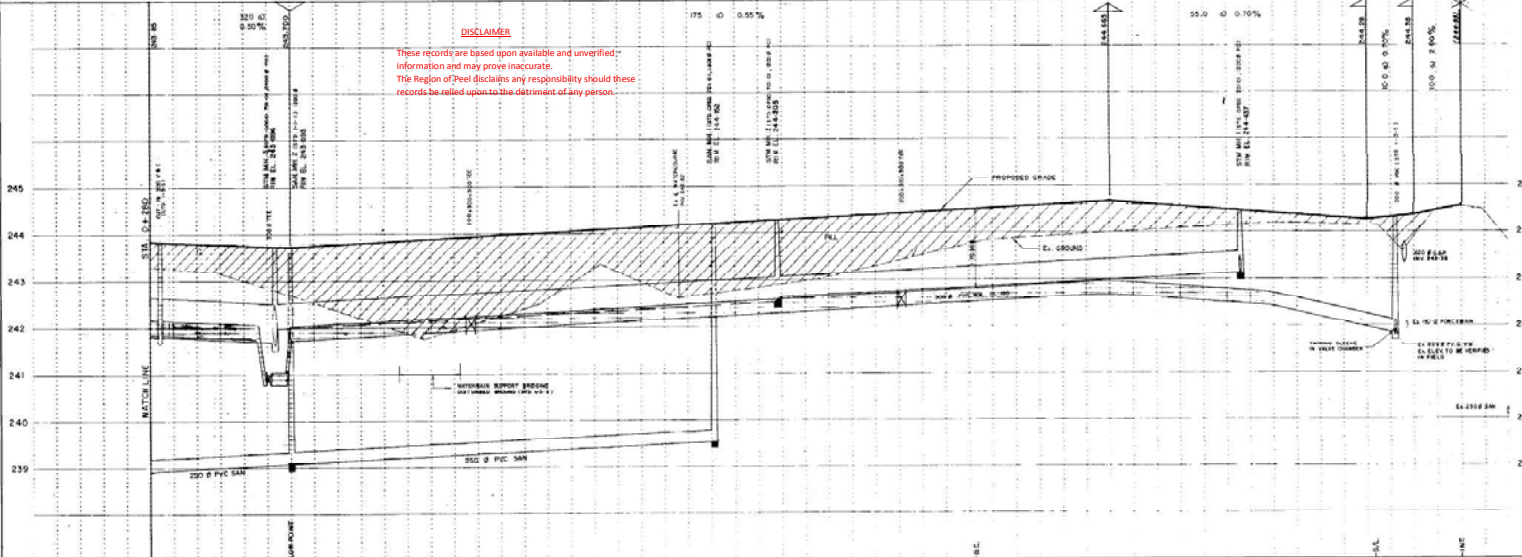
OPEN AIR BURNING SHALL NOT BE PERMITTED UNLESS APPROVED BY THE CHIEF FIRE OFFICER. O. REG. 57/88 THE FIRE CODE, CLAUSE 2.2.4.4



REVISIONS		
DATE	DETAILS	INI
NOV 5/97	ADDED WATER CONNECTIONS	
NOV 5/97	ADDED STREET L.A. AND TYPINGS	
APRIL 16/98	REVISED LOTTING AS PER 43P-1526 JAN 16/98	
MAY 27/98	REVISED THE TYPINGS (DRAWING)	
FEB 20/99	AS CONSTRUCTED STORM AND SANITARY	J.C.
NOV 4/99	AS CONSTRUCTED	J.C.



DISCLAIMER
 These records are based upon available and unverified information and may prove inaccurate. The Region of Peel disclaims any responsibility should these records be relied upon to the detriment of any person.



1. THE ENGINEER HAS NOT INSPECTED THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE AND HAS NOT CONDUCTED ANY SURVEYING OR TESTING.
2. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
3. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
4. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
5. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
6. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
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8. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
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12. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
13. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
14. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
15. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
16. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
17. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
18. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
19. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.
20. THE ENGINEER HAS NOT CONDUCTED ANY INVESTIGATION INTO THE PROPOSED WORK OR THE EXISTING CONDITIONS AT THE SITE.

BENCH MARK
 MET. PIERCE BENCH MARK NO. 728056 ELEVATION 251.263
 TABLE IS SET HORIZONTALLY IN FACE OF PIER OF BRIDGE
 (2ND SET. S. OF TRACK) 14M BELOW TOP OF FOOTING
 1.5M E. OF N.W. CORNER, SECOND LEVEL AND 4.3M
 E. OF E. OF HWY 50, NORTH OF HEALY ROAD.

APPROVED BY: J.C. McEwan, P. Eng.

EMC DWG. NO. 87112-7

EMC GROUP LIMITED
 Consulting Engineers and Project Managers, 4300-10010

BOLTON INDUSTRIAL PARK
 J.N.G. Coast Development Ltd.

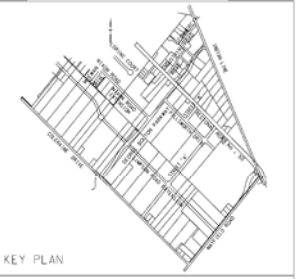
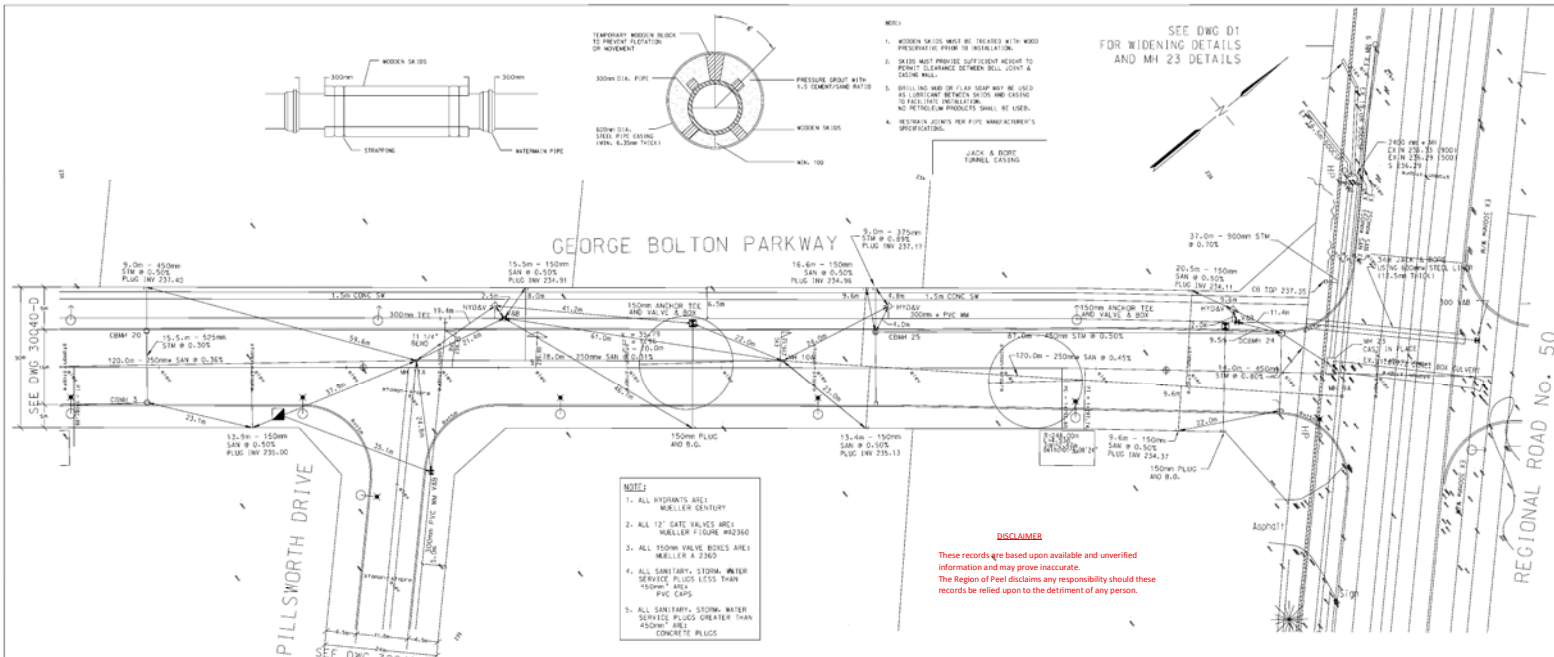
TOWN OF CALEDON
 REGION OF PEEL

217-75292

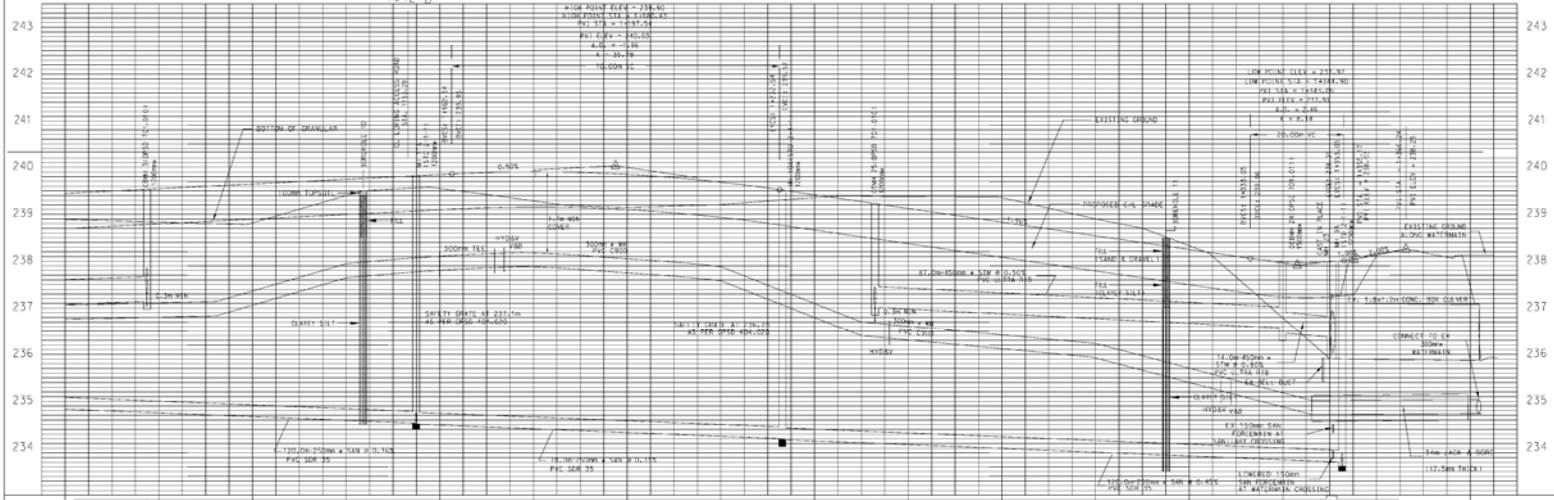
McEWAN DRIVE
 STN 0+000 TO STN 0+280

SCALE: 1" = 100'
 DRAWN BY: J.M./C.B. AREA: []
 CHECKED BY: P.C. PLAN NO: 1954-D
 DATE: []/ []/ [] SHEET: [] OF []

1954-D



- GENERAL NOTES:**
- LEGEND**
- CATCHBASIN
 - ⊙ CATCHBASIN MANHOLE
 - ⊕ DOUBLE CATCHBASIN MANHOLE
 - ⊖ MI 16 STORM MANHOLE
 - ⊖ MI 18A SANITARY MANHOLE
 - ⊖ MI 18B VALVE AND BOX
 - ⊖ MI 18C HYDRANT AND VALVE
 - ⊖ MI 2 BORE-HOLE LOCATION AND NUMBER
 - ⊖ MI 2 HYDRO POLE WITH STREET LIGHT
 - ⊖ GUY WIRE
 - SIGNAL INDICATED SINGLE PHASE TRANSFORMER (SUPPLIED AND INSTALLED BY DEVELOPER)



STORM INVERT	SANITARY INVERT	PROPOSED GRADES	CHAINAGE
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+120
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+160
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+200
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+240
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+280
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+320
M. 237.14 L. 231.22	M. 236.42 L. 230.51	235.42	1+360

BENCHMARK

REGION OF PEEL #37 ELEV. 227.187m
ON THE NORTH FACE AT THE EAST CORNER OF A RED BRICK HOUSE
LOCATED ON THE SOUTH WEST CORNER OF SEVENTEENTH STREET AND
REGION ROAD #161 AND HIGHWAY #56.

BOLTON SOUTH BUSINESS PARK

PROJECT: REGION OF PEEL FILE C02.303

BURNSIDE DEVELOPMENT SERVICES
A DIVISION OF K.J. BURNSIDE AND ASSOCIATES LIMITED
DEVELOPMENT ENGINEERING & ARCHITECTURE
1500 WATSON ROAD, SUITE 201, WILLOW PARK, ONTARIO L2Y 4K6
TEL: (905) 709-7079 FAX: (905) 709-7078

TOWN OF CALEDON

GEORGE BOLTON PARKWAY
PLAN AND PROFILE
STN. 1+080 TO STN. 1+384.63

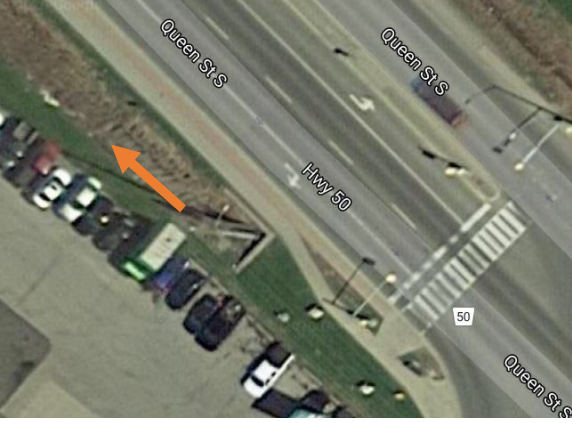
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DRAWN BY: R.A. PROJECT NO.: PB00088 DRAWING NO.: 30041-D
CHECKED BY: DATE: 09/11/20




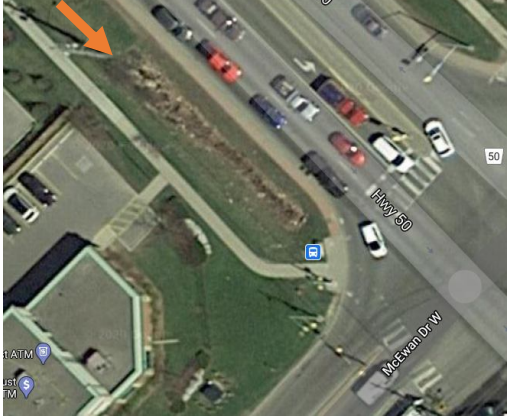
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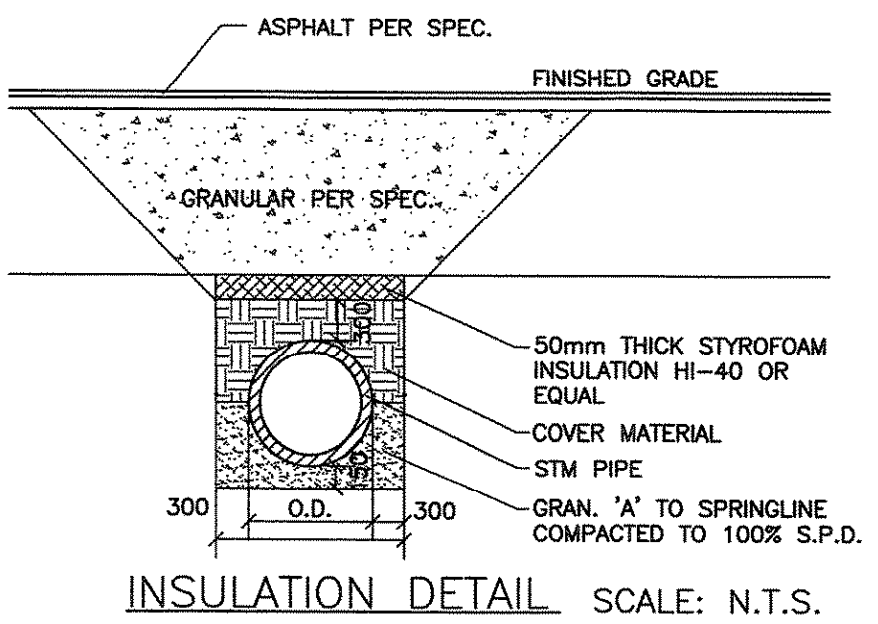
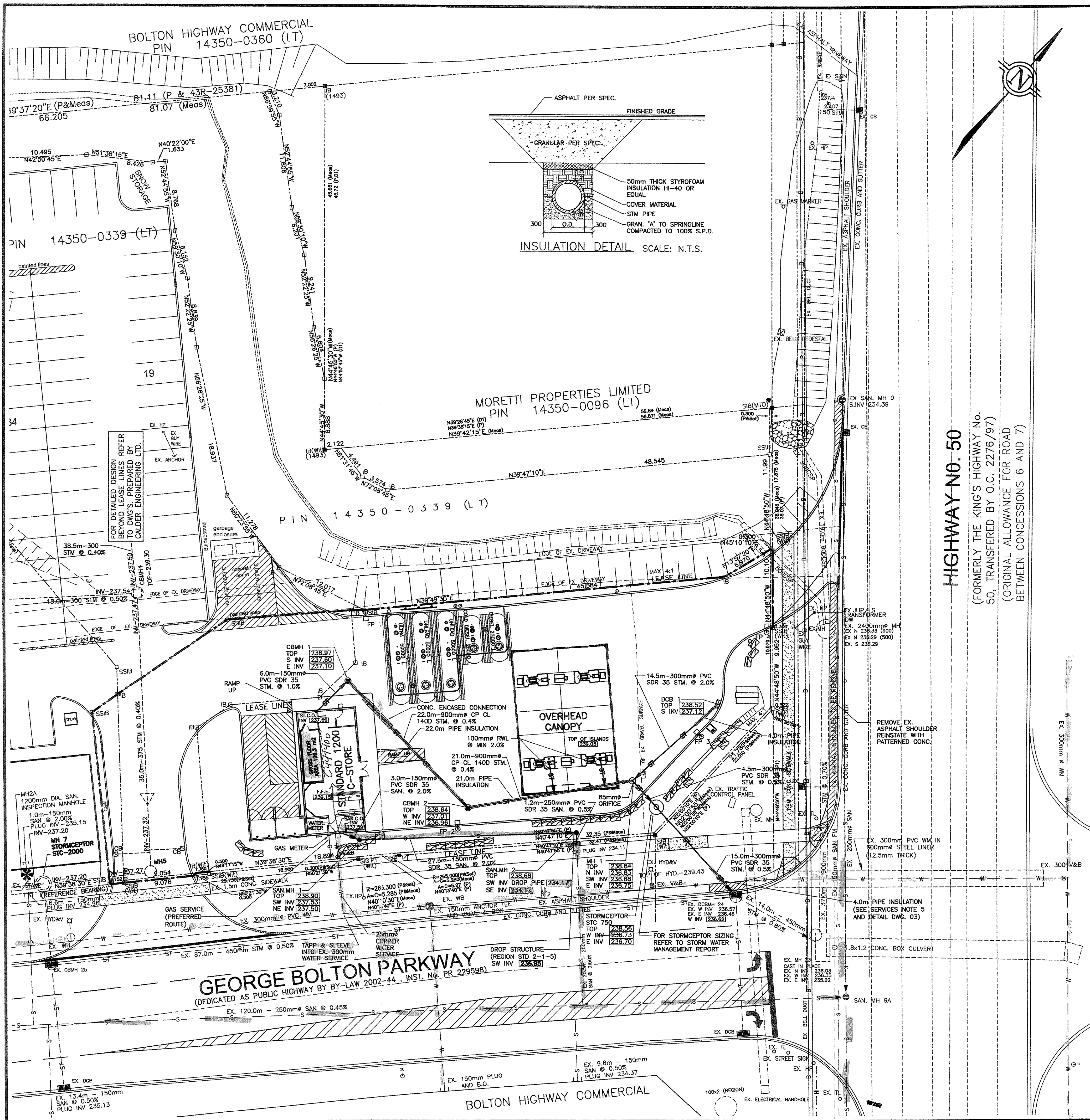
FILE: 30041-D

PRO00088PFS

PHOTOGRAPHS OF UPSTREAM DRAINAGE (June 22, 2020)

Photograph	Location
	
	
	

Photograph	Location
 <p>A photograph showing the exterior of an AGW restaurant. The building has a sign that reads "AGW HOME OF THE BURGER FAMILY". In the foreground, there is a grassy area with a circular manhole cover.</p>	 <p>An aerial satellite view of Highway 50. An orange circle highlights a specific location on the road. The road is labeled "Hwy 50" and "McEwan Dr E".</p>
 <p>A close-up photograph showing debris, including sticks and plastic, floating in a flooded area. A manhole cover is visible in the foreground.</p>	 <p>An aerial satellite view of Highway 50, similar to the one above. An orange arrow points to a specific location on the road. The road is labeled "Hwy 50" and "McEwan Dr W".</p>



HIGHWAY NO. 50
 (FORMERLY THE KING'S HIGHWAY NO. 50, TRANSFERRED BY O.C. 2276/97)
 (ORIGINAL ALLOWANCE FOR ROAD BETWEEN CONCESSIONS 6 AND 7)

STORM DRAINAGE:

- (A) ALL STORM WATER SHALL BE DIRECTED IN SUCH A MANNER THAT WATER WILL NOT ACCUMULATE AT OR NEAR A BUILDING INCLUDING FOUNDATION WALLS, WINDOWS, ENTRANCES AND WALKWAYS.
- (B) WHERE THE EXISTING GRADE IS ALTERED DUE TO GRADING, EXCAVATION, FILLING OR ANY OTHER RELATED WORK ALL SUCH WORK SHALL BE UNDERTAKEN AND COMPLETED IN ACCORDANCE WITH GOOD ENGINEERING PRACTICE TO ENSURE STABLE CONDITIONS AND SO AS NOT TO ADVERSELY AFFECT OR DAMAGE PUBLIC OR PRIVATE PROPERTY.
- (C) STORM DRAINAGE SYSTEM TO CONFORM TO THE STORM WATER MANAGEMENT REPORT PREPARED BY TROW ASSOCIATES INC.

SITE SERVICES NOTES:

- LEGAL INFORMATION TAKEN FROM DRAWINGS PREPARED BY YOUNG & YOUNG SURVEYING INC. (PROJECT No. 00-B3756).
- GENERAL CONTRACTOR TO ENGAGE A HYDRO LOCATE COMPANY TO CONFIRM THE LOCATION OF HIGH VOLTAGE CABLES, PRIOR TO START OF CONSTRUCTION.
- MANHOLES SHALL BE LOCATED A MINIMUM OF 1.5m AWAY FROM THE FACE OF CURB AND/OR ANY OTHER SERVICE.
- WHERE THE DIFFERENCE IN ELEVATION BETWEEN THE INLET AND OUTLET PIPES EXCEED 0.9m, A DROP PIPE AS INDICATED ON REGION OF PEEL STANDARD DWG. 2-1-5 SHALL BE PLACED ON THE INLET PIPE.
- THE DEPTH OF COVER OVER THE PROPOSED STORM & SANITARY SEWERS SHOULD BE CHECKED. WHEREVER THE COVER IS 1.2m OR LESS, IT IS TO BE INSULATED WITH 100mm THICK x 1.2m WIDE INSULATION PLACED INTO TWO (2) LAYERS WITH STAGGERED JOINTS, AND TO BE STYROFOAM BRAND H.I. TYPE IV.

STORM SEWERS:

- ALL STORM SEWERS & CONSTRUCTION METHODS TO BE IN ACCORDANCE WITH CURRENT MUNICIPAL SPECIFICATION.
- STORM SEWERS AND CONNECTIONS 375mm AND SMALLER TO BE PVC SDR 35.
- STORM SEWERS 675mm AND OVER SHALL BE CONCRETE AND EQUAL TO CSA SPECIFICATION 42572 CLASS 500 OR LATEST AMENDMENT, UNLESS NOTED OTHERWISE.
- ALL STORM SEWERS INCLUDING CATCH BASIN LEADS AND SERVICE CONNECTIONS TO BE FITTED WITH RUBBER GASKET JOINTS.
- SEWER BEDDING TO BE IN ACCORDANCE WITH O.P.S.D. 802.03 FOR RIGID PIPE OR O.P.S.D. 802.04 FOR FLEXIBLE PIPE.
- CSMH'S 1 & 2, MANHOLE 1 TO BE CONSTRUCTED IN ACCORDANCE WITH O.P.S.D. 701.012 (1800mm). CBMH'S SHALL HAVE CAST IRON COVER & SQUARE FRAME O.P.S.D. 401.010 TYPE 'B' AND MANHOLE 1 SHALL HAVE CAST IRON COVER & SQUARE FRAME O.P.S.D. 401.010 TYPE 'A'.
- CATCH BASINS TO BE IN ACCORDANCE WITH O.P.S.D. 705.01. TWIN INLET CATCH BASIN TO BE IN ACCORDANCE WITH O.P.S.D. 705.02 WITH FRAME AND GRATE AS PER O.P.S.D. 400.01, UNLESS NOTED OTHERWISE.
- ALL CATCH BASINS AND MANHOLES TO HAVE MINIMUM 300mm SUMP AND TOP AS PER MUNICIPAL STANDARDS.

SANITARY SEWERS:

- PVC SEWER PIPES UNLESS OTHERWISE NOTED MUST MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:
 - CSA 182.1, ASTM D-2729 AND ASTM D-3034 OR LATEST AMENDMENT, CLASS SDR35.
 - CSA B183.4-M90 FOR RIBBED PVC PIPE. (NOTE THAT THE MANUFACTURERS DIRECTIONS FOR INSTALLATION BEDDING AND BACK FILLING MUST BE FOLLOWED)
- MANHOLES TO BE CONSTRUCTED IN ACCORDANCE WITH REGION OF PEEL STD. 2-1-1 (1200mm) WITH RUBBER GASKET JOINTS.
- ALL SANITARY SEWER AND SERVICE CONNECTIONS TO BE FITTED WITH CHEMICALLY RESISTANT JOINTS AS PER MUNICIPAL STANDARDS. SANITARY CONNECTIONS TO BE PER O.P.S.D. 1006.020.
- SEWER BEDDING TO BE IN ACCORDANCE WITH O.P.S.D. 1005. (UNLESS NOTED OTHERWISE).
- SAFETY PLATFORMS TO BE IN ACCORDANCE WITH O.P.S.D. 404.02.
- SANITARY MANHOLES TO HAVE FRAME AND GRATE AS PER O.P.S.D. 401.03.
- MAINLINE AND STREETLINE MANHOLES TO BE IN ACCORDANCE WITH REGION OF PEEL STANDARDS.
- MAX. DROP BETWEEN THE INLET AND OUTLET ELEVATIONS OF SANITARY MANHOLE IS 0.03m. MIN. DEPTH OF SANITARY MANHOLE IS 2.13m.

WATERMANS:

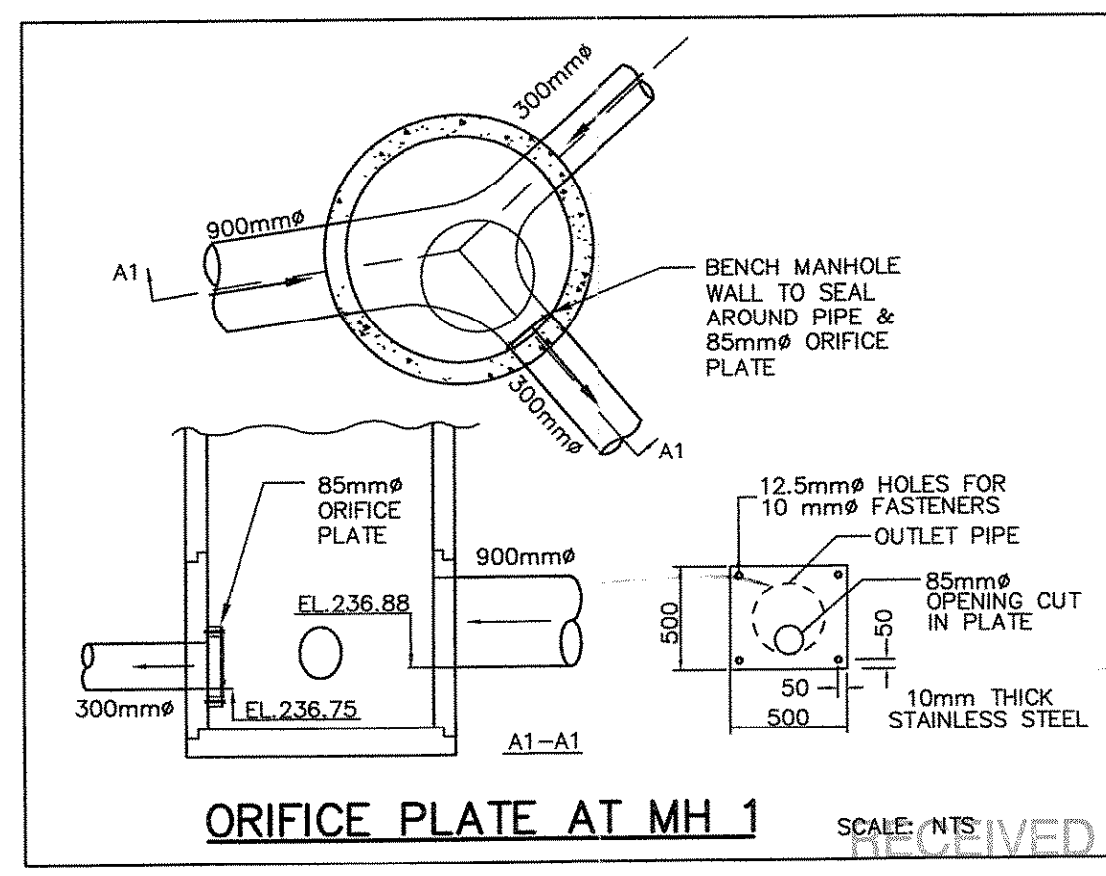
- ALL MATERIAL AND CONSTRUCTION METHODS MUST CORRESPOND TO THE CURRENT PEEL PUBLIC WORKS STANDARDS AND SPECIFICATIONS.
- WATERMAIN AND/OR WATER SERVICE MATERIALS 100mm AND LARGER MUST BE CLASS 150, A.W.W.A. C 900-75 P.V.C. PIPE. SIZE LARGER THAN 500mm TO BE P.V.C. 2306 TUBE SERIES 160 C.S.A. B.137.1 (A.W.W.A. C901). PIPE 50mm AND SMALLER TO BE SOFT COPPER TYPE 'K'.
- WATERMANS AND/OR WATER SERVICES ARE TO HAVE A MINIMUM COVER OF 1.7m WITH A MINIMUM HORIZONTAL SPACING OF 1.2m FROM THEMSELVES AND ALL OTHER UTILITIES.
- PROVISIONS FOR FLUSHING WATER LINES PRIOR TO TESTING, ETC. MUST BE PROVIDED WITH AT LEAST A 50mm OUTLET ON 100mm AND LARGER LINES. COPPER LINES ARE TO HAVE FLUSHING POINTS AT THE END, THE SAME SIZE AS THE LINE. THEY MUST ALSO BE HOSED OR PIPED TO ALLOW THE WATER TO DRAIN ONTO A PARKING LOT OR DOWN A DRAIN. ON FIRE LINES, FLUSHING OUTLET TO BE 100mm MINIMUM OR A HYDRANT.
- ALL CURB STOPS TO BE 3.0m OFF THE FACE OF THE BUILDING UNLESS OTHERWISE NOTED.
- HYDRANT AND VALVE SET TO REGION STANDARD 1-6-1 DIMENSION A AND B, 0.7m AND 0.9m AND TO HAVE PUMPER NOZZLE.
- WATERMANS TO BE INSTALLED TO GRADE AS SHOWN ON APPROVED SITE PLAN. COPY OF GRADE SHEET MUST BE SUPPLIED TO INSPECTOR PRIOR TO COMMENCEMENT OF WORK, WHERE REQUESTED BY INSPECTOR.
- WATERMANS MUST HAVE A MINIMUM VERTICAL CLEARANCE OF 0.3m OVER / 0.5m UNDER SEWERS AND ALL OTHER UTILITIES WHEN CROSSING.
- ALL PROPOSED WATER PIPING MUST BE ISOLATED FROM EXISTING LINES IN ORDER TO ALLOW INDEPENDENT PRESSURE TESTING AND CHLORINATING FROM EXISTING SYSTEMS.
- ALL LIVE TAPPING AND OPERATION OF REGION WATER VALVES SHALL BE ARRANGED THROUGH THE REGIONAL INSPECTOR ASSIGNED OR BY CONTACTING THE OPERATIONS AND MAINTENANCE DIVISION.

TRAFFIC SAFETY AND CONTROL:

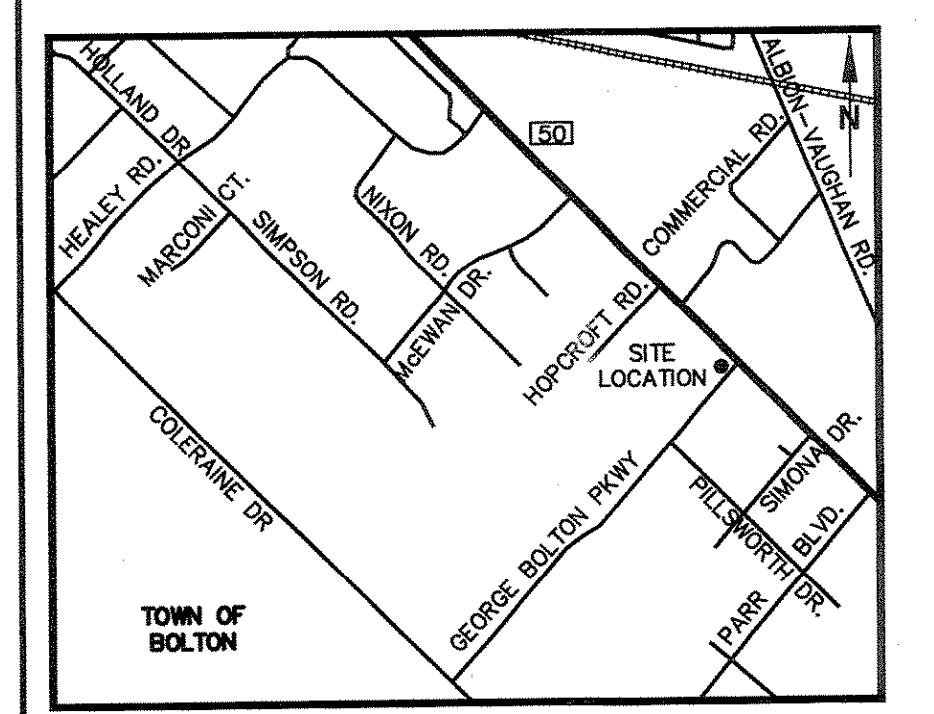
- IT IS THE RESPONSIBILITY OF THE SITE DEVELOPER TO PROVIDE ALL TRAFFIC SAFETY AND CONTROL MEASURES IN ACCORDANCE WITH THE ONTARIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, TEMPORARY CONDITIONS. THE SITE DEVELOPER SHALL MAKE ARRANGEMENTS WITH THE MUNICIPALITY AT LEAST 30 DAYS PRIOR TO COMMENCING WORK ON ANY PUBLIC ROADS.

FIRE DEPARTMENT:

- FIRE ROUTE WILL BE DESIGNATED AS PER MUNICIPAL BY-LAW.
- THE FIRE DEPARTMENT ACCESS ROUTE SHALL BE DESIGNED TO SUPPORT A LOAD OF NOT LESS THAN 11,363 KG PER AXLE AND HAVE A CHANGE IN GRADIENT OF NOT MORE THAN 1 IN 12.5 OVER A MINIMUM DISTANCE OF 15 m.



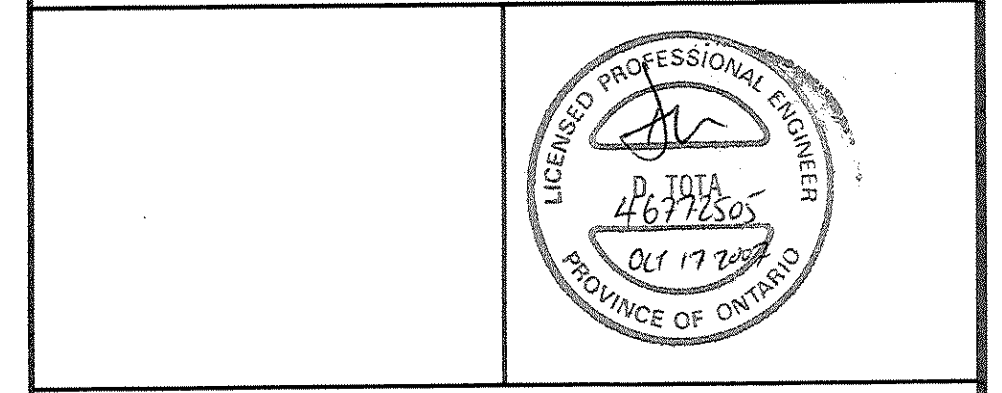
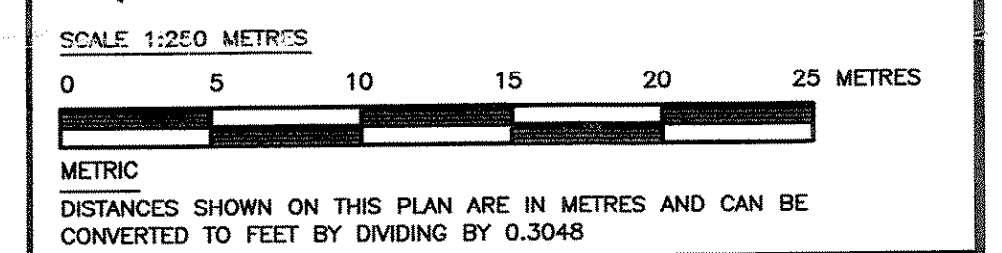
NO.	REVISIONS	DATE	BY	APP.
E	SANITARY SERVICE, WATER SERVICE, SITE SERVICES NOTE 4, WATERMAIN NOTE 8 REVISED, WATERMAIN NOTE 10 AND SANITARY SEWER NOTE 8 ADDED. STORM SERVICE BEYOND LEASE LINE SHOWN.	OCT 17 2007	A.B.	
D	DWG UPDATED TO REFLECT SITE PLAN REV "E".	SEP 10 2007	A.B.	
C	DWG UPDATED TO REFLECT SITE PLAN REV "D".	SEP 05 2007	A.B.	
B	DWG UPDATED TO REFLECT SITE PLAN REV "C". STORM SERVICE REVISED. CITY FILE NUMBER ADDED.	JUL 05 2007	A.B.	
A	ISSUED FOR INFORMATION.	MAR 09 2007	A.B.	



LEGEND:

IB	EX. IRON BAR	ST	EX. STORM SEWER
SIB	EX. STANDARD I.B.	S	EX. SANITARY SEWER
TL	EX. TRAFFIC LIGHT	W	EX. WATER MAINS
HP	EX. HYDRO POLE	C	EX. GAS MAINS
H	EX. HYDRO POLE	OH	EX. OVERHEAD HYDRO
ICV	EX. IRRIGATION CONTROL VALVE	UH	EX. HYDRO SERVICE
WV	EX. WATER VALVE	B	EX. 1/2" BELL SERVICE
HYD	EX. FIRE HYDRANT	EX	EX. FIBER OPTICS LINE
C.O.	NEW CLEAN OUT	ST	NEW STORM SEWER
RW	NEW DOWNSPOUT	S	NEW SANITARY SEWER
BP	EX. BELL PEDESTAL	W	NEW WATER SERVICE
FP	EX. FLOOR POLE	G	NEW GAS SERVICE
ST	EX. STORM M.H.	H	NEW HYDRO SERVICE
SM	EX. STORM M.H.	B	NEW BELL SERVICE
CS	EX. SANITARY M.H.	CB	EX. CONC. CURB
CSM	EX. SANITARY M.H.	CC	NEW CONC. CURB
CSN	EX. CATCH BASIN	CCB	NEW DEPRESSIONED CURB
CSNB	NEW CATCH BASIN	CCB	NEW CURB CUT
CSNB	NEW CATCH BASIN M.H.	+	EX. ELEVATION
CSNB	NEW SLOPE	+ 92.53	EX. ELEVATION TO REMAIN
		+ 92.53	ELEVATION (PROPOSED)
		+ 92.73	PROPOSED ELEVATION (BY OTHERS)

MAJOR OVERLAND FLOW DIRECTION



OWNER: **Farview Holdings**
 12599 Hwy. 50, Unit 7
 Bolton, ON, L7E 1M4
 Tel: (416) 420-7709
 Fax: (905) 893-3100

APPLICANT/CLIENT: **SUNCOR ENERGY**
 SUNCOR ENERGY PRODUCTS INC. 36 YORK MILLS RD., TORONTO, ON, M2P 2E5
 TEL: (416) 733-7224, FAX: (416) 733-2113

Trow Associates Inc.
 1595 Clark Boulevard
 Brampton, Ontario L6T 4V1
 Tel: (905) 793-9800
 Fax: (905) 793-0841

LOCATION: **12476 REGIONAL ROAD 50
 @ GEORGE BOLTON PKWY
 TOWN OF CALEDON, ONTARIO**

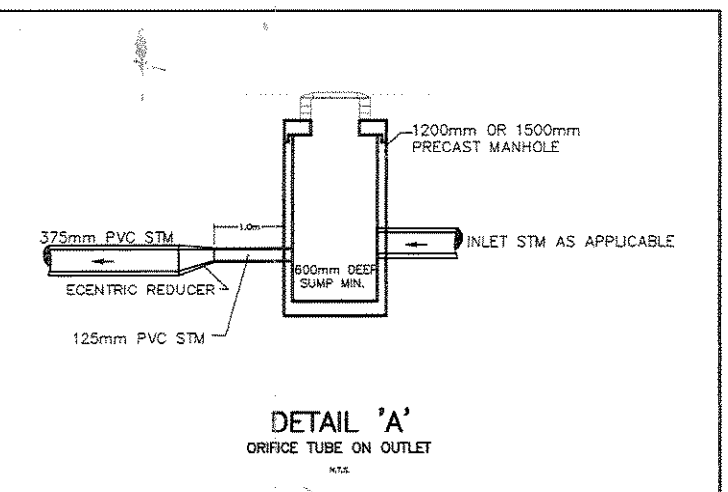
TITLE: **SITE SERVICES PLAN**
 SPA07-021

DESIGNED: A.B.	DRAWN: A.B.	S/S NO.:	D. No.:
CHECKED: B.H. REICHERT	DATE: FEB. 2007		
SCALE: 1:250	PROJ. NO.:	DWG. NO.:	
CAD FILE: 23611	DATE: MAR 18 2008		

MUNICIPAL SITE & REGIONAL EASEMENTS ONLY
 DATE: Mar. 18 2008
 C447400

INSPECTORS COPY

REV. NO.	DATE	REVISIONS	BY
1	MAY 16/07	ISSUED FOR APPROVAL	D.H.
2	OCT 25/07	RE-ISSUED FOR APPROVAL	D.H.
3	OCT 31/07	PER TRCA COMMENTS	D.H.
4	FEB. 21/08	PER TOWN OF CALEDON COMMENTS	D.H.
5	MAR. 18/08	PER REGION OF PEEL COMMENTS	D.H.



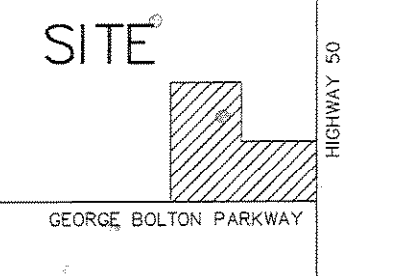
INDUSTRIAL / COMMERCIAL

GENERAL NOTES

MEASUREMENTS
ALL DIMENSIONS ARE IN METERS, EXCEPT PIPE DIAMETERS WHICH ARE IN MILLIMETERS, UNLESS OTHERWISE SPECIFIED.

GENERAL
1. CONSTRUCTION FOR THIS PROJECT TO COMPLY WITH THE MOST CURRENT VERSION OF THE DEPARTMENT STANDARDS, REGULATIONS AND GUIDELINES PREPARED BY THE TOWN OF CALEDON, PUBLIC WORKS & ENGINEERING DEPARTMENT, RECORDS OF PEEL STANDARDS AND SPECIFICATIONS, AND THE ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS.
2. ALL PROPOSED CONSTRUCTION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
3. LOCATION OF EXISTING SERVICES AND UTILITIES ARE NOT GUARANTEED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND MAINTAINING EXISTING UTILITIES. ANY CHANGES SHALL BE REPAIRED AT THE CONTRACTOR'S COST TO THE SATISFACTION OF THE APPROPRIATE UTILITY.
4. A MINIMUM OF 48 HOURS PRIOR TO COMMENCING CONSTRUCTION WITHIN THE MUNICIPAL RIGHT OF WAY THE CONTRACTOR MUST CONTACT THE FOLLOWING:
- TOWN OF CALEDON PUBLIC WORKS & ENGINEERING DEPARTMENT @ 905-884-2272
- REGION OF PEEL @ 905-741-7000
- ENERGY SERVICES @ 905-884-8100
- HYDRO ONE @ 1-888-855-4451
- BELL CANADA @ 416-291-4772
- ROGERS CABLE @ 905-887-2914
5. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
6. STANDARD CONTROL DEVICES ARE TO BE INSTALLED PRIOR TO ANY CONSTRUCTION AND SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD TO THE SATISFACTION OF THE TOWN AND THE APPROPRIATE COMMUNITY AUTHORITY. EXISTING CONTROL DEVICES TO BE REPAIRED AFTER EACH MAJOR PHASE OF CONSTRUCTION AND REPAIRED AS REQUIRED. EXISTING CONTROL DEVICES SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD.
7. ALL WORK IS TO BE PROVIDED AT ALL LOCATIONS WHERE CONSTRUCTION OPERATES ON THE SITE. AND MUST BE PROVIDED BY THE CONTRACTOR WITHIN THE MUNICIPAL RIGHT OF WAY. THIS SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD.
8. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
9. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
10. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
11. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
12. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
13. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
14. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.
15. ALL MATERIALS TO BE USED SHALL BE STORED AND DELIVERED TO A LOCATION APPROVED BY THE PUBLIC WORKS & ENGINEERING DEPARTMENT.

VILLAGE OF BOLTON
TOWN OF CALEDON



SANITARY AND STORM SEWER NOTES

1. ALL CONCRETE AND PLASTIC PIPE SHALL HAVE RUBBER JOINTS.
2. ALL CONCRETE SHALL BE CONSTRUCTED WITH BEDDING IN ACCORDANCE WITH CPD P-20.2 CLASS 'B' UNLESS OTHERWISE NOTED.
3. PLASTIC STORM SEWER PIPE SHALL BE PVC ULTRA RIB OR APPROVED EQUAL UP TO A MAX. DIAMETER OF 600MM.
4. SAN. SEWER PIPE TO BE PVC SDR 35 AND SHALL CONFORM TO C.S.A.-B-182.3.4.
5. SAN. SEWER MANHOLES SHALL BE INSTALLED IN ACCORDANCE WITH RECORDS OF PEEL STANDARD SPECIFICATIONS 3.1-1 TO 3.1-4 AND 3.2-1 TO 3.2-4. MANHOLE COVERS TO HAVE THE WORD 'SANITARY' CAST INTO THE COVER WITH 50MM LETTERS. STORAGE TANK DUTY FRAME AND COVER TO BE HEAVILY GALVANIZED UNLESS OTHERWISE SPECIFIED.
6. ALL PLUGS TO BE MARKED WITH '4' X '4' STUDS.

WATERMAIN NOTES

1. ALL WATERMAIN AND WATER SERVICE MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO THE CURRENT RECORDS OF PEEL PUBLIC WORKS STANDARDS AND SPECIFICATIONS.
2. WATERMAIN AND/OR WATER SERVICE MATERIALS 100MM (4") AND LARGER MUST BE PVC CLASS 150 WPA TO A.M.R.A. SPEC. C900-75. SIZES 50MM (2") AND SMALLER TO BE POLYETHYLENE PIPE 2500 TUBE STIFFNESS 185 IN ACCORDANCE WITH C.S.A. B-177-1975. A.M.R.A. SPEC. C900-75. SIZES 20 TO 50MM (1" TO 2").
3. WATERMAIN AND/OR WATER SERVICES ARE TO HAVE A MINIMUM COVER OF 1.7M (5'-7") WITH A MIN. HORIZONTAL SPACING OF 1.2M (4'-0") FROM THOSE SERVICES AND OTHER UTILITIES.
4. PROVISIONS FOR FLOODING WATER LINES PRIOR TO TESTING MUST BE PROVIDED WITH AT LEAST A 50MM (2") OUTLET ON 150MM (4") AND LARGER LINES. ON FIRE LINES, FLOORING DRAIN TO BE 100MM (4") DIA. MIN. OR A HORIZONTAL. ALL CURB STOPS TO BE 30MM (1 1/2") OFF THE FACE OF THE BUILDING UNLESS OTHERWISE NOTED.
5. HYDRANT AND VALVE SET TO REGION OF PEEL STANDARD DRAWING 1-6-1. DIMENSION 'A' AND 'B', 0.1m AND 0.2m, RESPECTIVELY. HYDRANT SHALL BE INSTALLED TO GRADING AS SHOWN ON THE APPROVED SITE PLAN. COPY OF SHEET MUST BE SUPPLIED TO INSPECTOR PRIOR TO COMMENCEMENT OF WORK, WHERE REQUESTED BY INSPECTOR.
6. WATERMANS MUST HAVE A MINIMUM VERTICAL CLEARANCE OF 1.5m (5') OVER AND 0.2m (8") UNDER SERVICES AND ALL OTHER UTILITIES WHEN CROSSING.
7. WATERMANS TO BE INSTALLED TO GRADING AS SHOWN ON THE APPROVED SITE PLAN. COPY OF SHEET MUST BE SUPPLIED TO INSPECTOR PRIOR TO COMMENCEMENT OF WORK, WHERE REQUESTED BY INSPECTOR.
8. WATERMANS MUST HAVE A MINIMUM VERTICAL CLEARANCE OF 1.5m (5') OVER AND 0.2m (8") UNDER SERVICES AND ALL OTHER UTILITIES WHEN CROSSING.
9. ALL PROPOSED WATERMAIN PIPING MUST BE ISOLATED FROM EXISTING LINES IN ORDER TO ALLOW INDEPENDENT PRESSURE TESTING AND CLEANING.
10. ALL WATERMANS LOCATED IN ENGINEERED FILL AREAS REQUIRE RESTRAINING JOINTS.
11. ALL PLUGS TO BE MARKED WITH '4' X '4' STUDS.

ROAD WORK NOTES

1. DRIVEWAY GRANULAR BASE AND SUB-BASE TO BE CONSTRUCTED OF THE FOLLOWING:
- BASE - 150MM GRANULAR 'A' OR 150MM OF 19MM CRUSHER RUN LIMESTONE
- SUB-BASE - 150MM GRANULAR 'B' OR 150MM OF 19MM CRUSHER RUN LIMESTONE
- GRANULAR BASE AND SUB-BASE MATERIALS SHALL CONFORM TO O.P.S.S. GRADATION SPECIFICATIONS AND SHALL BE COMPACTED TO 98% STANDARD PROCTOR MAXIMUM DRY DENSITY (SPD), OR AS RECOMMENDED BY THE GEOTECHNICAL CONSULTANT.
2. SUB-BASE TO BE PROOF ROLLED PRIOR TO INSTALLATION OF GRANULAR SUB-BASE MATERIALS. SPOT TESTS TO BE CONDUCTED AND FILED WITH GRANULAR BY LOGS. GRANULAR TO BE COMPACTED TO 98% OR AS RECOMMENDED BY THE GEOTECHNICAL CONSULTANT.
3. UPON COMPLETION OF THE INSTALLATION OF GRANULAR MATERIALS, ELEVATIONS SHOULD BE CHECKED TO ENSURE WORKS CORRESPOND TO THE APPROVED DRAWING DESIGN.
4. DRIVEWAY ENTRANCE TO BE FINISHED TO EDGE OF EX. PAVEMENT TO STREET LINE WITH 40MM HILL AND 65MM HIGH SIDE CURBS AND PARKING AREAS TO BE FINISHED WITH 40MM HILL AND 65MM HIGH SIDE CURBS AND PARKING AND LIGHT TRAFFIC AREAS AND 40MM HILL AND 65MM HILL FOR TRUCK TRAFFIC AND FIRE ROUTE AREAS.
5. CONCRETE CURB SHALL BE IN ACCORDANCE WITH MOST CURRENT VERSION OF PROJECT 'TYPE' PLAN.

REGION OF PEEL STANDARD DRAWINGS

- STD 1-1-1 - CIRCULAR PRECAST CHAMBER
- STD 1-2-5 - STANDARD CHAMBER STEPS ALUMINUM
- STD 1-3-5 - CHECK VALVE IN CHAMBER END, BTF-PRESS
- STD 1-3-8 - VALVE STEM EXTENSION IN VALVE BOX
- STD 1-4-5 - 38 AND 50 T-10 METER IN BUILDING
- STD 1-5-5 - WATERMAIN SUPPORT BRIDGING DISTURBED GROUND
- STD 1-5-6 - HORIZONTAL BUILDING RISER BONES AND REDUCERS
- STD 1-6-4 - FIRE LINE & DOMESTIC CONNECTION
- STD 1-6-5 - BUILDING RISER DETAIL 100MM & LARGER
- STD 1-7-1 - WATER SERVICE CONNECTION
- STD 1-8-5 - SERVICE FOR MULTIPLE BUILDING COMMERCIAL STEPS, IE RESTAURANTS AND OFFICE BUILDINGS
- STD 2-1-1 - PRECAST MANHOLE 900MM DIA.
- STD 2-1-4 - MANHOLE BENCHING DETAILS
- STD 2-2-2 - STANDARD HEAVY DUTY FRAME & COVER
- STD 2-2-4 - STANDARD MANHOLE STEPS ALUMINUM
- STD 2-3-1 - BEDDING DETAILS FOR SANITARY SEWERS
- STD 2-4-2 - SERVICE CONNECTIONS FOR FLEXIBLE (BTF) DOUBLE SERVICE CONNECTIONS IN COMMON TRENCH
- STD 2-4-3 - SERVICE CONNECTIONS IN COMMON TRENCH

ONTARIO PROVINCIAL STANDARD DRAWINGS

- OPSD - 60010 - CONCRETE BARRIER CURB

Calder Engineering Ltd.
13226 Coleraine Drive, Bolton, ON L7E 3B2
T 905-857-7600 F 905-857-5900

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PUBLIC WORKS
REGION OF PEEL

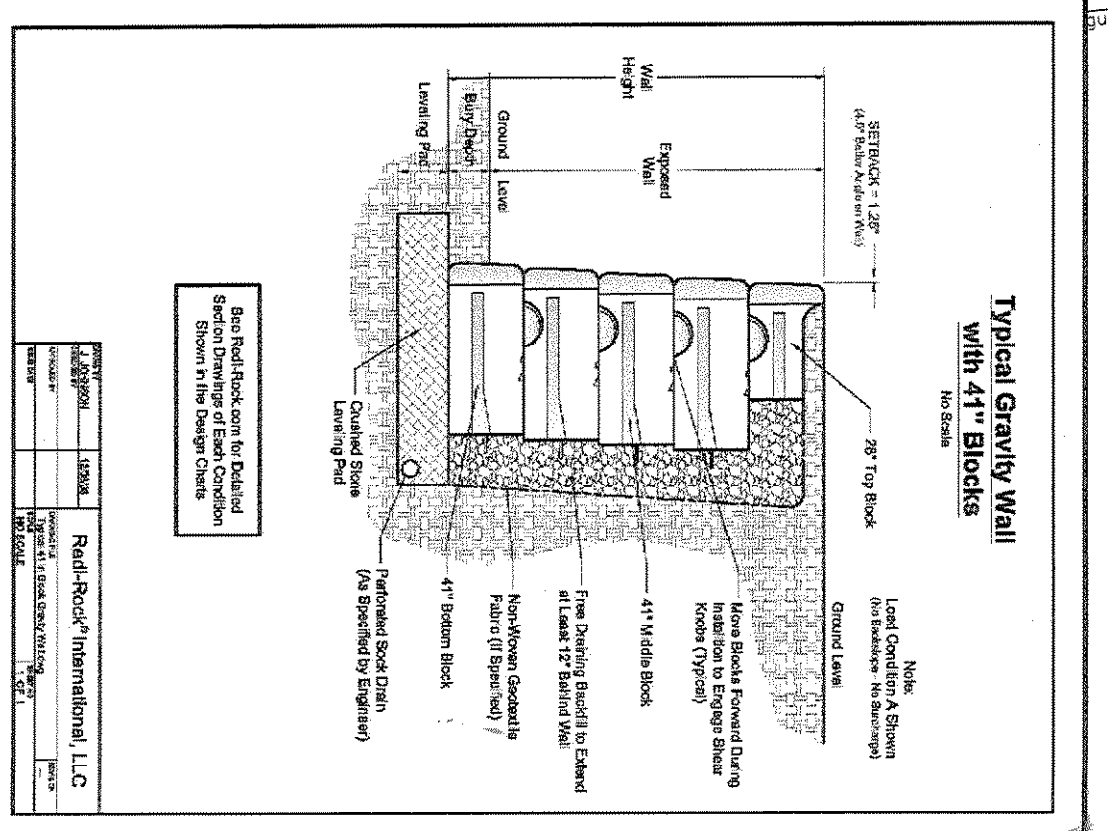
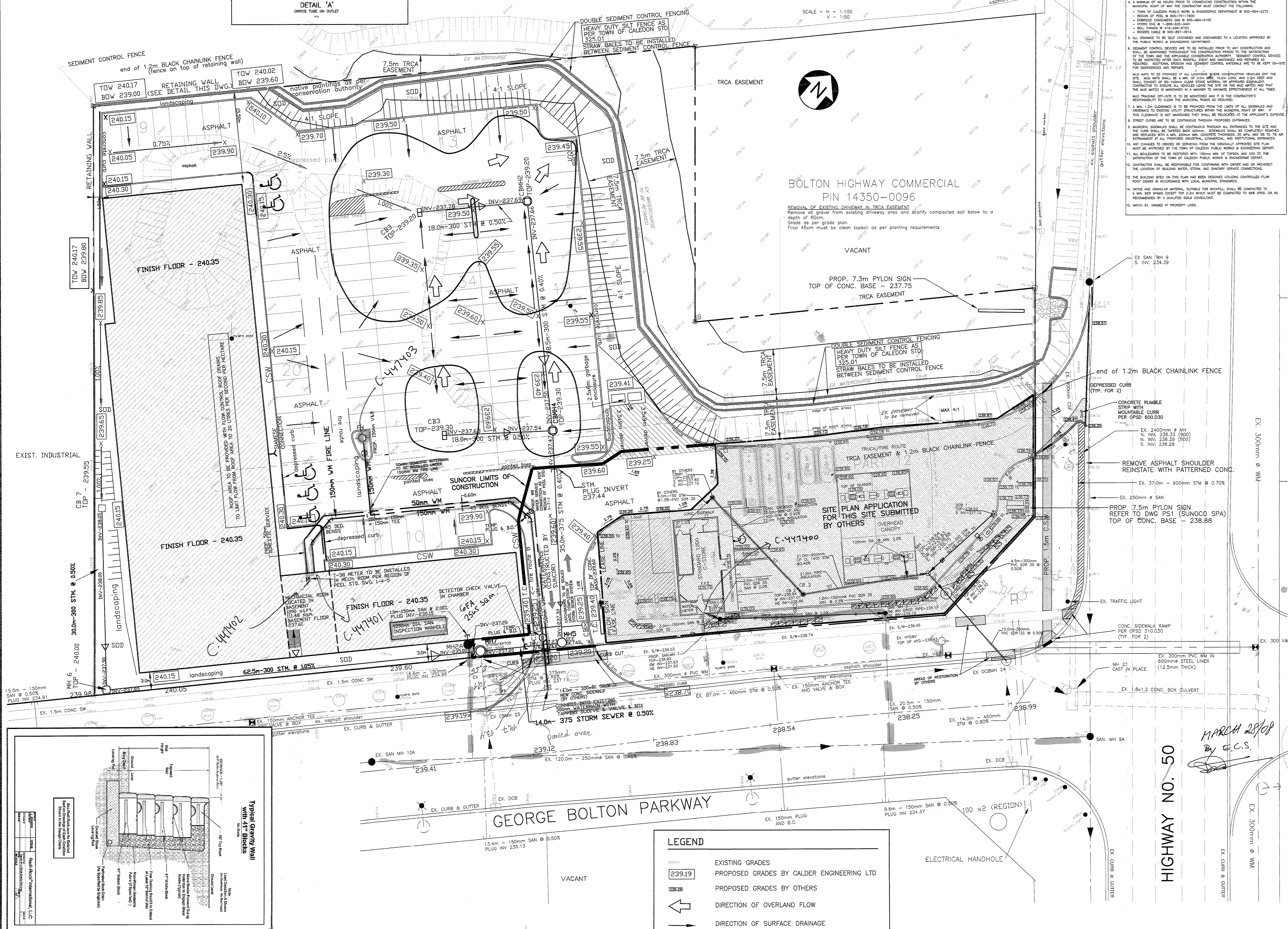
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APPROVED BY: [Signature]

CLIENT: **FARVIEW HOLDINGS**

PROJECT: **12476 REGIONAL ROAD 50**

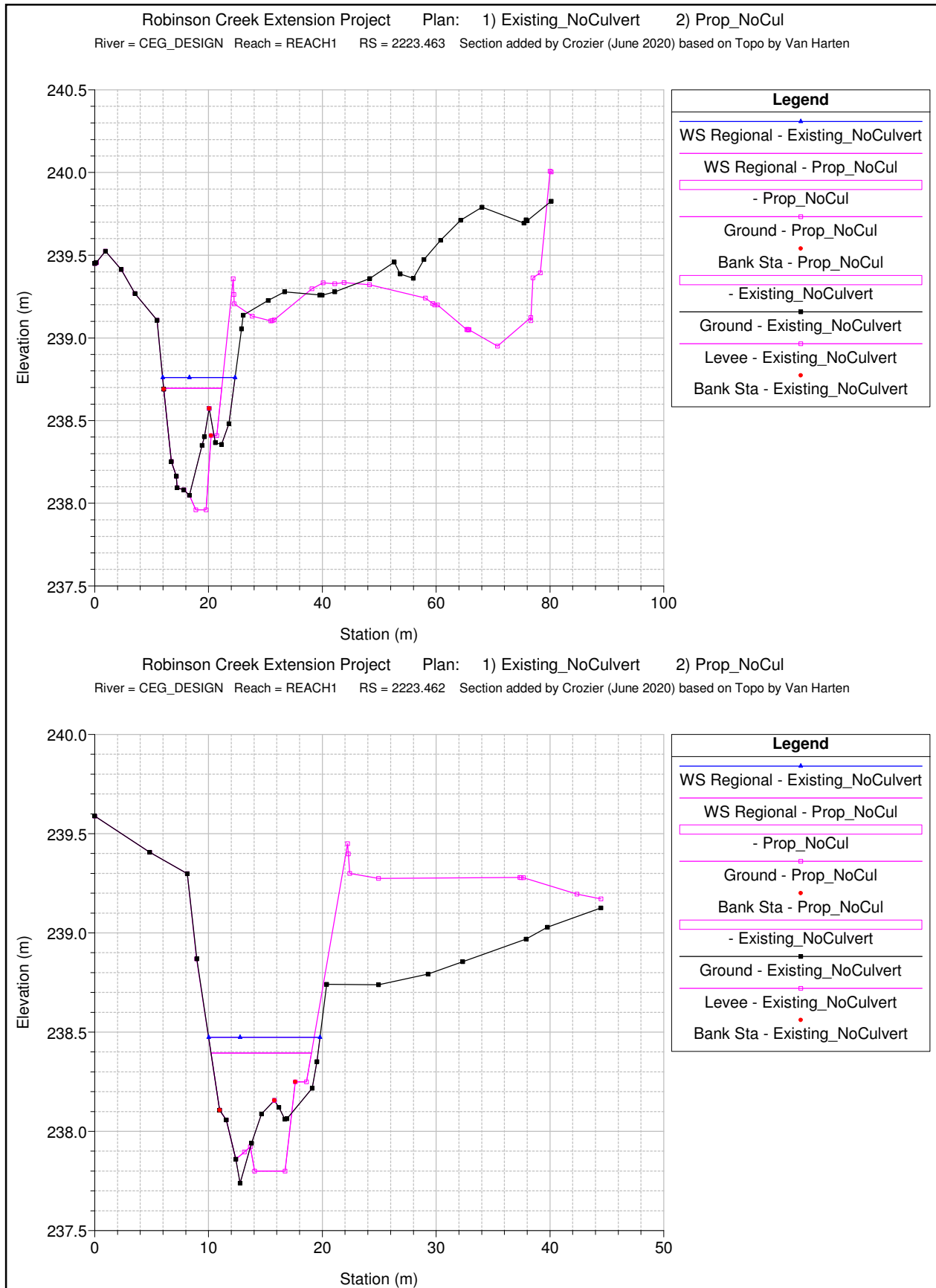
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Designed by: R.W. Drawing No. [Number] Sheet No. [Number]
Drawn by: AAF Date: JUNE 2006 03-122-001 1 / 1
Chk'd by: JP

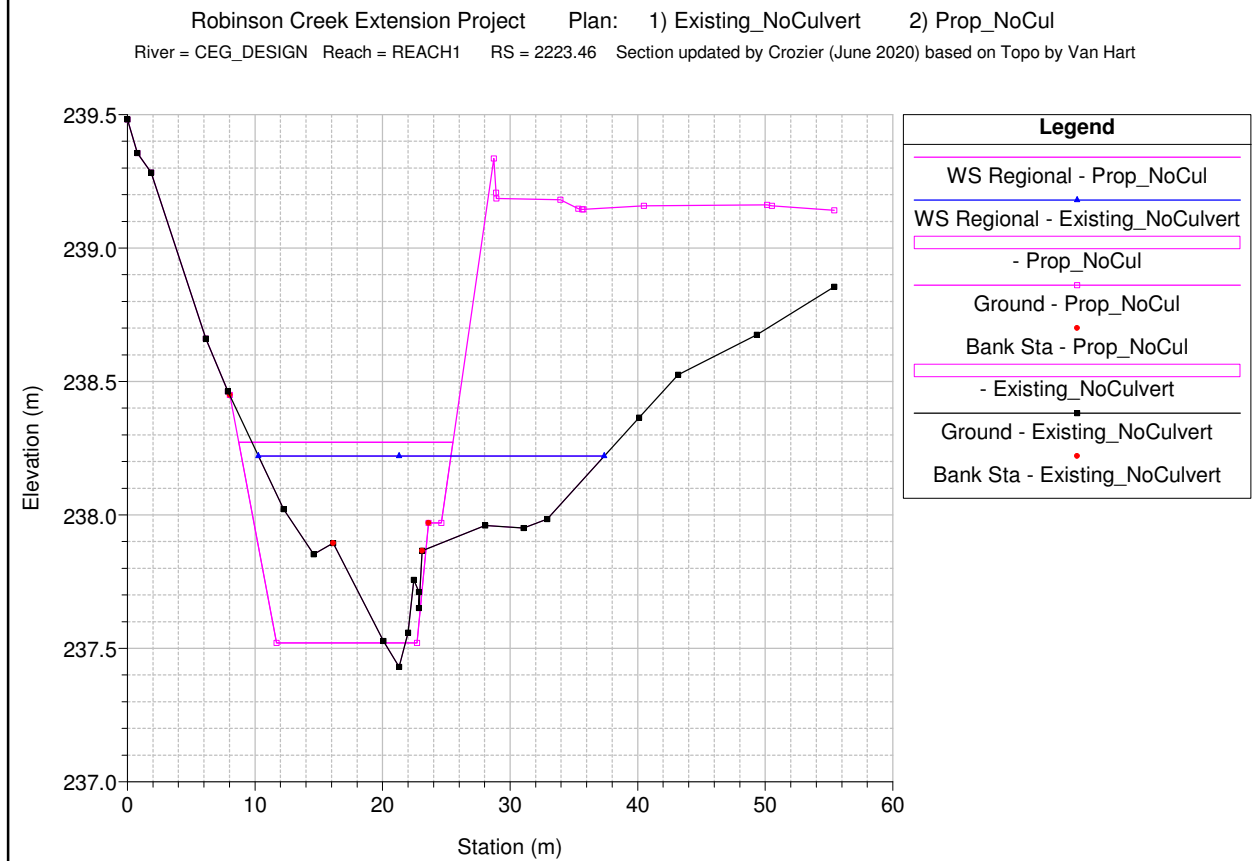
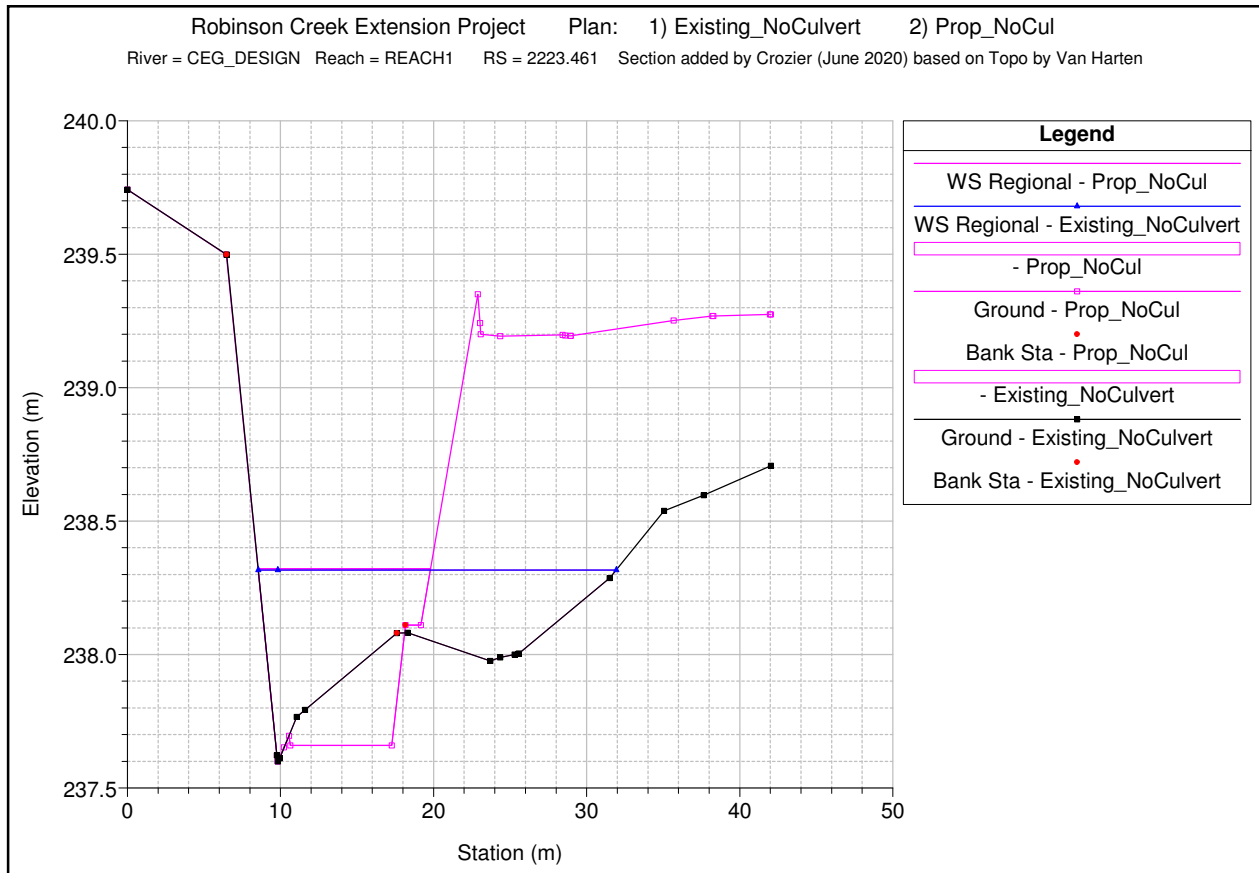


C-447401 503#1

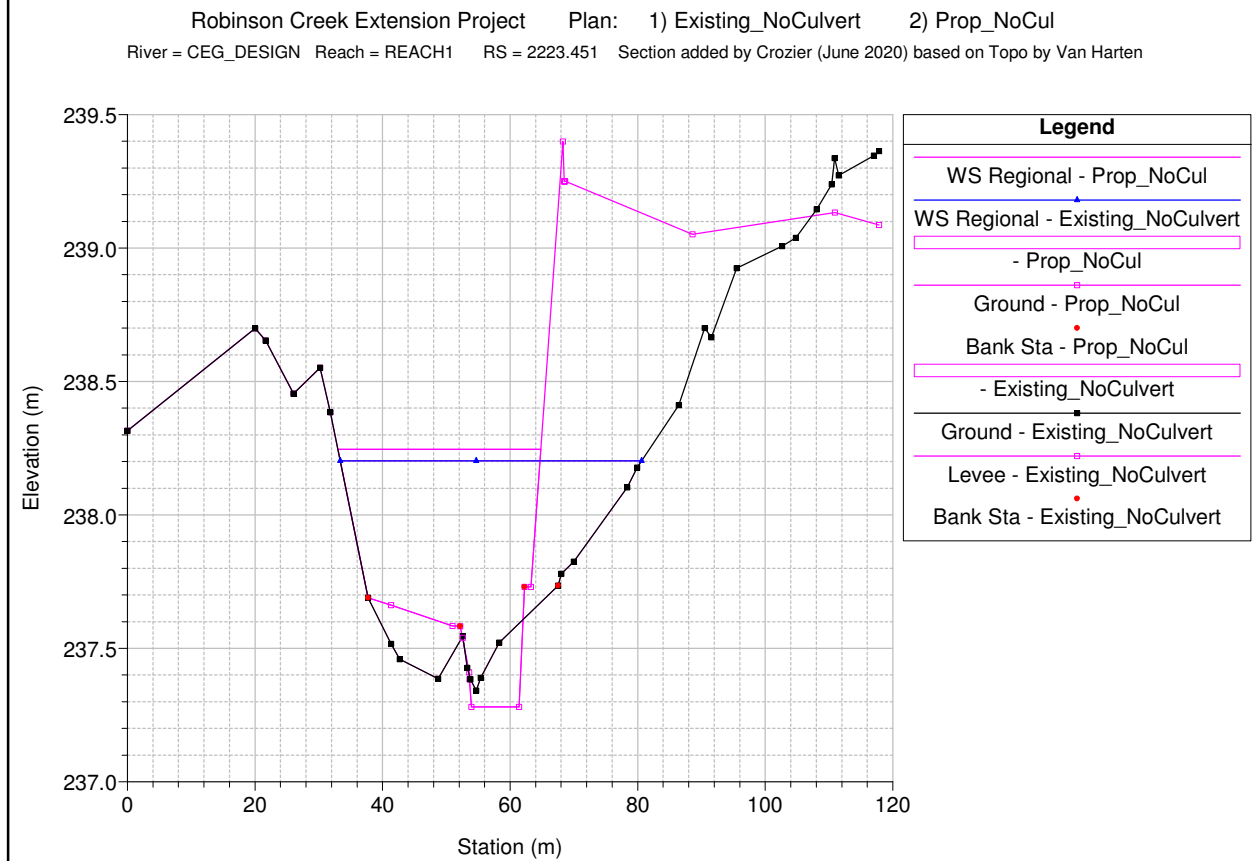
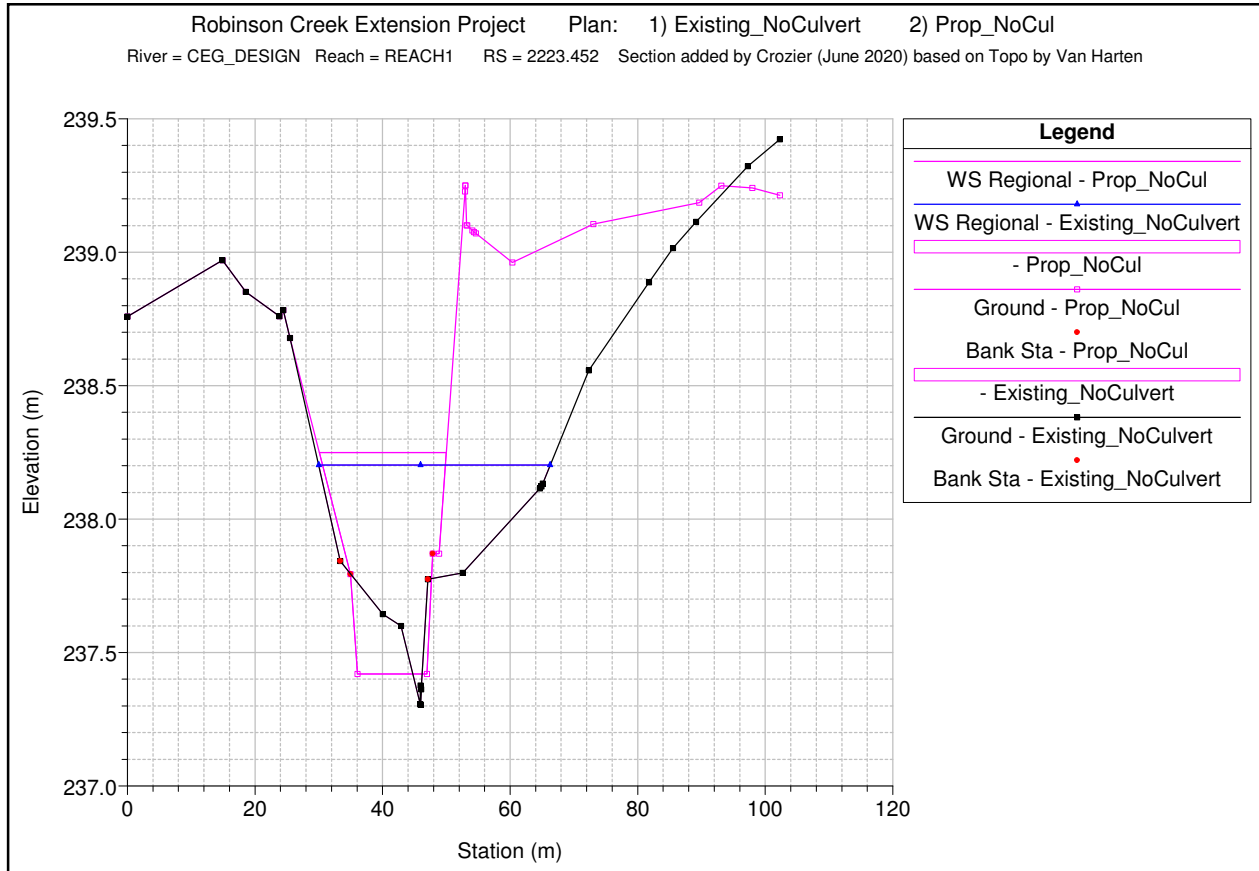
No Culvert Scenario



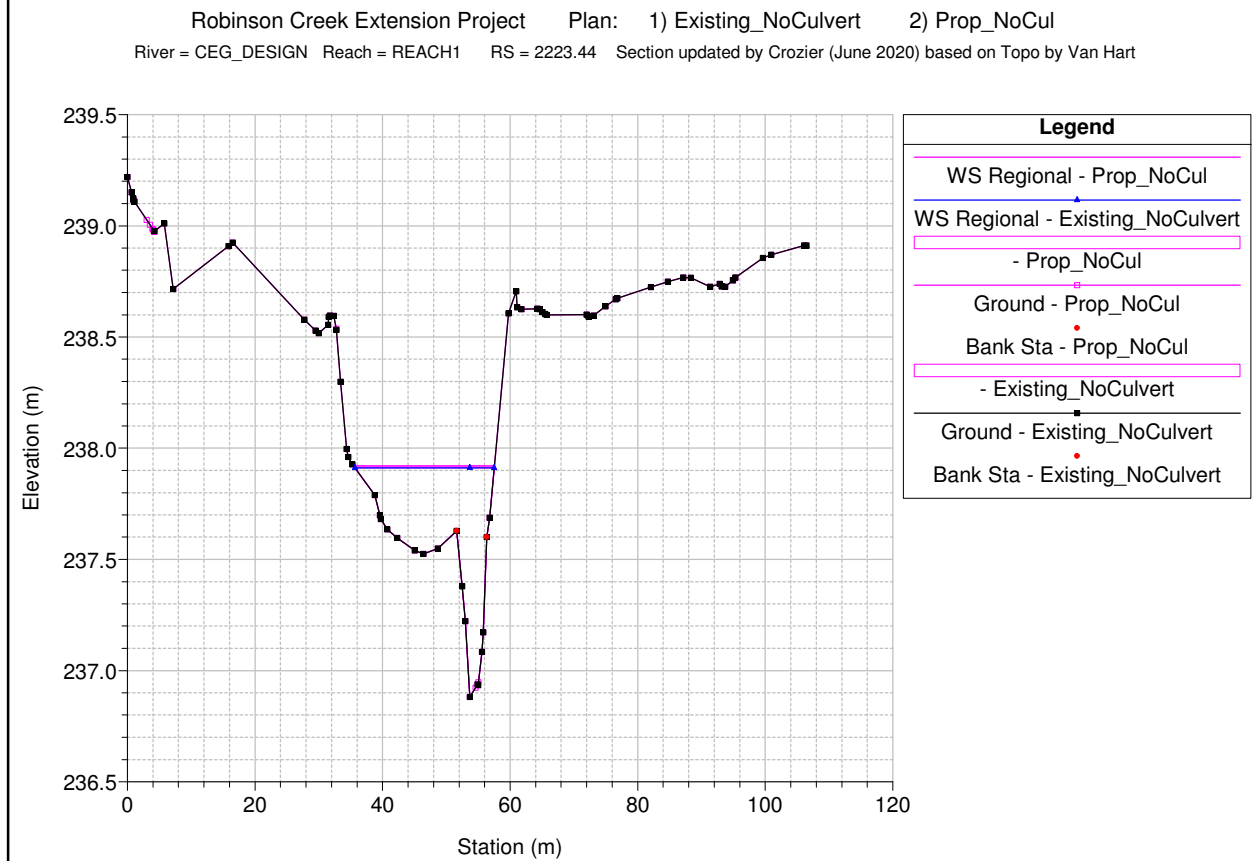
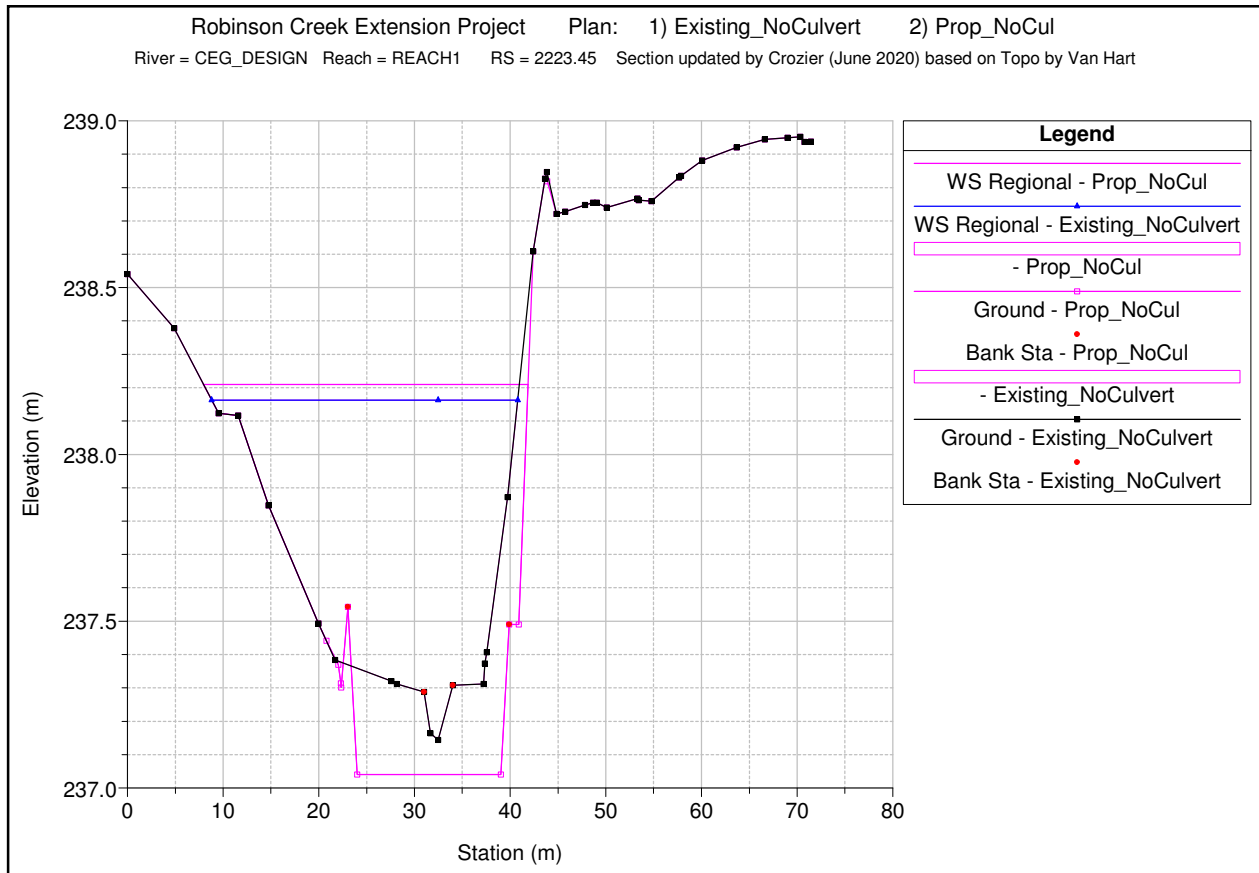
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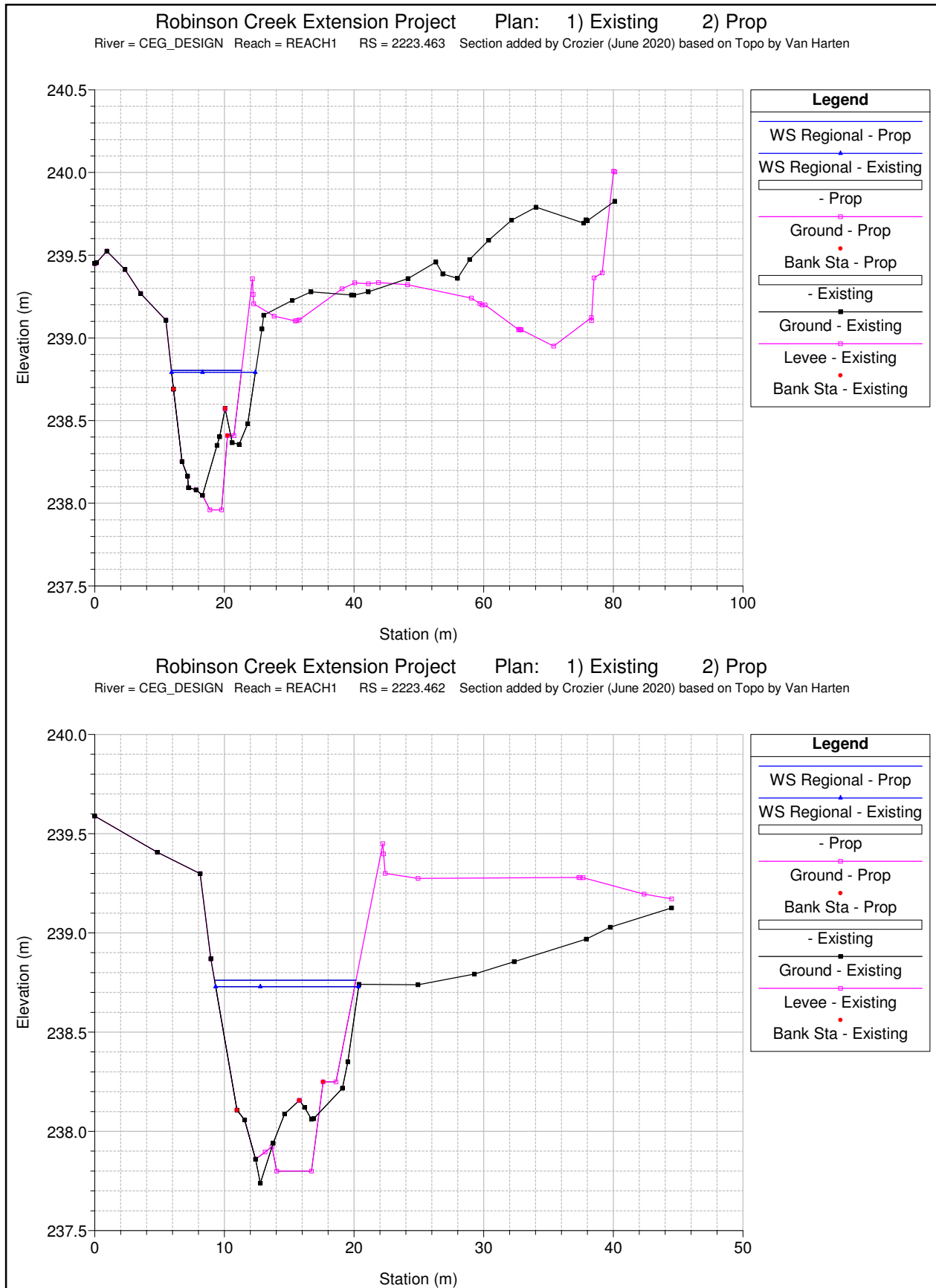


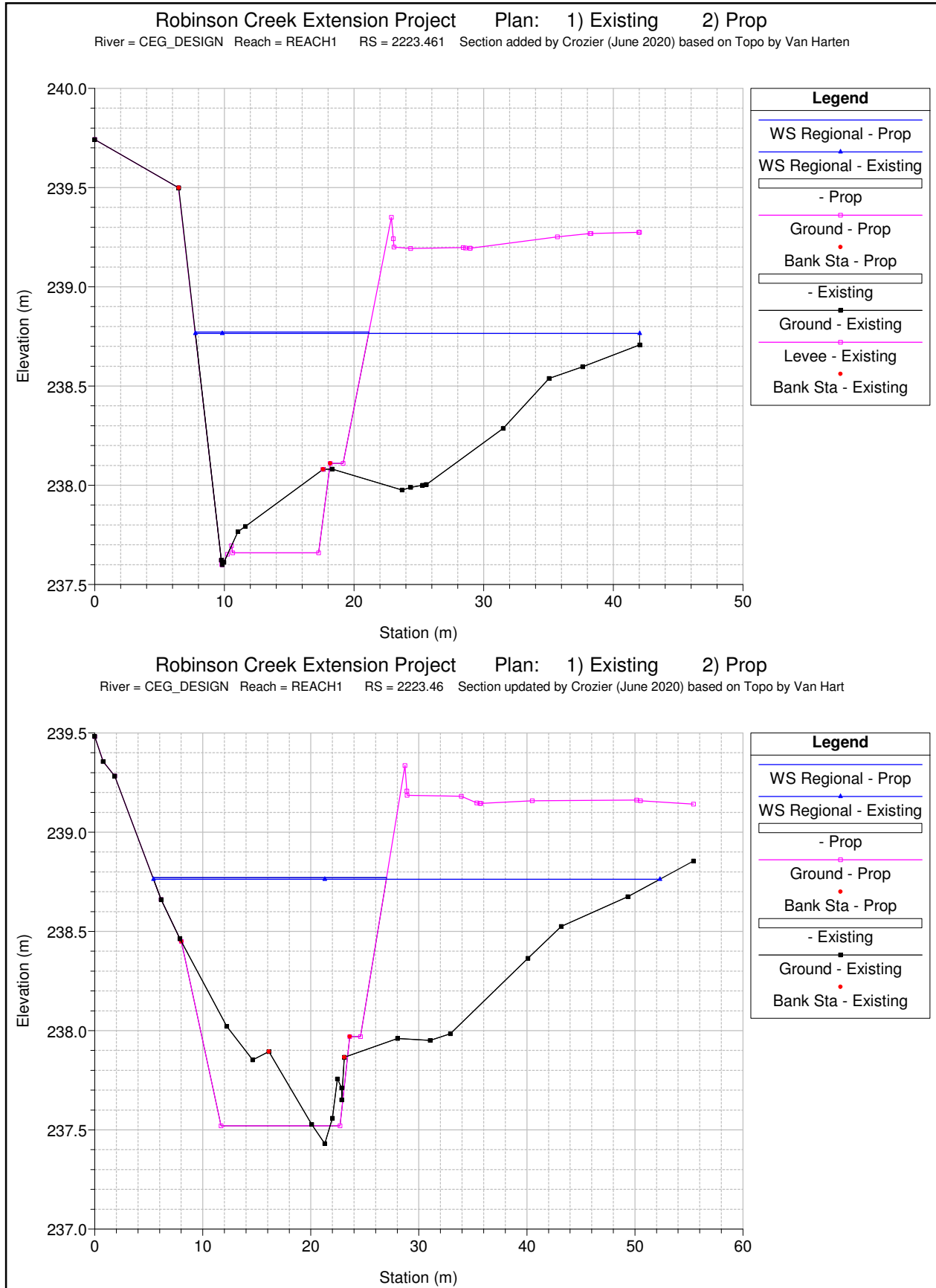
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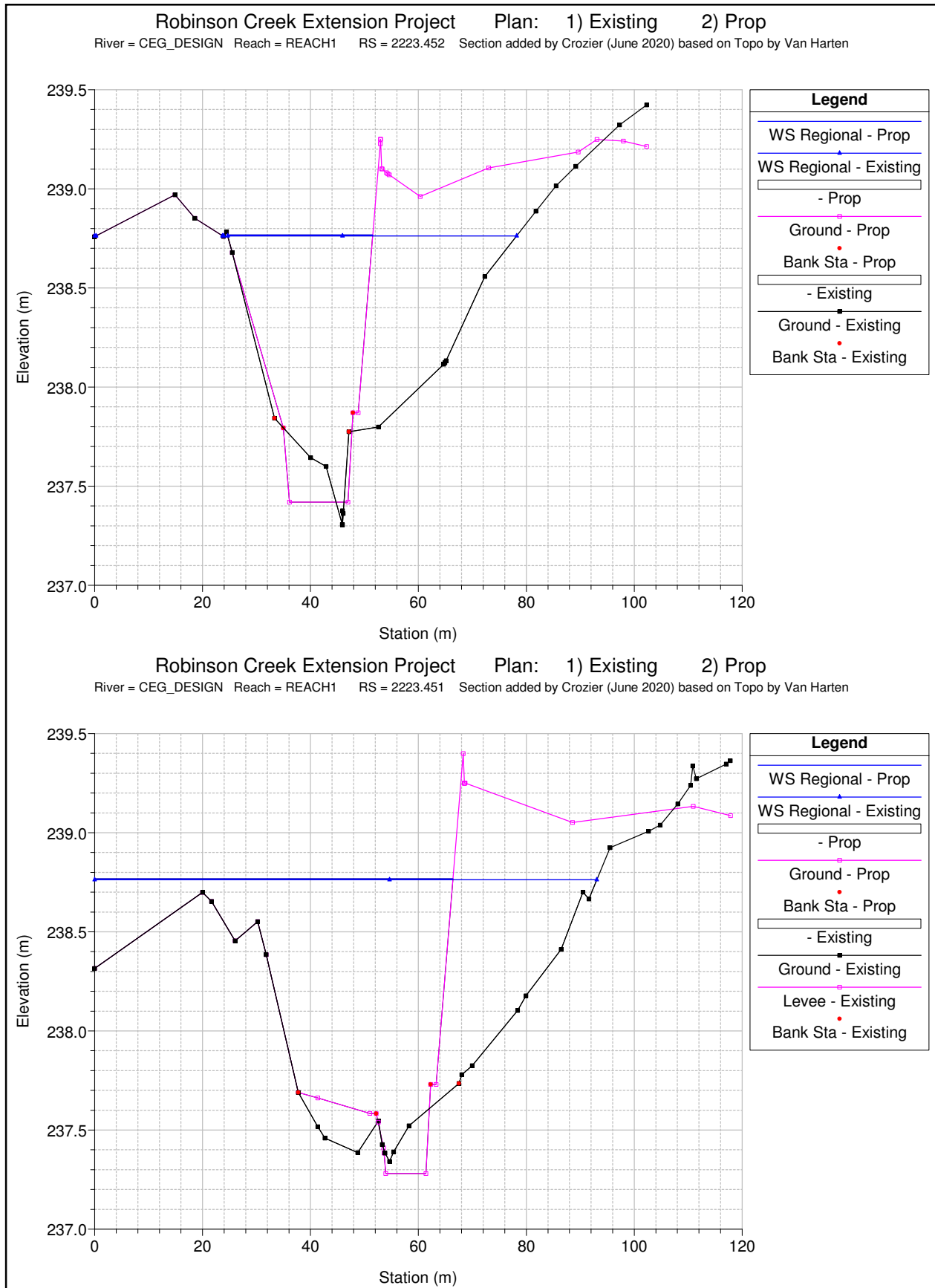


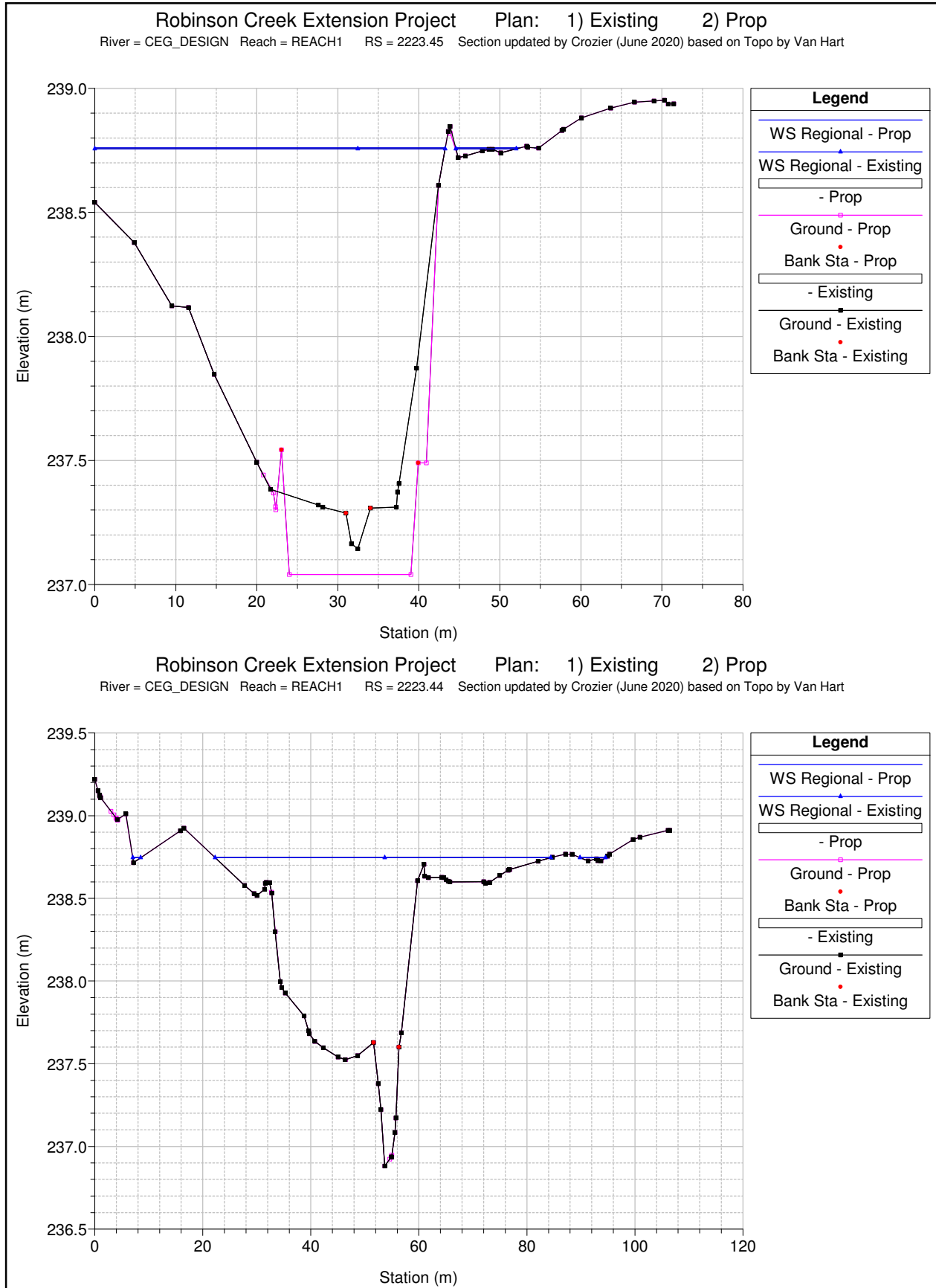
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HEC-RAS River: CEG_DESIGN Reach: REACH1 Profile: Regional (Continued)

Reach	River Sta	Profile	Plan	Q Total (m ³ /s)	Min Ch El (m)	W.S. Elev (m)	Crit W.S. (m)	E.G. Elev (m)	E.G. Slope (m/m)	Vel Chnl (m/s)	Flow Area (m ²)	Top Width (m)	Froude # Chl
REACH1	2223.25	Regional	Prop	15.17	230.50	232.62		232.67	0.001102	1.04	22.00	28.92	0.25
REACH1	2223.25	Regional	Prop_NoCul	15.17	230.50	232.62		232.67	0.001102	1.04	22.00	28.92	0.25
REACH1	2223.24	Regional	Existing	15.17	230.20	232.45	231.51	232.48	0.000654	0.94	21.14	33.30	0.24
REACH1	2223.24	Regional	Existing_NoCulvert	15.17	230.20	232.45	231.51	232.48	0.000654	0.94	21.14	33.30	0.24
REACH1	2223.24	Regional	Prop	15.17	230.20	232.45	231.51	232.48	0.000654	0.94	21.14	33.30	0.24
REACH1	2223.24	Regional	Prop_NoCul	15.17	230.20	232.45	231.51	232.48	0.000654	0.94	21.14	33.30	0.24
REACH1	2223.23	Regional	Existing	15.17	230.20	232.44	231.72	232.47	0.000708	0.96	24.42	53.80	0.23
REACH1	2223.23	Regional	Existing_NoCulvert	15.17	230.20	232.44	231.72	232.47	0.000708	0.96	24.42	53.80	0.23
REACH1	2223.23	Regional	Prop	15.17	230.20	232.44	231.72	232.47	0.000708	0.96	24.42	53.80	0.23
REACH1	2223.23	Regional	Prop_NoCul	15.17	230.20	232.44	231.72	232.47	0.000708	0.96	24.42	53.80	0.23
REACH1	2223.22	Regional	Existing	15.17	230.20	232.32	232.32	232.44	0.002303	1.91	17.31	63.65	0.56
REACH1	2223.22	Regional	Existing_NoCulvert	15.17	230.20	232.32	232.32	232.44	0.002303	1.91	17.31	63.65	0.56
REACH1	2223.22	Regional	Prop	15.17	230.20	232.32	232.32	232.44	0.002303	1.91	17.31	63.65	0.56
REACH1	2223.22	Regional	Prop_NoCul	15.17	230.20	232.32	232.32	232.44	0.002303	1.91	17.31	63.65	0.56
REACH1	2223.215		Culvert										
REACH1	2223.21	Regional	Existing	15.17	230.00	231.80	231.80	231.92	0.002879	1.91	15.65	56.06	0.65
REACH1	2223.21	Regional	Existing_NoCulvert	15.17	230.00	231.80	231.80	231.92	0.002879	1.91	15.65	56.06	0.65
REACH1	2223.21	Regional	Prop	15.17	230.00	231.80	231.80	231.92	0.002879	1.91	15.65	56.06	0.65
REACH1	2223.21	Regional	Prop_NoCul	15.17	230.00	231.80	231.80	231.92	0.002879	1.91	15.65	56.06	0.65
REACH1	2223.20	Regional	Existing	17.32	229.70	231.67	231.05	231.72	0.000983	1.16	26.90	64.84	0.30
REACH1	2223.20	Regional	Existing_NoCulvert	17.32	229.70	231.67	231.05	231.72	0.000983	1.16	26.90	64.84	0.30
REACH1	2223.20	Regional	Prop	17.32	229.70	231.67	231.05	231.72	0.000983	1.16	26.90	64.84	0.30
REACH1	2223.20	Regional	Prop_NoCul	17.32	229.70	231.67	231.05	231.72	0.000983	1.16	26.90	64.84	0.30
REACH1	2223.19	Regional	Existing	17.32	229.45	231.59	231.59	231.69	0.003486	1.82	17.59	84.47	0.40
REACH1	2223.19	Regional	Existing_NoCulvert	17.32	229.45	231.59	231.59	231.69	0.003486	1.82	17.59	84.47	0.40
REACH1	2223.19	Regional	Prop	17.32	229.45	231.59	231.59	231.69	0.003486	1.82	17.59	84.47	0.40
REACH1	2223.19	Regional	Prop_NoCul	17.32	229.45	231.59	231.59	231.69	0.003486	1.82	17.59	84.47	0.40
REACH1	2223.185		Culvert										
REACH1	2223.18	Regional	Existing	17.32	229.37	231.29	231.29	231.43	0.004197	2.03	14.02	49.43	0.58
REACH1	2223.18	Regional	Existing_NoCulvert	17.32	229.37	231.29	231.29	231.43	0.004197	2.03	14.02	49.43	0.58
REACH1	2223.18	Regional	Prop	17.32	229.37	231.29	231.29	231.43	0.004197	2.03	14.02	49.43	0.58
REACH1	2223.18	Regional	Prop_NoCul	17.32	229.37	231.29	231.29	231.43	0.004197	2.03	14.02	49.43	0.58
REACH1	2223.17	Regional	Existing	17.32	228.87	230.57	230.57	230.91	0.009942	3.28	12.73	23.71	0.87
REACH1	2223.17	Regional	Existing_NoCulvert	17.32	228.87	230.57	230.57	230.91	0.009942	3.28	12.73	23.71	0.87
REACH1	2223.17	Regional	Prop	17.32	228.87	230.57	230.57	230.91	0.009942	3.28	12.73	23.71	0.87
REACH1	2223.17	Regional	Prop_NoCul	17.32	228.87	230.57	230.57	230.91	0.009942	3.28	12.73	23.71	0.87
REACH1	2223.16	Regional	Existing	17.32	228.57	230.18	230.18	230.54	0.010151	2.84	9.19	16.75	0.87
REACH1	2223.16	Regional	Existing_NoCulvert	17.32	228.57	230.18	230.18	230.54	0.010151	2.84	9.19	16.75	0.87
REACH1	2223.16	Regional	Prop	17.32	228.57	230.18	230.18	230.54	0.010151	2.84	9.19	16.75	0.87
REACH1	2223.16	Regional	Prop_NoCul	17.32	228.57	230.18	230.18	230.54	0.010151	2.84	9.19	16.75	0.87
REACH1	2223.15	Regional	Existing	17.32	228.50	229.70	229.30	229.74	0.003874	1.03	23.83	47.62	0.33
REACH1	2223.15	Regional	Existing_NoCulvert	17.32	228.50	229.70	229.30	229.74	0.003874	1.03	23.83	47.62	0.33
REACH1	2223.15	Regional	Prop	17.32	228.50	229.70	229.30	229.74	0.003874	1.03	23.83	47.62	0.33
REACH1	2223.15	Regional	Prop_NoCul	17.32	228.50	229.70	229.30	229.74	0.003874	1.03	23.83	47.62	0.33
REACH1	2223.14	Regional	Existing	17.32	228.37	229.30		229.33	0.002337	1.27	29.29	66.03	0.43
REACH1	2223.14	Regional	Existing_NoCulvert	17.32	228.37	229.30		229.33	0.002337	1.27	29.29	66.03	0.43
REACH1	2223.14	Regional	Prop	17.32	228.37	229.30		229.33	0.002337	1.27	29.29	66.03	0.43
REACH1	2223.14	Regional	Prop_NoCul	17.32	228.37	229.30		229.33	0.002337	1.27	29.29	66.03	0.43
REACH1	2223.13	Regional	Existing	17.32	227.99	229.04	228.77	229.12	0.002685	1.44	17.10	34.31	0.47
REACH1	2223.13	Regional	Existing_NoCulvert	17.32	227.99	229.04	228.77	229.12	0.002685	1.44	17.10	34.31	0.47
REACH1	2223.13	Regional	Prop	17.32	227.99	229.04	228.77	229.12	0.002685	1.44	17.10	34.31	0.47
REACH1	2223.13	Regional	Prop_NoCul	17.32	227.99	229.04	228.77	229.12	0.002685	1.44	17.10	34.31	0.47
REACH1	2223.12	Regional	Existing	17.32	227.02	229.00		229.06	0.001006	1.30	29.18	63.27	0.31
REACH1	2223.12	Regional	Existing_NoCulvert	17.32	227.02	229.00		229.06	0.001006	1.30	29.18	63.27	0.31
REACH1	2223.12	Regional	Prop	17.32	227.02	229.00		229.06	0.001006	1.30	29.18	63.27	0.31
REACH1	2223.12	Regional	Prop_NoCul	17.32	227.02	229.00		229.06	0.001006	1.30	29.18	63.27	0.31
REACH1	2223.11	Regional	Existing	17.32	226.75	228.72	228.18	228.98	0.003459	2.30	7.54	35.72	0.57
REACH1	2223.11	Regional	Existing_NoCulvert	17.32	226.75	228.72	228.18	228.98	0.003459	2.30	7.54	35.72	0.57
REACH1	2223.11	Regional	Prop	17.32	226.75	228.72	228.18	228.98	0.003459	2.30	7.54	35.72	0.57
REACH1	2223.11	Regional	Prop_NoCul	17.32	226.75	228.72	228.18	228.98	0.003459	2.30	7.54	35.72	0.57
REACH1	2223.105		Culvert										
REACH1	2223.10	Regional	Existing	17.32	226.55	228.51		228.89	0.006315	2.71	6.52	29.96	0.71
REACH1	2223.10	Regional	Existing_NoCulvert	17.32	226.55	228.51		228.89	0.006315	2.71	6.52	29.96	0.71
REACH1	2223.10	Regional	Prop	17.32	226.55	228.51		228.89	0.006315	2.71	6.52	29.96	0.71
REACH1	2223.10	Regional	Prop_NoCul	17.32	226.55	228.51		228.89	0.006315	2.71	6.52	29.96	0.71

HEC-RAS River: CEG_DESIGN Reach: REACH1 Profile: Regional (Continued)

Reach	River Sta	Profile	Plan	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
				(m ³ /s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m ²)	(m)	
REACH1	2219.50	Regional	Prop_NoCul	38.30	220.15	222.14	222.14	222.42	0.009218	2.97	29.43	54.20	0.85